

CITY OF PORT MOODY

Development Permit No. DP000064

BC TRANSPORTATION FINANCING AUTHORITY
940 BLANSHARD STREET
VICTORIA, BC
V8W 3E6

1. This Development Permit is issued subject to compliance with all applicable City Bylaws, except as specifically varied or supplemented by this Permit.
2. This Permit applies to those lands in Port Moody, British Columbia more particularly described below and including all buildings, structures and other development thereon:

CLARKE STREET ROAD ALLOWANCE IN BETWEEN GRANT STREET AND
MOODY STREET ADJACENT TO THE SKYTRAIN RAIL LINE AS SHOWN IN
THE KEY PLAN IN THE DRAWING PACKAGE INCLUDED IN SCHEDULE "A"

(the "**Lands**")

3. The following plans and documents are made part of this Permit and, notwithstanding any other provision, no works shall be performed upon the Lands covered by this Permit, nor shall any building or structure be erected, constructed, repaired, renovated or sited, that is not in substantial accordance with the following and strictly in accordance with all terms and conditions of this Permit:
4. The following requirement is hereby imposed under section 490(1)(c) of the *Local Government Act*:
 - a) Substantial construction shall commence within two (2) years of the date of the Council resolution authorizing issuance of this Development Permit or the Permit will lapse.
5. The following requirements are hereby imposed under sections 489(b), 490(2) and 491(2), (4), (7), and (8) of the Local Government Act:

a) The site shall be developed in accordance with the attached plans and documents:

- 1) EMUP Propulsion Power Upgrades – 90% Submission, prepared by Franci Architecture Inc., dated July 19, 2023, pages 1 through 20. and attached as Schedule 'A'

6. As conditions of this Development Permit, the following shall be provided to the City of Port Moody for review:
 - a) Final IFC Architectural Drawings
 - b) Final Tree Management Plan
 - c) Final Landscape Plan
 - d) Stormwater Management Plan
 - e) Construction Management Plan
 - f) Erosion and Sediment Control Plan
 - g) Fire Department Access Plan
7. As a condition of this permit the owner shall notify the City Arborist upon installation of tree protection fencing, prior to commencement of any construction activities.
8. As a condition of this permit the owner will be required to provide full-depth road reconstruction for areas within the road right-of-way disturbed for underground service or mainline addition or abandonment.

Development Permit: Hazardous Lands – Soil Liquefaction

- (a) 60% Geotechnical Design Report – Titled “EMUP Propulsion Power Upgrades Project, Moody Centre Station – 60% Geotechnical Design Report, South Coast British Columbia Transportation Authority SNC-Lavalin Project: 675448, on file with the City, prepared SNC Lavalin and dated December 19, 2022. and attached as Schedule B to this permit, and any amendments thereto subsequently reviewed by the City.
4. Prior to the commencement of construction the owner will provide the City with an final Geotechnical Design Report which includes a statement confirming that the site is safe for the intended use.

CITY OF PORT MOODY, by its authorized signatories:

Michael Olubiyi
Manager of Development Planning
(for)

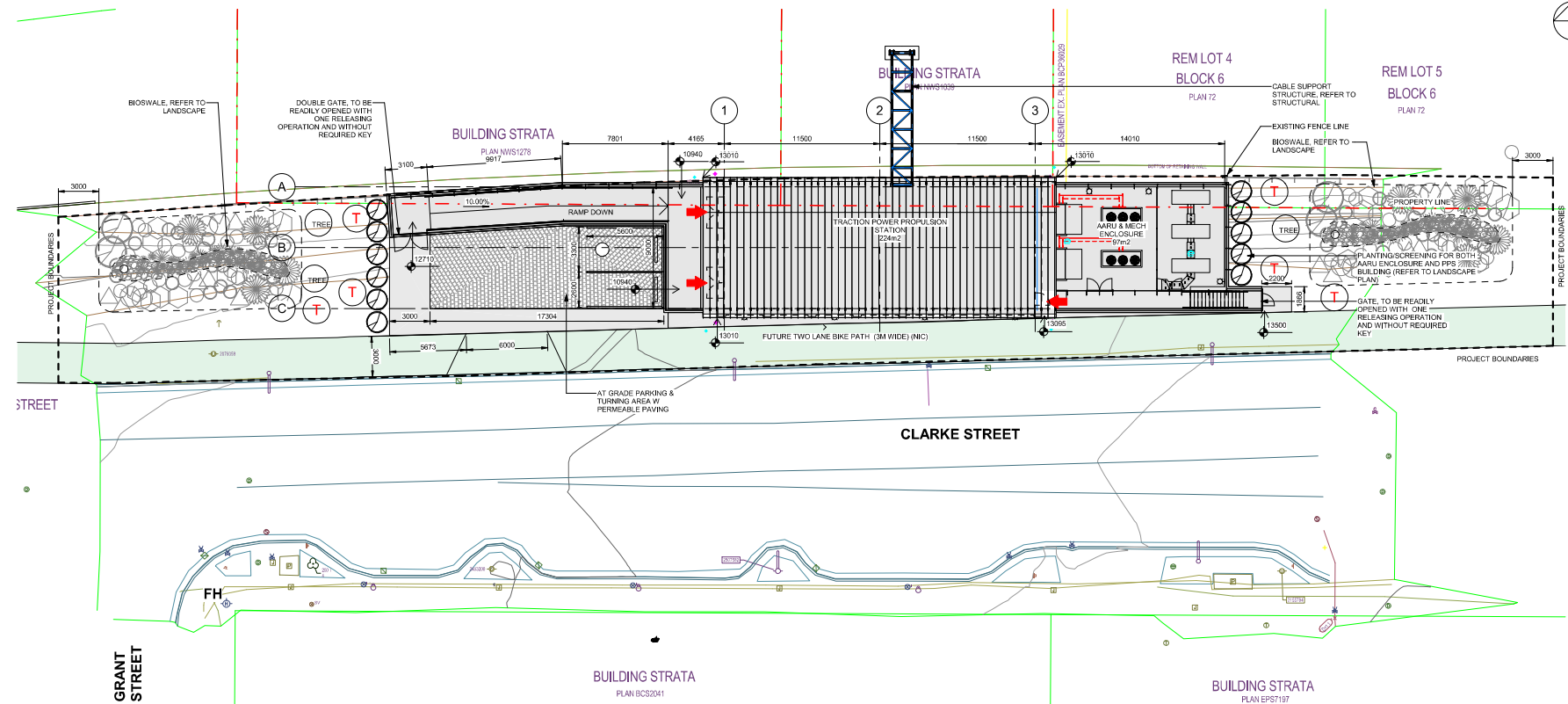
Kate Zanon
General Manager of Community Development

Pursuant to City of Port Moody Development Permit Delegation Bylaw, 2015, No. 3032

Dated on the ____ of _____, 2024.

SCHEDULE A

Architectural Drawings



1 SITE PLAN - CLARKE STREET
0005 1:100

SITE INFORMATION
(calculated based on project boundaries)

LOT AREA: 1472.1sq.m
BUILDING FOOTPRINT: 490.1sq.m
LOT COVERAGE = 33.3%
FLOOR AREA RATIO: 0.33

ISSUE/REVISION		REFERENCES
B	2023-07-19	90% SUBMISSION
A	2022-11-23	60% SUBMISSION
PA	2022-08-04	30% SUBMISSION
OR DATE	BY	APPRO



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SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

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SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
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CONSULTANTS

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-
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REGISTRATION

KEY PLAN

PROJECT

**EMUP
PROPULSION
POWER
UPGRADES**
PORT MOODY

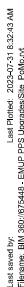
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DRAWN BY	VERONIKA KURBET
DESIGNED BY	VERONIKA KURBET
CHECKED BY	BRENDAN AVERY
APPROVED BY	LEWIS JUDD

CONTRACT NUMBER

PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
SITE PLAN

DRAWING NUMBER
193028-03 -1000- STMCZ- FR- AG- 0100-
-B

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Filename: BIM 360://675448 - EMUP PPS Upgrades/Site_PoMo.rvt



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604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

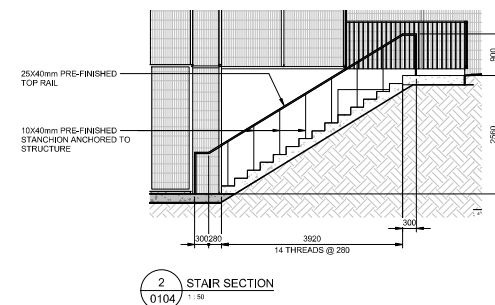
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Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

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APPROVED BY LEWIS JUDD

PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
GROUND LEVEL PLAN, RAMP

DRAWING NUMBER
193028-03 -1000- STMCZ- FR- AG- 0103-
-B



1 GROUND FLOOR PLAN. ELCLOSURE
0005 1:50

ISSUE/REVISION			REFERENCES
B	2023-07-19	90% SUBMISSION	
A	2022-11-23	60% SUBMISSION	
PA	2022-09-04	30% SUBMISSION	
IR	DATE	DESCRIPTION	APPR



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604 662-3555 tel 604 662-7688 fax
www.sncdavalin.com

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SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
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604 662-3555 tel 604 662-7688 fax
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CONSULTANTS

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

REGISTRATION

KEY PLAN

PROJECT

EMUP PROPULSION POWER UPGRADES

DRAWING INFORMATION	
DRAWN BY	VERONIKA KURBET
DESIGNED BY	VERONIKA KURBET
CHECKED BY	BRENDAN AVERY
APPROVED BY	LEWIS JUDD

CONTRACT NUMBER

PROJECT NUMBER	193028 (SNC #675448)
SHEET TITLE	GROUND LEVEL PLAN ENCLOSURE

DRAWING NUMBER
193028-03 -1000- STMCZ- FR- AG- 0104-
-B



ELEVATION MATERIAL LEGEND

MAS-2	FACE BRICK, COLOR TBC
MTL-3e	STANDING SEAM - BLACK, COLOUR TBC
MTL-3d	GUARD W PICKETS - BLACK, COLOUR TBC
CON-1	CONCRETE FACED RIGID INSULATION
PMP-DR	INSULATED METAL DOOR w/PRESSED METAL FRAME, PAINTED, COLOUR TBC
FC-1	FENCE, TYPE TBC

ISSUE/REVISION						REFERENCES	
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IR	DATE	BY		DESCRIPTION	APPR		



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SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
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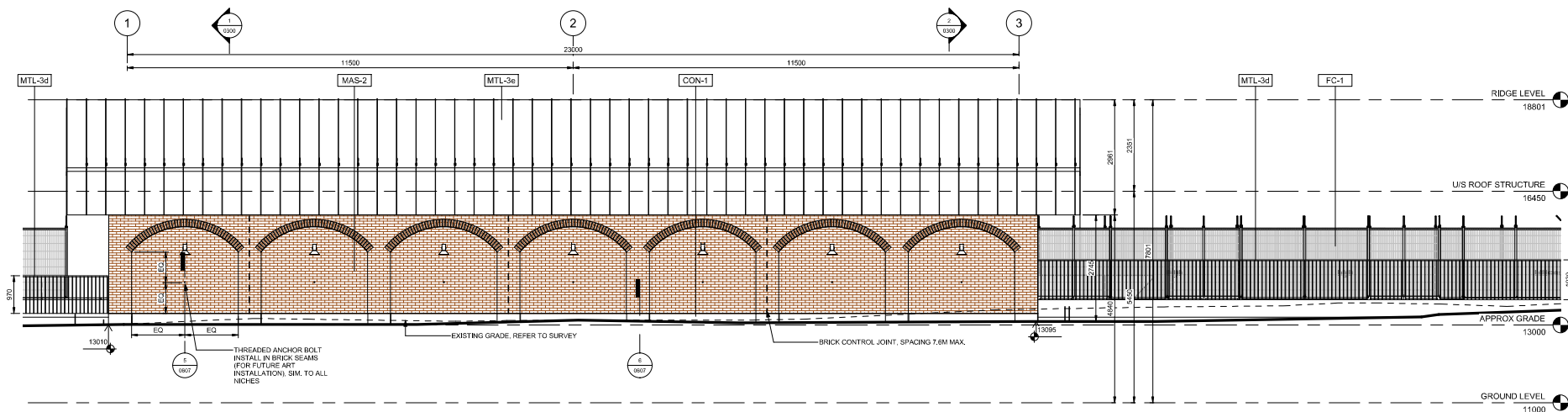
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Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
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CONSTRUCTION
2023-07-19

PROJECT
EMUP
PROPULSION
POWER
UPGRADES
PORT MOODY

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APPROVED BY	LEWIS JUDD

CONTRACT NUMBER
PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
ELEVATIONS
DRAWING NUMBER
193028-03-1000- STMCZ- FR- AG- 0200 -B



1 SOUTH ELEVATION
0102 1:50

ELEVATION MATERIAL LEGEND

MAS-2	FACE BRICK, COLOR TBC
MTL-3e	STANDING SEAM - BLACK, COLOUR TBC
MTL-3d	GUARD W PICKETS - BLACK, COLOUR TBC
CON-1	CONCRETE FACED RIGID INSULATION
PMP-DR	INSULATED METAL DOOR w/PRESSED METAL FRAME, PAINTED, COLOUR TBC
FC-1	FENCE, TYPE TBC

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SNC-Lavalin
Suite 1360 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
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STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

REGISTRATION

KEY PLAN

PROJECT

EMUP PROPULSION POWER UPGRADES

DRAWING INFORMATION

DRAWN BY	VERONIKA KURBET
DESIGNED BY	VERONIKA KURBET
CHECKED BY	BRENDAN AVERY
APPROVED BY	LEWIS JUDD

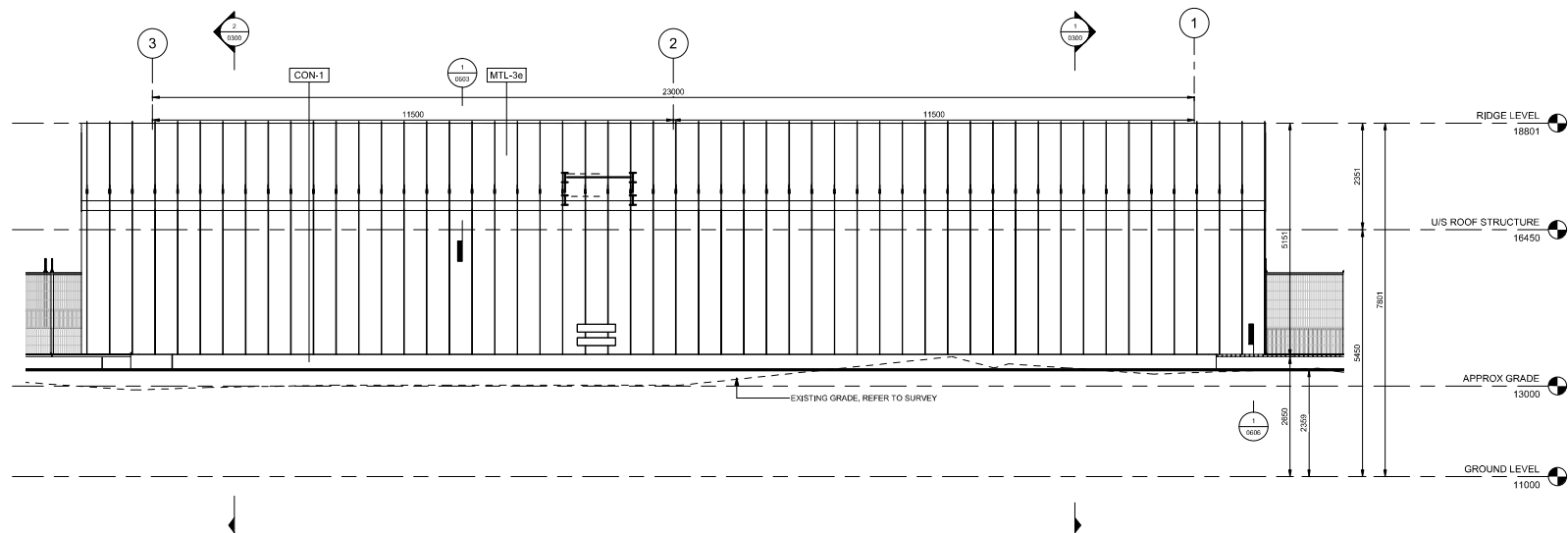
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PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
ELEVATIONS

DRAWING NUMBER

193028-03 -1000- STMCZ- FR- AG- 0201-
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NOT FOR
CONSTRUCTION
2023-07-19



1 NORTH ELEVATION
0102 1:50

ELEVATION MATERIAL LEGEND

MAS-2	FACE BRICK, COLOR TBC
MTL-3e	STANDING SEAM - BLACK, COLOUR TBC
MTL-3d	GUARD W PICKETS - BLACK, COLOUR TBC
CON-1	CONCRETE FACED RIGID INSULATION
PMF-DR	INSULATED METAL DOOR w PRESSED METAL FRAME, PAINTED, COLOUR TBC
FC-1	FENCE, TYPE TBC

ISSUE/REVISION			REFERENCES
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Suite 1300 - 3777 Kingsway Street
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604 662-3555 tel 604 662-7688 fax
www.sncdvalain.com

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SNC-Lavalin
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604 662-3555 tel 604 662-7688 fax
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CONSULTANTS

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1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc.lavalin.com

REGISTRATION KEY PLAN

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CONSTRUCTION
2023-07-19

PROJECT
EMUP
PROPULSION
POWER
UPGRADES
PORT MOODY

DRAWING INFORMATION	
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CONTRACT NUMBER
PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
ELEVATIONS

DRAWING NUMBER
193028-03 -1000- STMCZ- FR- AG- 0202-
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PMF-DR

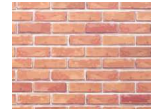
MTL-3d

FC-1

MAS-2

CON-

MAS-2 - FACE BRICK



ELEVATION MATERIAL LEGEND

MAS-2 FACE BRICK, COLOR TBC
MTL-3c STANDING SEAM, BLACK, COLORED TBC

MTL-3d GUARD W PICKETS - BLACK, COLOUR TBC

CON-1	CONCRETE FACED RIGID INSULATION
PME-08	INSULATED METAL DOOR w/PRESSED METAL

EC-1	FENCE TYPE TBC
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10. $\log_2 8 = 3$ 11. $\log_2 16 = 4$ 12. $\log_2 32 = 5$ 13. $\log_2 64 = 6$ 14. $\log_2 128 = 7$

ISSUE/REVISION				REFERENCES
A	2023-07-19		90% SUBMISSION	
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Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

MECHANICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

CONSULTANTS

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

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KEY PLAN

PROJECT

**EMUP
PROPULSION
POWER
UPGRADES**
PORT MOODY

PORT MOODY

DRAWING INFORMATION

DRAWN BY VERONIKA KURBET
DESIGNED BY VERONIKA KURBET
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CONTRACT NUMBER

PROJECT NUMBER	193028 (SNC #675448)
SHEET TITLE	3D VIEW

DRAWING NUMBER

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2023-07-19

2023-07-19



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Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.sncdavalin.com

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SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.sncdavalin.com

CONSULTANTS

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 668 3252 tel
www.francarchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
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REGISTRATION

KEY PLAN

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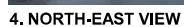
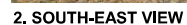
**EMUP
PROPULSION
POWER
UPGRADES**
PORT MOODY

DRAWING INFORMATION	
DRAWN BY	Author
DESIGNED BY	Designer
CHECKED BY	Checker
APPROVED BY	Approver

CONTRACT NUMBER

PROJECT NUMBER	193028 (SNC #675448)
SHEET TITLE	3D VIEW

DRAWING NUMBER
193028-03 -1000- STMCZ- FR- AG- 204-

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SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
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REGISTRATION

KEY PLAN

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CONTRACT NUMBER

EMUP PROPULSION POWER UPGRADES

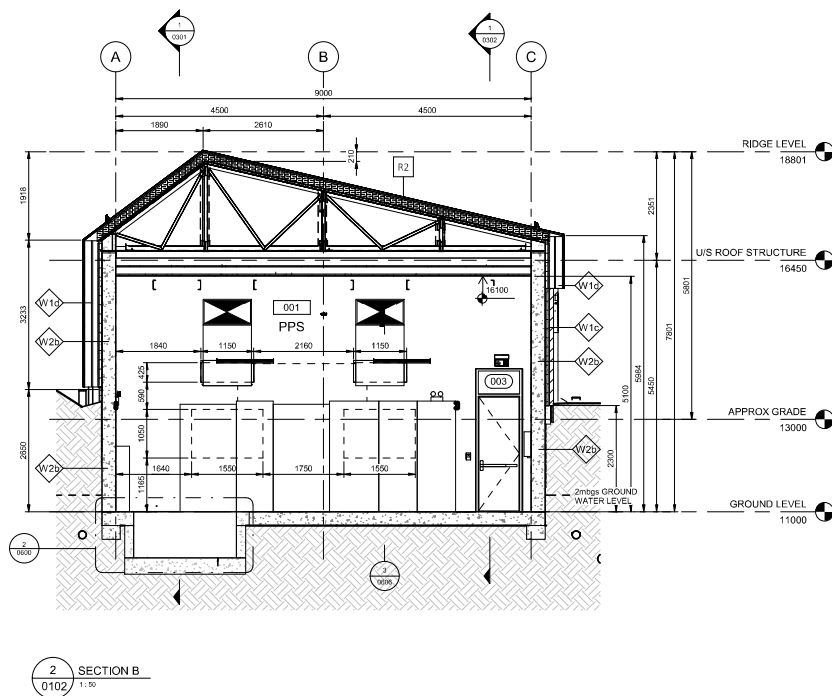
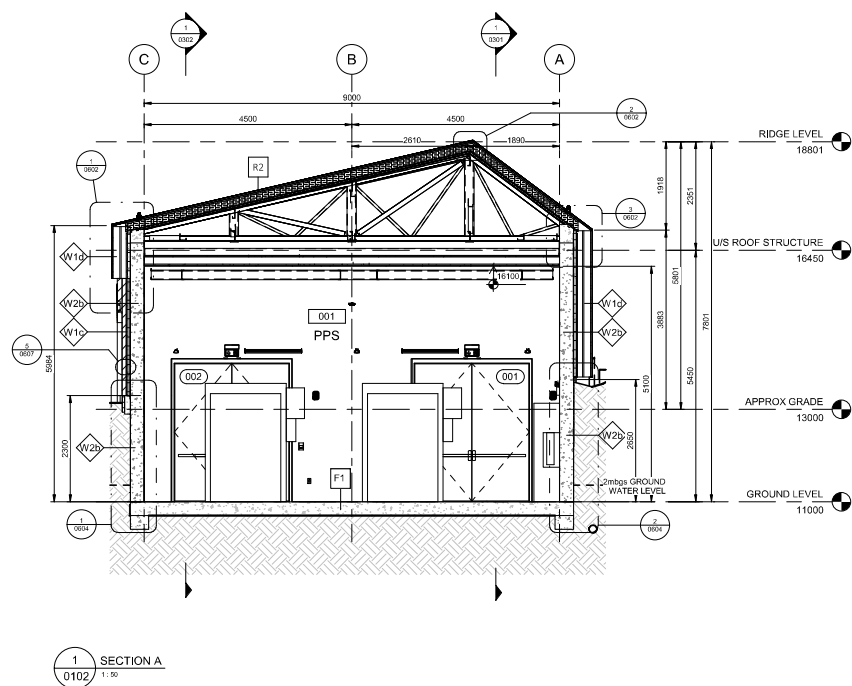
DRAWN BY	Author
DESIGNED BY	Designer
CHECKED BY	Checker
APPROVED BY	Approver

PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
SITE PHOTOS

DRAWING NUMBER

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SNC-Lavalin
Suite 1360 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

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Suite 1360 - 3777 Kingsway Street
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APPROVED BY	LEWIS JUDD

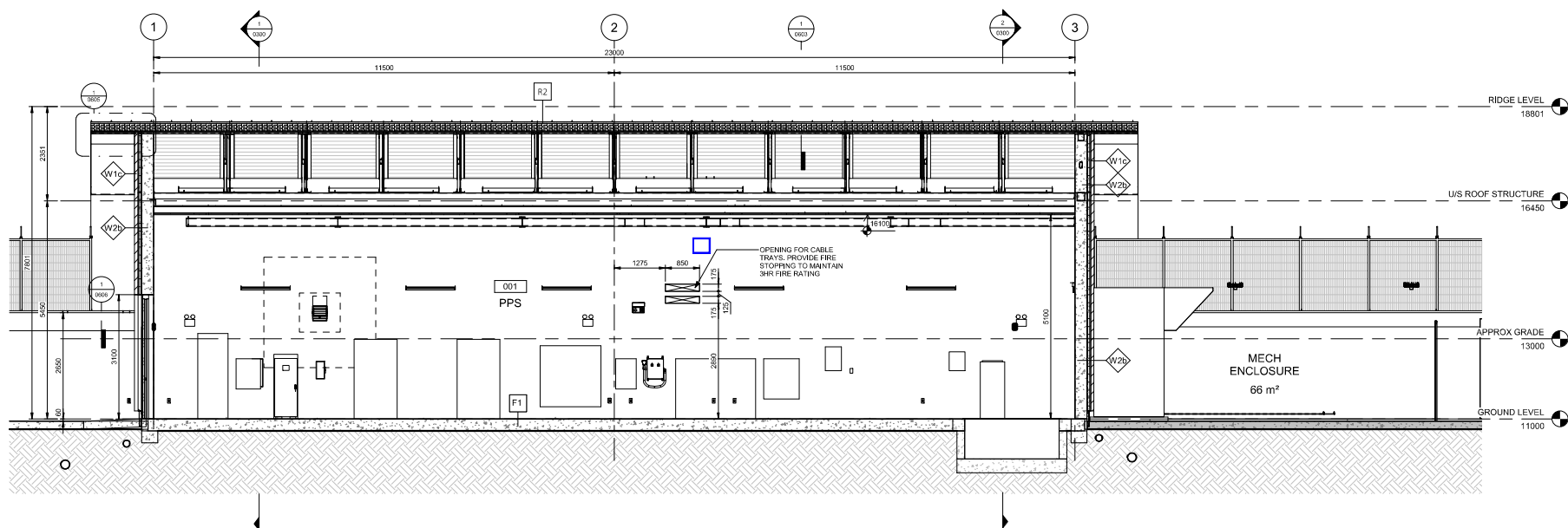
CONTRACT NUMBER

PROJECT NUMBER	193028 (SNC #675448)
SHEET TITLE	SECTIONS

DRAWING NUMBER
193028-03 -1000- STMCZ- FR- AG- 0300-
-B

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CONSTRUCTION
2023-07-19

EMUP PROPULSION POWER UPGRADES



1 SECTION C
0102 1:50

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ELECTRICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 293 3888

MECHANICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

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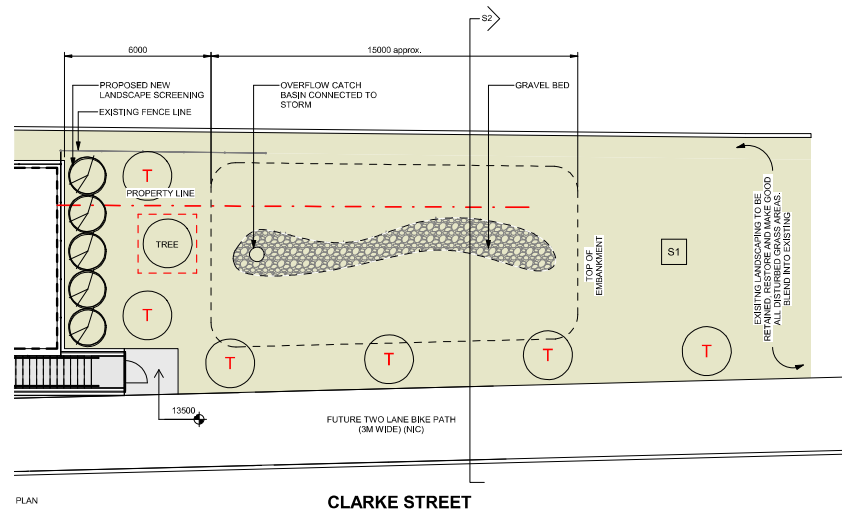
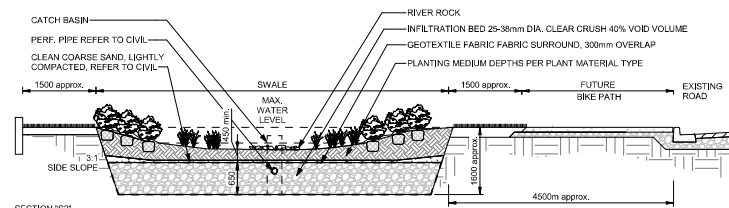
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PROJECT NUMBER	193028 (SNC #675448)
SHEET TITLE	SECTIONS

DRAWING NUMBER

193028-03 -1000- STMCZ- FR- AG- 0301-
-B



5 BIOSWALE EAST FOR
0005 STORMWATER MANAGEMENT
1:100

ISSUE/REVISION						REFERENCES
A	2023-07-19			90% SUBMISSION		
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REGISTRATION KEY PLAN

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CONSTRUCTION

2023-07-19

PROJECT

EMUP PROPULSION POWER UPGRADES

PORT MOODY

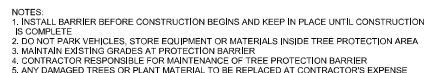
DRAWING INFORMATION	
DRAWN BY	JASONE OLLOQUE
DESIGNED BY	JOSE GONZALEZ
CHECKED BY	BRENDAN AVERY
APPROVED BY	LEWIS JUDD

CONTRACT NUMBER	
PROJECT NUMBER	193028 (SNC #675448)
SHEET TITLE	SWALES

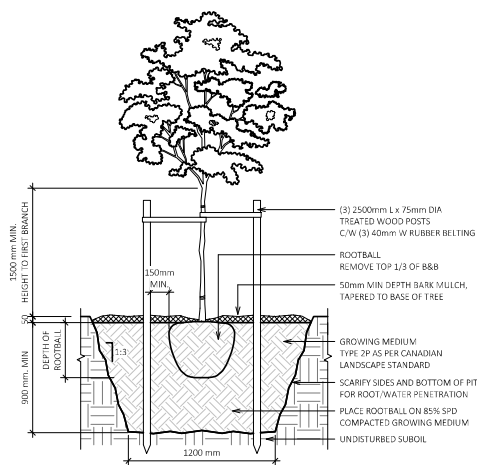
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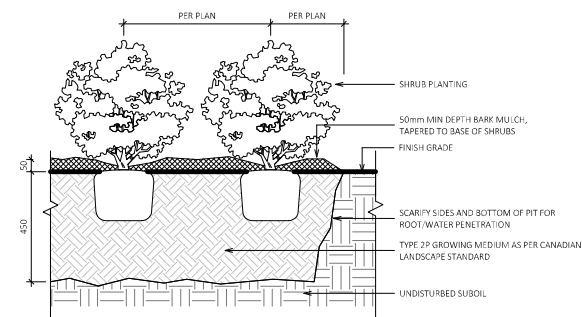
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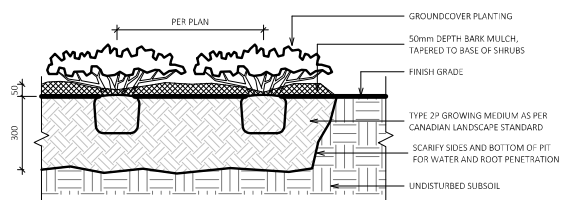
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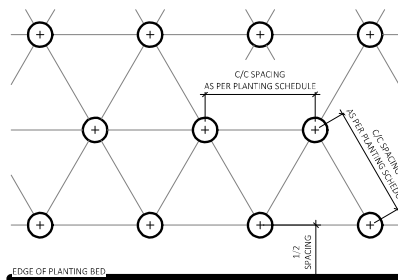
2 TREE PLANTING



3 SHRUB PLANTING



4 GROUND COVER PLANTING



5 PLANTING SPACING PLAN

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CONSULTANTS

ELECTRICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

MECHANICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel | 604 662-7688 fax
www.snc-lavalin.com

CONSULTANTS

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel | 604 662-7688 fax
www.snc-lavalin.com

REGISTRATION

KEY PLAN

PROJECT

DRAWING INFORMATION

DRAWN BY	JASONE OLLOQUI
DESIGNED BY	JOSE GONZALEZ
CHECKED BY	LEWIS JUDD
APPROVED BY	LEWIS JUDD

CONTRACT NUMBER

PROJECT NUMBER
193028 (SNC #675448)
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EMUP PROPULSION POWER UPGRADES

SCHEDULE B

Geotechnical Design Report



SNC • LAVALIN

EMUP Propulsion Power Upgrades Project

Moody Centre Station – 60% Geotechnical Design Report

South Coast British Columbia Transportation Authority

December 19, 2022

SNC-Lavalin Project: 675448

Internal Reference: 193028-03-2000-STMCZ-SL-BJ-0001> Draft Rev. A

Signature Page

Prepared By:

Mohammad Khandan Bakavoli, P.Eng.

Senior Geotechnical Engineer

Geoscience & Materials Practice

Engineering Services Canada

Reviewed By:

Mohsen Nicksiar, PhD, P.Eng.

Geotechnical Operations Manager – Lower Mainland

Geoscience & Materials Practice

Engineering Services Canada

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Signature Page

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- II. Borehole/CPT Logs
- III. NBCC 2020 Seismic Hazard Parameters
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1 Introduction

SNC-Lavalin Inc. (SNC-Lavalin) has been retained by South Coast British Columbia Transportation Authority (SCBCTA, or TransLink) to provide geotechnical design services for the SkyTrain Expo and Millennium Line Propulsion Power Upgrades (EMUP) project. This project consists of upgrades to multiple SkyTrain stations located in Vancouver, Burnaby, Surrey, Coquitlam and Port Moody, British Columbia (BC). The locations and scope of the upgrades are generally variable between the SkyTrain stations that are being improved however, in some cases, a part of the upgrade includes a new Propulsion Power System (PPS) building.

The scope of this 60% design report is to review the geotechnical components of the Moody Centre SkyTrain Station PPS and provide geotechnical recommendations for the design and construction of the PPS elements.

The following tasks were performed as a part of this 60% design report:

- Compile and review existing information pertinent to this project site, including:
 - Available geotechnical reports provided by TransLink and SNC-Lavalin's internal documents for nearby projects, including:
 - EBA Engineering Consultants Ltd., Evergreen Line Rapid Transit Project Combined Moody Centre Station and At-Grade Guideway (Section 340) provided for SNLC-Lavalin Construction (Western) Inc. (SLCW), August 2013.
 - EBA Engineering Consultants Ltd., Evergreen Line Rapid Transit Project At-Grade Guideway Structures (Section 330B) provided for SNLC-Lavalin Construction (Western) Inc. (SLCW), January 2014.
 - Aerial photos, topographic maps, ortho-imagery, utilities mapping and other relevant drawings and datasets; and
 - Published geological maps and technical papers for the general project area.
- Provide design seismic hazard parameters based on the newly published NBCC 2020.
- Provide foundation design parameters and recommendations for select foundation types and construction methodologies.
- Provide general construction recommendations including for site preparation, frost penetration, subgrade preparation, backfilling, site drainage, excavations, dewatering.
- Provide recommendation for the new utility design construction.

Note, the environmental aspect of the project is beyond the scope of this report.

2 Site and Project Description

The Moody Centre SkyTrain Station is situated in the north of Barnet Highway and south of Murray Street, between Williams and Hugh Streets in Port Moody, B.C. The station and surrounding area are shown in Figure 2.1.

Figure 2.1: Moody Centre SkyTrain Station (image obtained from Google Earth)

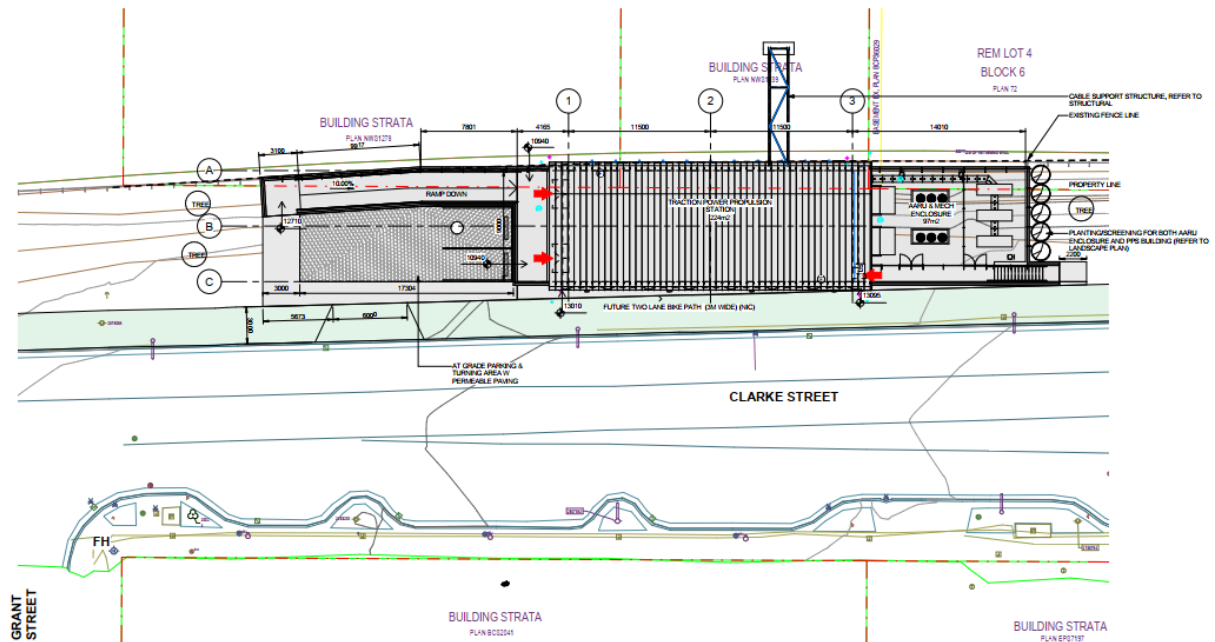


TransLink has proposed that the PPS building at Moody Centre Station to be located north of Clarke Street, between Grant and Moody Streets (See Appendix I, Drawing No. 193028-03 -1000-STMCZ- FR- AG- 0001- A).

The proposed upgrades will include construction of slab foundations to support the PPS building. The development of the PPS site will include the construction of a new paved road, new buried utilities, and parking spaces.

Figure 2.2 shows the proposed layout of the PPS building relative to nearby landmarks.

Figure 2.2: Proposed PPS Building Location. (Reference Drawing 193028-03 -1000- STMCZ- FR- AG- 0100-A)



Preliminary drawings showing some design elements in plan view can be found in the Drawings section at the end of this report. A map showing the locations of historical boreholes can be found in Appendix II.

3 Historical Document Review

3.1 General

As a part of the geotechnical design process, SNC-Lavalin reviewed the available geotechnical reports and as-build drawings provided by TransLink. The following documents were provided to us:

- Available geotechnical reports provided by SNC-Lavalin's internal documents for nearby projects.
- EBA Engineering Consultants Ltd., Evergreen Line Rapid Transit Project Combined Moody Centre Station and At-Grade Guideway (Section 340) provided for SNLC-Lavalin Construction (Western) Inc. (SLCW), August 2013.
- EBA Engineering Consultants Ltd., Evergreen Line Rapid Transit Project At-Grade Guideway Structures (Section 330B) provided for SNLC-Lavalin Construction (Western) Inc. (SLCW), January 2014.
- Published geological maps and technical papers for the general project area.

The geotechnical information found in these historical reports has been compiled and summarised in this section. The below subsurface geology should be considered as a summary only; further information can be found in the individual historical reports (Appendix II).

- Fill: loose to compact, grading from sandy silt to silty sand to gravelly sand and sand with trace of silt, scattered cobbles and boulders, and organics including wood debris. The fill is up to 3.5 m thick in the vicinity of Kyle Creek.
- Debris fan, marine and shoreline deposits (Salish Sediments): These deposits were formed by debris floods and flows sourced from the gullies south of the alignment, but with some interlayering of finer-grained marine and shoreline sediments. The deposits vary in thickness from 2 m to 10 m and generally consist of loose to compact interbeds of sand, silty sand, silt and gravel. Occasional cobbles and organic layers including peat and wood debris are also known to occur in these deposits.
- Marine deposits (Capilano Sediments): These deposits vary in thickness from 3 m to 13 m and generally consist of soft to stiff silt and clay with zones of sandy silt and thin lenses of sand and gravel overlying loose to compact interlayered silt and sand. Shell fragments, wood debris, wood fibers and organic layers are also present.
- Glaciomarine Deposits (Capilano Sediments): These deposits vary in thickness from 5 m to 14 m and generally consist of a compact to very dense assemblage of sand, gravel, silt and some clay with occasional cobbles and boulders. In some areas these deposits have a similar texture to till-like soils but have generally not been overridden by glacial ice, and therefore, tend to be less dense than the underlying Vashon Sediments.
- Till-like deposits (Vashon Sediments): These deposits generally consist of massive, very dense silty sand to sandy silt with variable gravel and occasional cobbles and boulders. Based on local experience and the results of previous site investigations these deposits may also contain lenses/seams of silt or water-bearing sand and gravel. The depth to these deposits ranges from about 15 m to 30 m below the existing ground surface.

Around the footprint of the proposed PPS building (100m from each side), there are four boreholes and seven CPT soundings from EBA 2013 and 2014 reports (See Appendix II). Table 3-1 summarized the depth of selected boreholes, CPTs, lab and In-situ tests performed in each borehole.

Table 3-1: Summary of the Selected Boreholes, Depth, Lab and In-situ Testing

BH/CPT	Year of drilling/CPT sounding	Approximate Distance from the footprint of PPS Building (m)	Depth (m)	Performed Lab Tests	In-situ Testing
BH09-305	2009	At the footprint	33.30	Moisture Content, Index limits, Organic Contents	SPT
BOH/BPT12-S3-525	2012	100	29.10	Moisture Content, Index limits, Organic Contents	BPT
BOH12-03	2012	60	32.00	-	BPT
MR12-S3-524	2012	100	38.20	Moisture Content, Index limits, Cyclic Direct Simple Shear (CDSS)	SPT, Downhole, Field Vane
CP13-619-800	2013	80	18.65	-	-
CP13-619-880	2013	At the footprint	21.40	-	-
CP13-619-900	2013	At the footprint	19.70	-	-
CP13-619-920	2013	At the footprint	17.45	-	-
CP13-619-961	2013	20	20.90	-	-
CP13-619-980	2013	40	23.35	-	-
CP13-620-000	2013	70	18.35	-	-

Based on the above boreholes and CPTs, the general subsoil for the studied area is described in the following Section 4.

4 Subsurface Conditions

4.1 General

As there is no site-specific field investigation for the PPS building at Moody Centre Station, the EBA 2013 and 2014 reports were used for geotechnical subsurface conditions. Soil descriptions discussed in this section are based on the classifications and methodologies that were used in both reports. The closest borehole/CPT to the proposed footprint of the PPS building are BH09-305, CPT619880, CPT619900 and CPT619920, that subsurface conditions are inferred mainly from these borehole and CPT soundings. Additionally, the other nearby boreholes such as BH/BPT12-S3-525, BH12-03, and MR12-S3-524 were also considered in the current project. The location of the boreholes and the most relevant borehole logs are provided in Appendix II.

4.2 Surface Fills

The above-referenced reports include the borehole information obtained in 2009 in which the Fill layer was described as silty sand, some gravel with cobbles and organic material. However, since that time, the existing buildings at the location of BH09-305 were demolished to provide room for the Evergreen Line construction, and a green area was built at that location (as per historical imagery of the Google Earth). Hence, the thickness of the surface Fill is expected to be changed since 2009. The approximate thickness of the Fill layer was considered to be 2 m to 2.5 m.

4.3 Upper Silty Sand to Sandy Silt

Below the Fill, the material is generally loose to compact silty Sand to Sand, contains cobbles with some silt and gravel with increasing fine content with depth becoming silty Sand to sandy Silt. The thickness of this layer is about 8 m.

4.4 Silty Clay to Clayey Silt

There is a 7 m layer of soft to firm silty Clay to clayey Silt, which contains lenses of sandy Silt and silty Sand. The Atterberg limits were measured on two samples and plasticity index of 18 and 28 were reported.

4.5 Lower Silty Sand to Sandy Silt

Below the silty Clay to clayey Silt layer, there is an approximately 2m thickness layer which mainly consists of loose to compact silty Sand to sandy Silt layer, trace gravel.

4.6 Sand and Gravel

This layer generally consists of compact to very dense Sand and Gravel, trace cobbles. The thickness of the layer is approximately 15 m.

4.7 Till-Like

The very dense sandy Silt, some gravel with trace cobbles encountered at depth of 32 m, inferred as till-like soil. The Till-like material in the studied area is generally silty Sand to sandy Silt with variable gravel, cobbles and boulders content and may contain water-bearing lenses of sand and gravel.

4.8 Groundwater

The groundwater table was located within the subsurface fill layer at approximately 1.5 metre below ground surface (mbgs) to 3.5 mbgs. In BH09-305, the groundwater table was not recorded but a wet sample collected at depth of 2 m. In the current design, water table considered to be at depth of 2 m. It is considered likely that the water table on site is influenced by the nearby creek, and vary seasonally with snow melt and increased precipitation.

5 Seismic Considerations

NBCC 2020 has adopted the use of foundation factors that are dependent on local site soils condition, shaking level, and site period for structural design considerations. The effects of local site conditions are characterized based on the average shear wave velocity or SPT N value of the soils or rock within the upper 30 m of the project location.

Based on the high-level liquefaction screening, the sand and silt underlying the site are susceptible to liquefaction during major earthquake events (1:2475-year return periods). Since these layers are potentially liquefiable, the site is classified as “Site Class F” in accordance with NBCC 2020. However, since the fundamental period of the proposed structure is less than 0.5 second (according to the structural team it is equal to 0.17 seconds) values of “Site Class D” can be used for the design.

SNC-Lavalin has obtained site specific hazard parameters for the investigation area from the National Resources Canada website¹. A copy of the results for the area can be found in Appendix III.

The predicted peak ground horizontal accelerations and spectral accelerations for the different earthquake return period at the project site are provided in Table 5-1 based on NBCC 2020 seismic hazard values.

Table 5-1: Summary of 2020 NBCC Site Class D Seismic Hazard Parameters

Location	Seismic Return Period (yr)	PGA (g)	S _a (0.2)	S _a (0.5)	S _a (1.0)	S _a (2.0)
Moody Centre PPS Building	2475	0.468	1.02	1.06	0.753	0.460

5.1 Liquefaction Analysis

Since there is no site-specific response analysis required, the simplified total-stress based procedure was incorporated using the site-specific Peak Ground Acceleration (PGA) and earthquake magnitude. The liquefaction assessments were completed using available SPT blow counts from BH09-305. The following subsections describe methodology and summarize results of liquefaction analyses.

5.1.1 Methodology

The potential for seismic liquefaction triggering for the non-cohesive soils have been assessed using Boulanger and Idriss (2014) and Idriss and Boulanger (2008) SPT-based methods using the commercially available software CLiq. 3.3.2.9 developed by GeoLogismiki.

In the current assessment, as per Section 4, the upper soil layers are generally silty Sand, sandy Silt to Sand, which are potentially liquefiable. The factor of safety against liquefaction/cyclic softening at each depth was calculated using the following relation:

$$FS = \frac{CRR_{7.5} MSF}{CSR} K_{\alpha} K_{\sigma}$$

Where MSF=magnitude scaling factor

K_α = ground slope correction factor

K_σ = overburden stress correction factor

¹ Natural Resources Canada (2020). *Seismic Hazard Calculator*. Retrieved from [2020 National Building Code of Canada Seismic Hazard Tool \(nrcan.gc.ca\)](https://www.nrcan.gc.ca/2020-national-building-code-of-canada-seismic-hazard-tool)

A factor of safety less than 1.0 Indicates a high liquefaction potential. However, computing a factor of safety against soil liquefaction is not as important as carrying out the analysis with a consistent methodology and taking into consideration the anticipated variability of the in-situ soil resistance and variables that define the earthquake ground motions.

The Cyclic Resistance Ratio (CRR) of layers to cyclic loading was estimated from the SPT measurements, specifically from BH09-305. In addition, the CSR was modified for earthquake magnitude, overburden pressure, and shear stress level in static condition.

It is worth noting that, at the time of this report and as per SNC-Lavalin correspondence with National Resources Canada (dated July 19, 2022), the 6th generation of seismic hazard deaggregation analyses are not yet released, so that for the purpose of the current liquefaction assessment, the 5th generation deaggregation plots were used. As per the 5th generation hazard model provided by Geological Survey of Canada (GSC), the moment magnitude (M_w) for the site is considered 7.1 for crustal and Inslab earthquake suites and 8.4 for interface earthquakes.

5.1.1.1 Cyclic Stress Ratio

For sand, the CRR for liquefaction assessment is calculated using both Boulanger and Idriss (2014) and Idriss and Boulanger (2008) methods by normalizing the SPT blow counts to overburden pressure, and adjustment based on the fines content in order to represent the equivalent clean sand ($N_{160,cs}$).

5.1.1.2 Extent of Potential Liquefiable Layers

Liquefaction triggering assessment was completed according to the methodology described in Section 5.1.1. Table 5-2 demonstrates the extent of liquefaction.

Liquefaction triggering assessment results and the profile of FOS are presented schematically in Appendix IV.

Table 5-2: Extent of Liquefaction

	Extent of Liquefaction Elevation (m)	
	Idriss & Boulanger (2008)	Boulanger & Idriss (2014)
Simplified Procedure, 2475yr	+8.8 to +8.1	+8.8 to +8.1
	+7.4 to +2.0	+7.4 to +2.0

5.1.1.3 Post-Liquefaction Settlement and Lateral Ground Movement

Dissipation of the excess pore water pressure developed during and after strong ground motions would cause settlement of the ground. Semi-empirical method developed and proposed by Idriss and Boulanger (2008) and Wu et al. (2002) were used to estimate the post-liquefaction settlement. The proposed methodology is very similar to the one proposed by Yoshimine et al (2006).

Post-liquefaction free-field settlement was estimated for the generalized soil profile and presented in Table 5-3. The anticipated differential settlement should be expected to be 50% of the total.

Soil liquefaction will result in lateral ground displacements. The lateral displacements are caused by strain softening of liquefied soil and earthquake-induced cyclic inertia forces. The ground lateral displacements, for a screening level assessment, can be estimated using the simplified empirical method proposed by Youd et al. (2002). In addition, liquefaction may induce large lateral ground spreading displacements, maybe in the order of several metres, however the liquefaction-induced permanent displacements are

difficult to quantify with confidence. As the SkyTrain At-grade guideway is supported by Deep Soil Mixing (DSM) wall (EBA 2014 report), and the footprint of the PPS building located in south side of the guideway, it is anticipated that there is no post-liquefaction later displacement along the North-South direction. However, there would be lateral displacement in the East-West direction (the ground slope is around 1.5%) in case of liquefaction. The estimated post-liquefaction and lateral displacement are presented in Table 5-3.

Table 5-3: Post-liquefaction Reconsolidation Settlement and Lateral Displacement

	Post Liquefaction Settlement (mm)	Lateral Displacement (m)
2475yr Earthquake	200-300 mm	0.05-0.3m

6 Geotechnical Recommendations

6.1 General Construction Recommendations

6.1.1 Site Preparation

Preparation of the site for construction is anticipated to consist of some or all of the following items:

- Removal and replacement of the existing green area;
- Stripping of the PPS footprint, including the removal of organic materials, such as topsoil, grass, stumps, and garbage from the area adjacent to the existing SkyTrain retaining wall/fence; and
- Since there is no site-specific field investigation, BH09-305 (EBA 2013 and 2014 report) was used. Based on this borehole the fill and topsoil material (to the EL +9.00m) should be removed and backfilled with engineering fill.

6.1.2 Frost Protection

Frost penetration will impact foundations due to the expansion of pore water in the soils. Based on historical temperature data for the Port Moody area, the estimated maximum frost penetration depth is approximately 450 mm. The effect of snow cover, a higher ground water table, vehicle traffic, and higher moisture contents all affect the depth of frost penetration.

Any concrete constructions such as footings should be designed so that they are founded on a freely draining material with their base at least 450 mm below the surrounding ground cover to reduce the risk of frost induced stresses.

6.1.3 Subgrade Preparation

Subgrade preparations should consist of the removal of organics and loose or saturated surficial soils. The exposed native deposits should be inspected for organic materials, saturated spots, and material consistency by a geotechnical engineer upon exposure through excavation. Any soft spots, or saturated zones shall be removed and sub-excavated by 200 mm before being replaced with an approved compacted engineered granular fill. When the excavation floor is deemed acceptable, a proof roll test shall be performed to verify its competence. This proof roll shall be witnessed and verified by SNC-Lavalin's geotechnical engineer.

In the absence of site-specific field investigation, the BH09-305 (EBA 2013 and 2014 report) was used. In this borehole, the depth of fill and topsoil was around 2 m to 2.5 m. Due to variability of the existing fill soils, it is anticipated that this material should be removed from the project footprint and replaced with engineered fill material. Besides, as per Section 5.1, the soil below EL. +8.8 m is deemed to be liquefiable, hence to minimize the differential settlement the subgrade should be reinforced with geogrid.

Furthermore, subgrade preparation prior to structural elements installation should include excavation to design elevation, sub-excavation of boulders, any isolated locations that contain organic fill, protruding construction debris, wood waste or other oversaturated soils, and soft or yielding soils, and replacement of this unsuitable soil with engineered structural fill prepared as per Section 6.1.4.

Subgrade soils should be reviewed by SNC-Lavalin's geotechnical engineer prior to compaction or backfilling.

6.1.4 Engineered Fill

Engineered fill material should be free from any frozen soil, organic materials, and contamination. All placed engineered fill material should be compacted to 100% of Standard Proctor Maximum Dry Density (SPMDD) at moisture contents within $\pm 2\%$ of optimum. Individual layers of placed loose material (lifts) shall not exceed 300 mm in thickness.

Qualified geotechnical personnel should monitor the quality and placement of all fill soils. The compaction of the fill should be routinely monitored by field density testing. Test results should be submitted to the geotechnical engineer for review.

6.1.5 Building Drainage and Dewatering

Groundwater, where encountered, will likely be found just above the level of the nearby creek and within the fill soils. If dewatering during construction is necessary (either from seepage or during rainfall events), construction dewatering procedures should include routing of water to a suitable outlet. Within excavations, it is recommended that dewatering include directing water to sump areas, from where it may be pumped to a suitable outlet. These sumps and pumps (along with a power source) are to be provided by the contractor and be readily available to prevent water from pooling in the excavations. Both surface water run-off and groundwater should be managed in this way while following local guidelines and bylaws on construction water discharge.

The foundation drainage system for the PPS building at Moody Centre should consist of a minimum 150 mm diameter perforated rigid pipe placed (perforations downwards) along the exterior base of the foundation perimeter and tight-lined to a storm drain system or another suitable outlet. The pipe should be bedded on at least 100 mm of 19 mm coarse clear crushed gravel and be backfilled with minimum 200 mm of a similar drainage gravel wrapped in nonwoven geotextile with a minimum of 150 mm of overlap.

The site should also be graded so that surface water run-off is directed away from the PPS building and towards catch-basins, which are connected to a storm sewer drain or another suitable outlet.

6.1.6 Construction Excavations

Excavations that remain open for less than seven days are considered to be temporary and are likely to be the dominant excavation type encountered during construction. Although not anticipated to be deeper than 1 m, temporary excavation slopes shall be no steeper than 1.5(H):1(V) for temporary shallow (less than 1.8 m) excavations in the native, compact, fill. Water should not be allowed to flow on the side slopes of the excavation or pond at the base of the excavation. Relevant British Columbia (WorkSafeBC) regulations should be adhered to for minimum requirements for temporary excavations. Note, excavations in excess of 1.2 m depth should be reviewed by the geotechnical engineer as required by WorkSafeBC.

6.1.7 Utility Construction and Bedding Requirements

It is understood that new catch-basins and associated sewer lines are to be constructed as a part of the PPS building at Moody Centre.

Newly constructed sewer pipes should be founded on 200 mm of compacted, clean, bedding sand. This same bedding sand should be placed and compacted around the pipe on all sides to 95 % SPMDD using a vibrating plate tamper.

Utility trenches should be excavated to below the frost penetration depth described previously and using the same excavation recommendations presented above. Where sloped excavations are not desired to facilitate utility installation, trench boxes or an alternative, compliant, engineered shoring system can be used to limit the footprint of utility excavations, subject to approval of the geotechnical engineer. Support excavation may be near vertical provided a shoring system is properly installed prior to worker entry.

6.1.8 Slab-on-Grade

Prior to slab-on-grade construction, the subgrade should be prepared and reviewed as described in Section 6.1.3. Slabs-on-grade should be constructed on a minimum 150 mm thick layer of 19 mm clear crushed gravel to act as a capillary break. It is also recommended that a vapor barrier consisting of polyethylene sheeting be placed between the slab and the bedding material to limit the migration of moisture.

Grade re-instatement below the slab should be conducted using engineered fill as described in Section 6.1.4. In-place density testing should be completed on the under-slab fills to confirm that the fill placed below the slab has been compacted to a minimum of 100% of SPMDD. The Geotechnical Engineer should review the subgrade prior to both concrete and fill placement.

A geotextile liner should be placed between the clear crushed gravel fills and silt or clay soils (where encountered) to act as a particle break and improve the long-term performance of the subgrade. This geotextile should consist of a Nilex 4551 or approved equivalent and rolls of this geotextile should overlap at least 200 mm to ensure coverage during the placement of backfill soils.

6.2 Foundation Design Recommendations

6.2.1 Design Recommendations

Based on the anticipated PPS elements and the presence of near surface competent soils (having engineered fill to the elevation of +9.0 m), shallow raft foundations are the recommended foundation type to be utilized at the Moody Centre PPS location. This section provides recommendations for the design and construction of these footings.

Footings placed on the engineered fill, if placed as per Sections 6.1.1, 6.1.3 and 6.1.4 may be designed using the following:

- Raft footings may be designed using Serviceability Limit State (SLS) bearing resistance of 150 kPa. Factored ultimate bearing resistance (ULS) may be taken as 225 kPa.

The subgrade modulus for engineering fill can be considered as 10 MN/m³.

Footings/slabs should not be constructed during freezing conditions without appropriate heating and hoarding procedures being followed. As mentioned in Section 6.1.3, the engineered fill below the raft foundation should be reinforced with geogrids (Nilex Biaxial BX1200 or approved equivalent product) to minimize the effect of post-liquefaction differential settlement.

6.2.2 Long-term Consolidation Settlement

As stated in Section 4, there is a 7 m-thick layer of soft to firm silty Clay to clayey Silt, which contains lenses of sandy Silt and silty Sand with the thickness of 25 mm to 50 mm. These fine-grained materials are considered to be potentially compressible, and under sustained loading there may experience long-term consolidation settlement.

The long-term consolidation settlement was estimated using Settle3 (Rocscience, Ver. 5.007). Considering a sustained uniform load of 45 kPa was applied to the raft footing (Correspondence with structural engineer [October 31, 2022]), the settlement was estimated to be in the order of 120 mm to 150 mm. The long-term settlement will be decreased to 50 mm if the minimum distance within 3 m from footprint of the structure.

It is worth noting that in the absence of consolidation tests, the parameters for calculations were estimated using empirical correlations.

6.3 Pavement Recommendations

According to the architectural design team, the preliminary plans for the Moody Centre PPS site call for the addition of a Permeable Architectural Precast Concrete Blocks (PACB) for the access roads and parking areas. Based on the results of the field investigation, SNC-Lavalin has developed a road pavement structure using PACB system that is designed to support the expected traffic loads for the project area. The pavement details are provided in Table 6-1.

Table 6-1: Pavement Structure Recommendations

Pavement Element	Pavement	Bedding Layer/Road Base Fill	Sub-Base Fill
Composition	Architectural Precast Heavy Duty Concrete Blocks	19 mm MINUS with no fines	75 mm MINUS
Thickness (mm)	150 mm	100 mm	450 mm

An excavation depth of approximately 700 mm is expected in order to remove the existing mixed fills (though final designed site grading may vary the ultimate excavation depth required). Should excavations encounter contaminated soil, soil that is saturated or overly soft, hog fuel or other debris-heavy fills, some over-excavations may be required to remove these unsuitable materials.

It is recommended that a layer of biaxial geogrid to be placed below the sub-base fill layer along the access road and parking areas to control potential differential settlement due to the presence of compressible subsurface soil. The geogrid should be protected from any potential damages during construction by using either geotextile layers or smaller particle materials such as 19 mm minus material with minimum 100 mm thickness.

Gradation of the bedding layer material should be in accordance with AASHTO #57 and should be free of fine content. As with foundation fills, all fill materials are to be compacted to 100% of SPMDD and be within $\pm 2\%$ of optimum moisture content as determined through Proctor laboratory testing. Loose lift thickness is not to exceed 300 mm in thickness. Some existing fills (if deemed suitable by the on-site geotechnical engineer) may be reused below the designed pavement sections as below pavement fill, however, it is expected that the existing fill will not be suitable for use as “other Grade Fill” and will be removed or otherwise recycled.

Site grading and catch-basin (CB) coverage should be designed to ensure proper drainage of the gravel road surface and to prevent pooling. Service connections and pipe diameters should be designed to provide efficient drainage, and oil/water separators may be added as needed.

7 Construction Review

The preceding section outlines recommendations for the design and construction of the proposed development. Review of certain aspects of construction are required to satisfy the requirements of the BC Building Codes and Engineers and Geoscience BC (EGBC) requirements and to document that the recommendations of the geotechnical report are followed.

It is the contractor's responsibility to contact SNC-Lavalin a minimum of 48 hours in advance to notify us that a field review is required. In summary, geotechnical engineer field reviews are required for the following aspects of the work but not limited to:

- Review of site preparation;
- Review of grading, drainage system installation, Quality Assurance on structural grade fill placement, compaction testing, and road/parking construction;
- Review of engineered fill placement and compaction testing, including density testing of any engineered fill placed under slabs-on-grade and footings;
- Review of foundation subgrade prior to footing construction;
- Review of foundation backfill material and compaction; and
- Review of excavations in excess of 1.2 m depth as required by WorkSafeBC.

Since construction will happen in proximity of existing elevated guideway structures and existing utility lines, a detailed construction impact assessment, including a geotechnical instrumentation and monitoring plan, is required prior to the construction commencement.

A construction support scope and cost estimate can be provided in later stages of the project.

8 Closure

Please be advised that the contents of this report are based on information provided to us and results of field and laboratory testing performed by SNC-Lavalin, and our understanding of the proposed development as described in this report. If the development plans change, or if during construction the subsurface soil conditions are noted to be different than those described in this report, SNC-Lavalin should be notified promptly and the recommendations on the geotechnical aspects of the proposed development reviewed and adjusted accordingly. This report assumes that SNC-Lavalin will complete field reviews during construction.

SNC-Lavalin trusts that this report meets your expectations and requirements. Please do not hesitate to contact the signees of this report with any questions, comments or feedback concerning the content or presentation of this report.

9 Notice to Reader

This report has been prepared and the work referred to in this report has been undertaken by SNC-Lavalin Inc. (SNC-Lavalin) for the exclusive use of TransLink, who has been party to the development of the scope of work and understands its limitations. The methodology, findings, conclusions, and recommendations in this report are based solely upon the scope of work and subject to the time and budgetary considerations described in the proposal and/or contract pursuant to which this report was issued. Any use, reliance on, or decision made by a third party based on this report is the sole responsibility of such third party. SNC-Lavalin accepts no liability or responsibility for any damages that may be suffered or incurred by any third party as a result of the use of, reliance on, or any decision made based on this report.

The findings, conclusions, and recommendations in this report (i) have been developed in a manner consistent with the level of skill normally exercised by professionals currently practicing under similar conditions in the area, and (ii) reflect SNC-Lavalin's best judgment based on information available at the time of preparation of this report. No other warranties, either expressed or implied, are made as to the professional services provided under the terms of our original contract and included in this report. The findings and conclusions contained in this report are valid only as of the date of this report and may be based, in part, upon information provided by others. If any of the information is inaccurate, new information is discovered, site conditions change, or standards are amended, modifications to this report may be necessary. The results of this assessment should in no way be construed as a warranty that the subject site is free from any and all environmental impact.

Any soil and rock descriptions in this report and associated logs have been made with the intent of providing general information on the subsurface conditions of the site. This information should not be used as geotechnical data for any purpose unless specifically addressed in the text of this report. Groundwater conditions described in this report refer only to those observed at the location and time of observation noted in the report.

This report must be read as a whole, as sections taken out of context may be misleading. If discrepancies occur between the preliminary (draft) and final version of this report, it is the final version that takes precedence. Nothing in this report is intended to constitute or provide a legal opinion.

The contents of this report are confidential and proprietary. Other than by TransLink, copying or distribution of this report or use of or reliance on the information contained herein, in whole or in part, is not permitted without the express written permission of TransLink and SNC-Lavalin.

Appendix I

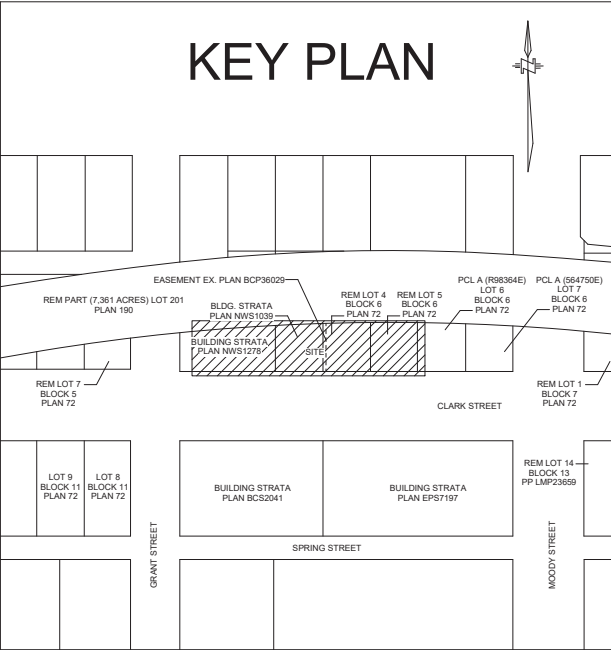
Drawings



Expo and Millennium Upgrade Project (EMUP) Propulsion Power Upgrades

ARCHITECTURAL PORT MOODY

60% SUBMISSION
23/11/22



DRAWING LIST			
Sheet Number	Sheet Name	Sheet Issue Date	Current Revision
0001	COVER SHEET	23/11/22	A
0004	GENERAL NOTES	23/11/22	A
0005	CODE COMPLIANCE	23/11/22	A
0100	OVERPLAN	23/11/22	A
0102	GROUND FLOOR PLAN	23/11/22	A
0103	GROUND FLOOR PLAN - RAMP	23/11/22	A
0104	GROUND FLOOR PLAN - ENCLOSURE	23/11/22	A
0105	ROOF PLAN	23/11/22	A
0200	ELEVATIONS	23/11/22	A
0201	ELEVATIONS	23/11/22	A
0202	ELEVATIONS	23/11/22	A
0300	SECTIONS	23/11/22	A
0301	SECTIONS	23/11/22	A
0302	SECTIONS	23/11/22	A
0400	FOUNDATION DETAILS	23/11/22	A
0401	FOUNDATION DETAILS	23/11/22	A
0402	ROOF & WALL DETAILS	23/11/22	A
0403	WALL & FOUNDATION DETAILS	23/11/22	A
0404	WALL DETAILS	23/11/22	A
0405	WALL DETAILS	23/11/22	A
0406	MISC DETAILS	23/11/22	A

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ARCHITECTURE



PROVINCE CONTRACT No.	SUB-CONSULTANT No.
193028	675448
DRAWING No.	193028-03 -1000- STMCZ- FR- AG- 0001- A
	A

PROJECT NAME: EXPO AND MILLENNIUM UPGRADE PROJECT (EMUP) PROPULSION POWER UPGRADES
PORT MOODY

MAJOR OCCUPANCY: GROUP F, DIVISION 2
CONSTRUCTION TYPE: NON-COMBUSTIBLE
BUILDING HEIGHT: 1 STOREY (7.0m)
BUILDING AREA: 224 sq.m
STREETS FACING: 1
SPRINKLER SYSTEM: N/A
FIRE ALARM SYSTEM: TBC

REFERENCE CODES: (REFER TO CODE REPORT FOR FULL LIST OF APPLICABLE CODES)
BCBC - BRITISH COLUMBIA BUILDING CODE 2018
ASHRAE/IES 90.1 2016 EDITION
TRANSLINK BUILDING CODE CRITERIA 2019, FIXED GUIDEWAY RAPID TRANSIT SYSTEMS (TBCC)

2. THE GENERAL NOTES ARE TO BE READ IN CONJUNCTION WITH THE PROJECT SPECIFICATIONS AND ALL OTHER DISCIPLINES' DOCUMENTS. DO NOT SCALE FROM THE DRAWINGS. USE THE INDICATED DIMENSIONS. CONSULT WITH THE ARCHITECT FOR ANY UNCLEAR DIMENSION.
3. ALL DIMENSIONS ARE METRIC U.N.I.T.S.
4. ALL DIMENSIONS SHALL BE VERIFIED AND COORDINATED ON SITE PRIOR TO PROCEEDING WITH CONSTRUCTION.
5. ARCHITECTURAL DOCUMENTS GOVERN OVER STRUCTURAL FOR LAYOUT OF ALL COMPONENTS. STRUCTURAL DOCUMENTS SHALL GOVERN FOR MEMBER AND COMPONENT SIZES, AND REINFORCEMENT REQUIREMENTS.
6. EXCEPT WHERE SHOWN OTHERWISE, ALL DIMENSIONS SHALL BE TO THE FACE OF THE RESPECTIVE DIMENSIONS 15 AND 16, AND IS IN ACCORDANCE WITH THE APPLICABLE REGULATIONS.
7. THE PROJECT SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE 2018 BRITISH COLUMBIA BUILDING CODE AND ALL BUILDING REGULATIONS (SEE BUILDING CODE SUMMARY FOR APPLICABLE BYLAWS).
8. THE ARCHITECT SHALL BE NOTIFIED IN WRITING OF ANY DISCREPANCIES AND REQUEST CLARIFICATION IN WRITING ON INTERPRETATION OF ALL ASPECTS OF THE PROJECT.
9. WHERE EXISTING COMPONENTS ARE AFFECTED OR DAMAGED BY WORK OF THIS CONTRACT, ALL SUCH AREAS ARE TO BE MADE GOOD.
10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS IN ACCORDANCE WITH THE APPLICABLE REGULATIONS.
11. PROVIDE CONTINUOUS SEALS AT JOINTS OF WALLS, CEILINGS, PARTITIONS, PLenums AND DOORS TO ENSURE CONTINUITY OF FIRE SEPARATIONS AND EFFICIENCY OF AIR PRESSURIZATION WHERE REQUIRED.
12. ALL MATERIALS AND CONSTRUCTION METHODS AND COMPONENTS ARE NOT ACCEPTABLE IN PUBLIC AREAS.
13. HEADROOM IS CRITICAL. ALL SERVICES ON THE CEILINGS AND MUST BE INSTALLED THOUGH THE CEILINGS AND MUST FOLLOW THE LAYOUTS SHOWN. NOTIFY THE ARCHITECT IMMEDIATELY OF ANY CONFLICTS TO THE LAYOUTS SHOWN.
14. IF APPLICABLE SPALLS SHALL BE FINISHED WITH A FINISH TO MATCH EXISTING CONCRETE AND ALL FINISHES SHALL BE DRAWN AT ALL LEVELS.
15. ALL WORK PERFORMED SHALL COMPLY WITH THE CONSTRUCTION DOCUMENTS, DRAWINGS AND SPECIFICATIONS, INCLUDING THOSE OF THE APPLICABLE REGULATIONS.
16. THE SPECIFICATIONS ARE BOUND SEPARATELY IN BOOKS AND ARE PART OF THE CONSTRUCTION DOCUMENTS.
17. THE ORGANIZATION OF THE DRAWINGS AND SPECIFICATIONS IS NOT INTENDED TO CONTROL HOW THE WORK SHALL BE DIVIDED AMONG THE CONTRACTOR'S SUBS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE SEQUENCING OF THE WORK AND THE EXTENT OF WORK PERFORMED BY EACH TRADE.
18. THE CONTRACTOR SHALL BE RESPONSIBLE FOR EXISTING OR BETWEEN ANY PART(S) OF THE CONSTRUCTION DOCUMENTS, OR BETWEEN THE CONSTRUCTION DOCUMENTS AND ANY EITHER APPLICABLE INDUSTRY STANDARD(S) OR APPLICABLE CODE(S), ORDINANCE(S) OR OTHER RELATED LEGAL REQUIREMENTS. THE MOST STRINGENT REQUIREMENTS SHALL APPLY. INFORMATION SHOWN IN DETAILS AND SMALLER SCALE DRAWINGS TAKES PRECEDENCE OVER INFORMATION SHOWN IN LARGER SCALE DRAWINGS.
19. NO DEVIATION FROM THE CONSTRUCTION DOCUMENTS (DRAWINGS OR SPECIFICATIONS) SHALL BE MADE WITHOUT WRITTEN APPROVAL OF THE ARCHITECT.

W1c

R2

FLOOR ASSEMBLIES

EXTERIOR FENCING

W3	GRAPHIC	FENCING - FENCE, TYPE TBC	NOTES
			
FRR			
STC			

AARU AUTOMATIC ASSURED RECEPTIVITY UNITS $(0$

GRIDLINE (VERTICAL GRIDLINES ARE NUMBERS,
HORIZONTAL GRIDLINES ARE LETTERS)



1. ALL EXTERIOR DOORS ARE TO CONFORM TO THE ASTM F476-84 STANDARD.
2. DOOR HARDWARE IS TO BE REKEYED AFTER CONSTRUCTION
3. ALL DOORS WITH EXTERIOR GLASS SHALL BE GLAZED TO CAN/CSG-92.5-M RATED DOORS AND FRAMES SHALL HAVE ULC LABEL
4. ALL DOORS TO BE SUPPLIED WITH TYPICAL DOOR HARDWARE AND SPECIFIC APPLICATION HARDWARE LISTED IN THE LEGEND.
5. ALL HARDWARE TO BE COMMERCE
6. HARDWARE FINISH TO BE SATIN CHROME, SATIN STAINLESS STEEL, SATIN CHROMIUM OR CLEAR ANODIZED ALUMINUM.
7. ALL DOORS WITHIN A FIRE RATED WALL, OR IN A ROOM CONTAINING HAZARDOUS MATERIALS TO HAVE SELF CLOSING MECHANISM AND SMOKE SEALS
8. ALL DOORS WITHIN A FIRE SEPARATION TO HAVE AN ALUMINUM THRESHOLD IF THE FLOORING MATERIAL IS COMBUSTIBLE

ELEVATION MATERIAL LEGEND

PSF	INSULATED METAL FRAME	MA5-2	GUARD BOLT, COLOR: CYCLE
INSM	PAINTED METAL DOOR	MTL-37	GUARD WITH PICKETS - BLACK, COLOR: CYCLE
PT-2	PAINTED - COLOR: BROWN	MTL-39	STANDING BOLT - BLACK, COLOR: CYCLE
CL	LOCKET HARDWARE	MTL-36	GUARD W PICKETS - BLACK, COLOR: CYCLE
LS	ELECTRONIC MORTISE LOCK	CON-1	CONCRETE FACED ROUGH INSULATION
CS	SMOKE SEAL	PAC-DR	INSULATED METAL FRAME, WEATHER METAL FRAME, PAINTED, COLOR: CYCLE
SL	THRESHOLD	F1-1	FENCE, TYPE: CYC
CR	WEATHER STRIPPING		
SP	SPRING BOLT		
CR	CARD READER		
PB	PUSH BAR		
AS	SECURITY ASTRAHAL FULL LENGTH		
ES	ELECTRIC STRIKE		

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Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

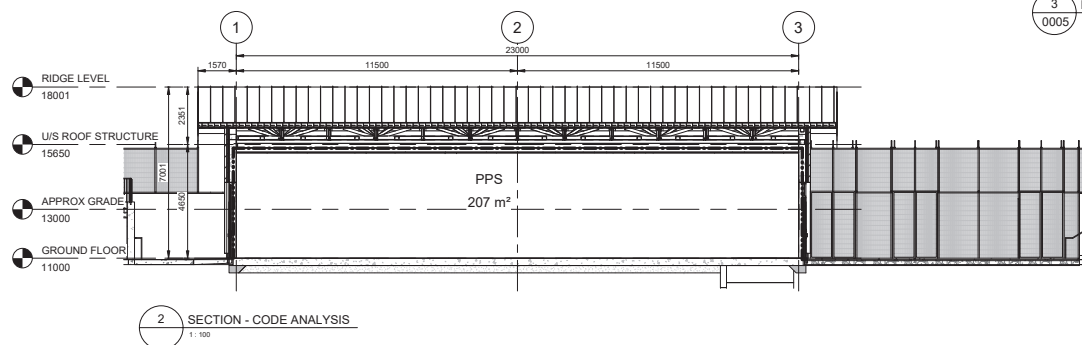
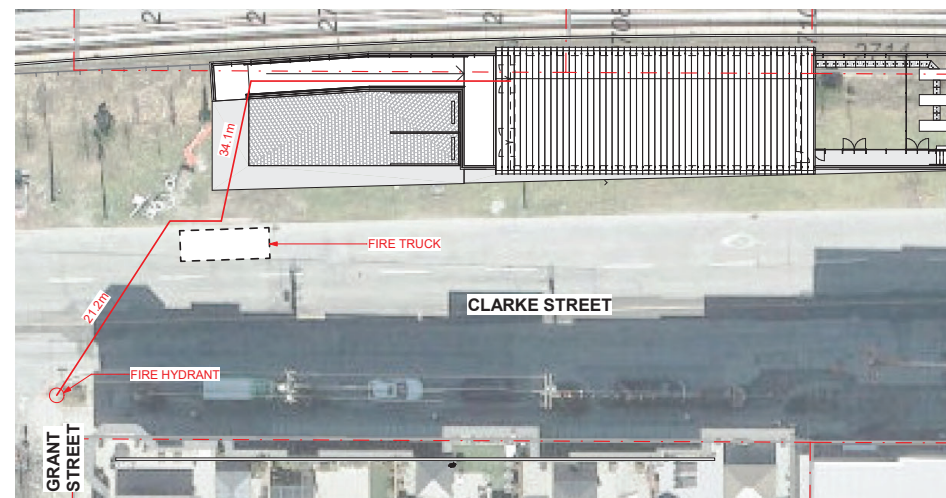
**EMUP
PROPULSION
POWER
UPGRADES**
PORT MOODY

DRAWN BY	Author
DESIGNED BY	Designer
CHECKED BY	Checker
APPROVED BY	Approver

PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
GENERAL NOTES

DRAWING NUMBER

193028-03 -1000- STMCZ- FR- AG- 0004-
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3 FIRE DEPARTMENT ACCESS
0005 1:200

FIRE SEPARATIONS

2HR FR

3HR FR

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CONSULTANTS

ELECTRICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

MECHANICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.sncclavalin.com

CONSULTANTS

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
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www.sncilavalin.com

REGISTRATION

KEY PLAN

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EMUP PROPULSION POWER UPGRADES

PORT MOODY

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ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.sncilavalin.com

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SITE PLAN

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ELECTRICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

MECHANICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snciavalin.com

CONSULTANTS

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

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SNC-Lavalin
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PROJECT NUMBER
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SHEET TITLE
GROUND FLOOR PLAN

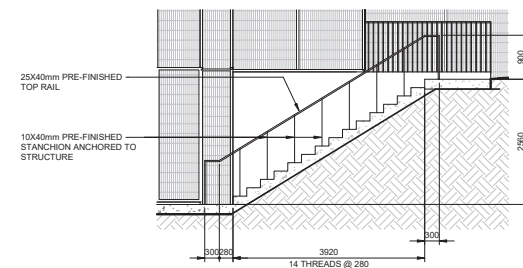
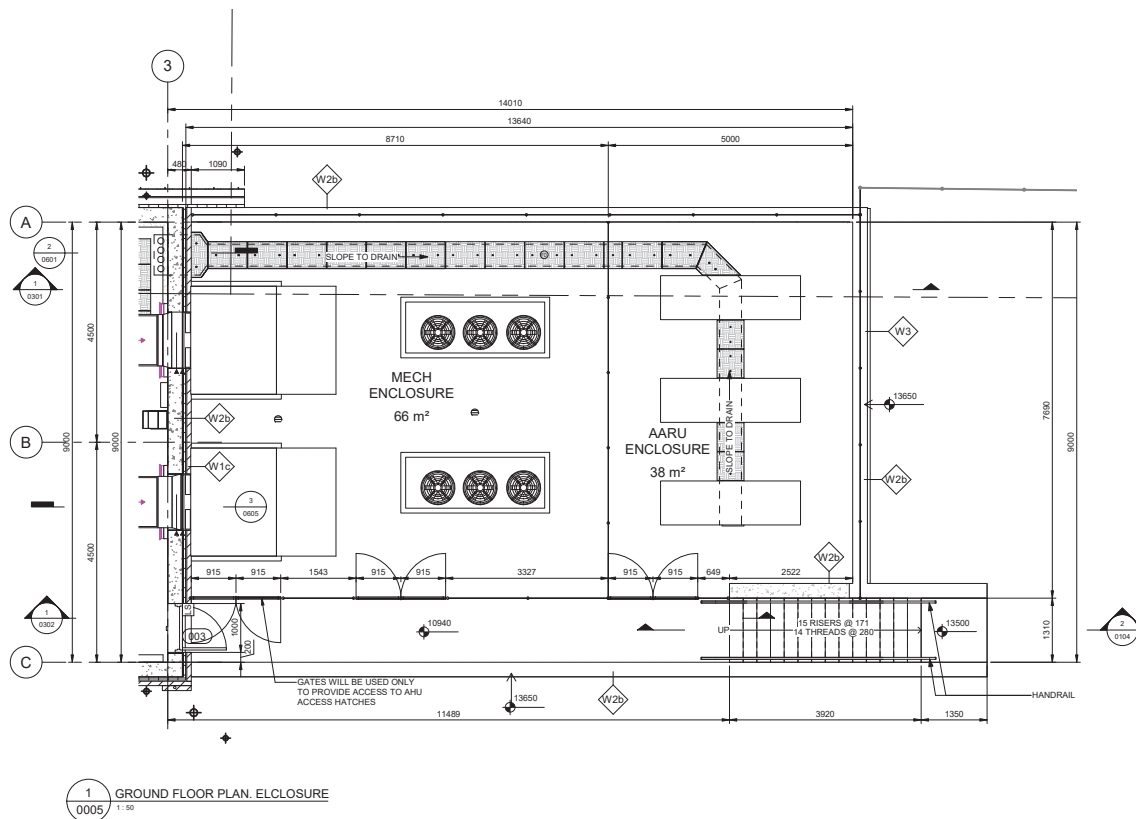
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2 STAIR SECTION
0104 1:50

1 GROUND FLOOR PLAN. ELCLOSURE
0005 1:50

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www.snc-lavalin.com

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SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

CONSULTANTS

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
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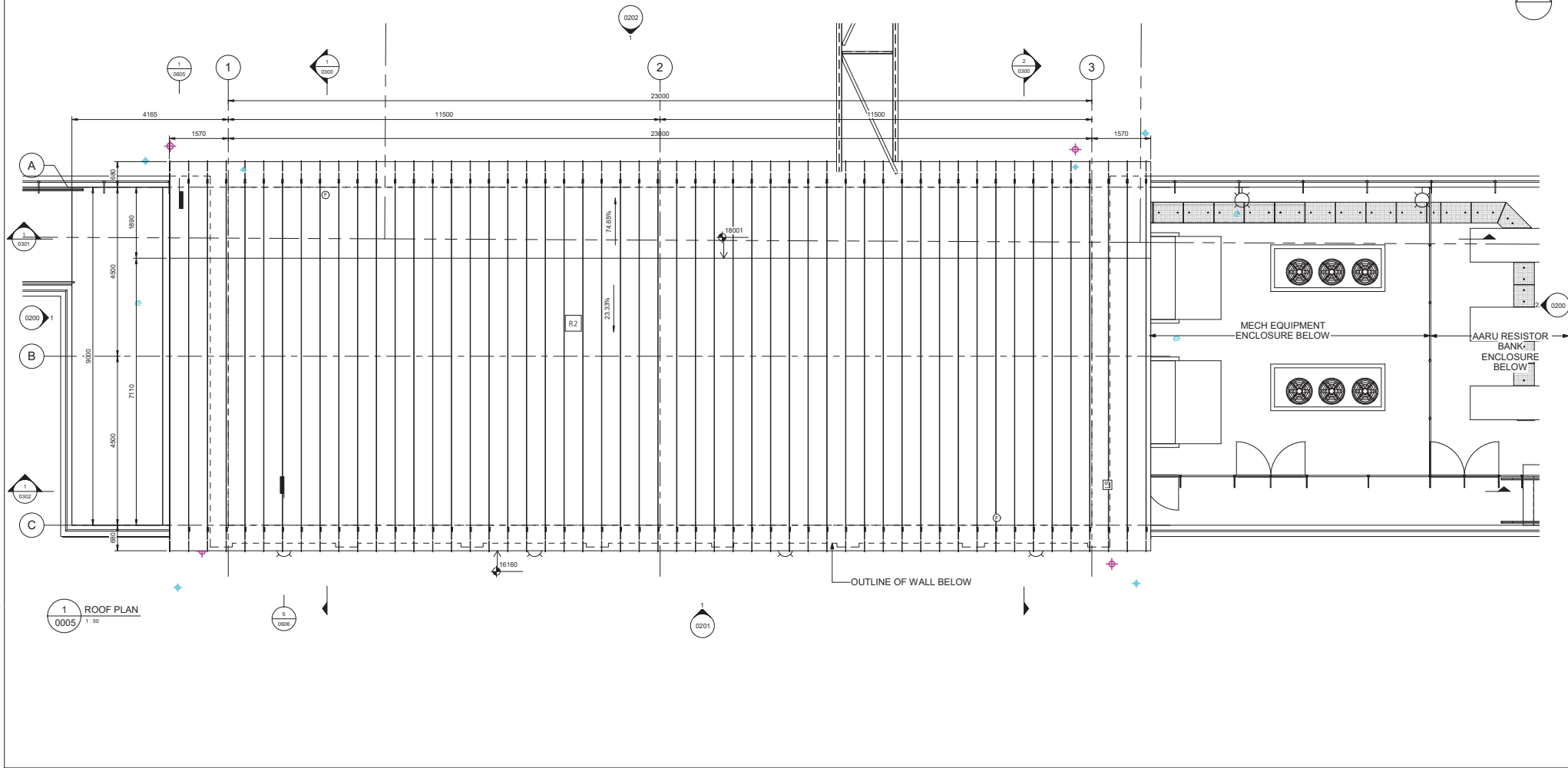
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193028 (SNC #075448)
SHEET TITLE
GROUND FLOOR PLAN.
ENCLOSURE

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SNC-Lavalin
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Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

MECHANICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

CONSULTANTS

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
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KEY PLAN

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**EMUP
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PORT MOODY

PORT MOODY

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CONTRACT NUMBER

PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
ROOF PLAN

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193028-03 -1000- STMCZ- FR- AG- 0105-
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MAS-2	FACE BRICK, COLOR TBC
MTL-3f	GUARD WITH PICKETS - BLACK, COLOR TBC
MTL-3s	STANDING SEAM - BLACK, COLOUR TBC
MTL-3d	GUARD W PICKETS - BLACK, COLOUR TBC
CON -1	CONCRETE FACED RIGID INSULATION
PMF-DR	INSULATED METAL DOOR w PRESSED METAL FRAME, PAINTED, COLOUR TBC
FC-1	FENCE, TYPE TBC

ISSUE/REVISION			REFERENCES
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CONSULTANTS	CONSULTANTS
ELECTRICAL SNCLavalin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 3Z7 604 662-3555 tel 604 662-7888 fax www.snclavalin.com	ARCHITECTURE FRANCL Architecture Inc. 1684 West 2nd Avenue Vancouver, BC V6J 1Y4 604 688 3252 tel www.francharchitecture.com
MECHANICAL SNCLavalin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 3Z7 604 662-3555 tel 604 662-7888 fax www.snclavalin.com	STRUCTURAL SNCLavalin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 3Z7 604 662-3555 tel 604 662-7888 fax www.snclavalin.com

REGISTRATION	KEY PLAN
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PROJECT

EMUP PROPULSION POWER UPGRADES

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CONTRACT NUMBER
PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
ELEVATIONS

DRAWING NUMBER
193028-03 -1000- STMCZ- FR- AG- 0200-
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IR	DATE	BY DESCRIPTION	APPR	



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Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

MECHANICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
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CONSULTANTS

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

REGISTRATION

KEY PLAN

PROJECT

EMUP PROPULSION POWER UPGRADES

PORT MOODY

DRAWING INFORMATION

DRAWN BY	Author
DESIGNED BY	Designer
CHECKED BY	Checker
APPROVED BY	Approver

CONTRACT NUMBER

PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
ELEVATIONS

DRAWING NUMBER

193028-03 -1000- STMCZ- FR- AG- 0201-
A



MAS-2	FACE BRICK, COLOR TBC
MTL-3f	GUARD WITH PICKETS - BLACK, COLOR TBC
MTL-3s	STANDING SEAM - BLACK, COLOUR TBC
MTL-3d	GUARD W PICKETS - BLACK, COLOUR TBC
CON -1	CONCRETE FACED RIGID INSULATION
PMF-DR	INSULATED METAL DOOR w PRESSED METAL FRAME, PAINTED, COLOUR TBC
FC-1	FENCE, TYPE TBC

ISSUE/REVISION		REFERENCES
A	2022-11-23	60% SUBMISSION
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BY	DATE	DESCRIPTION
		APPR



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SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

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Burnaby, B.C. V5H 3Z7
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REGISTRATION

KEY PLAN

PROJECT

EMUP PROPULSION POWER UPGRADES

PORT MOODY

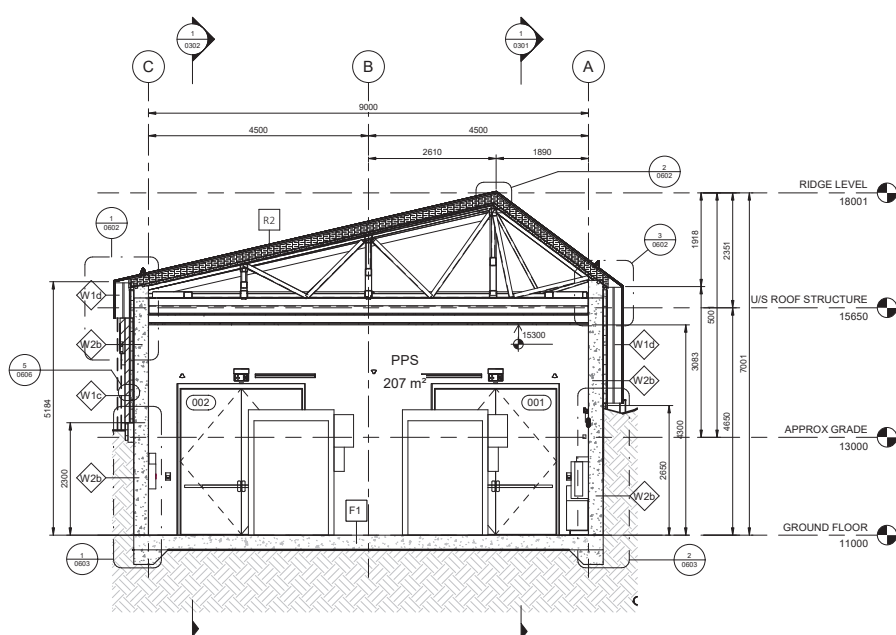
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DRAWN BY	Author
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CHECKED BY	Checker
APPROVED BY	Approver

CONTRACT NUMBER

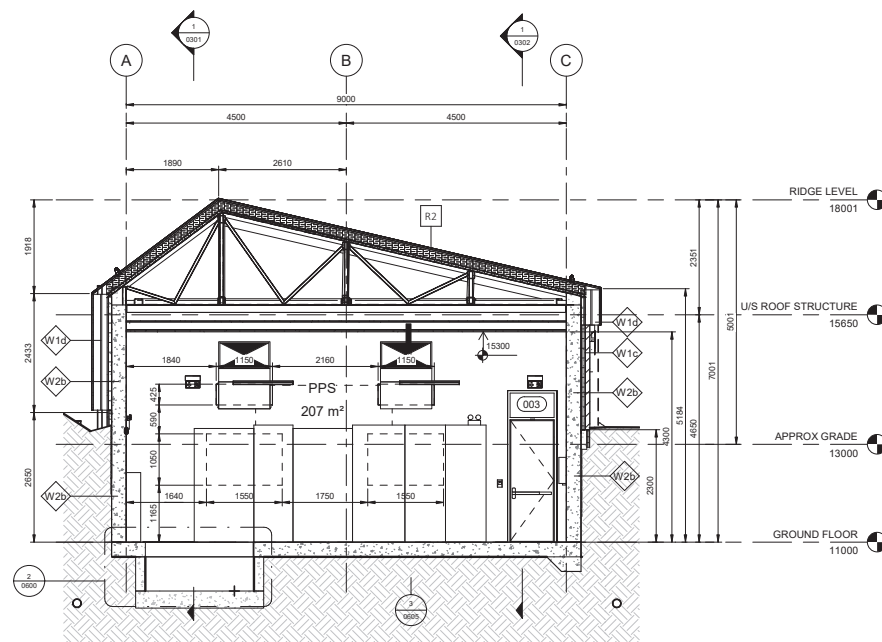
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193028 (SNC #675448)
SHEET TITLE
ELEVATIONS

DRAWING NUMBER

193028-03 -1000- STMCZ- FR- AG- 0202-
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1
0102 SECTION A
1 : 50



2
0102 SECTION B
1 : 50

ISSUE/REVISION		REFERENCES	
A	2022-11-23	60% SUBMISSION	
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Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

MECHANICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

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ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.francarchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

REGISTRATION

KEY PLAN

PROJECT

EMUP PROPULSION POWER UPGRADES

PORT MOODY

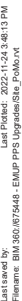
DRAWING INFORMATION	
DRAWN BY	Author
DESIGNED BY	Designer
CHECKED BY	Checker
APPROVED BY	Approver

CONTRACT NUMBER

PROJECT NUMBER	193028 (SNC #675448)
SHEET TITLE	SECTIONS

DRAWING NUMBER

193028-03 -1000- STMCZ- FR- AG- 0300-
A



ISSUE/REVISION			REFERENCES	
A	2022-11-23		60% SUBMISSION	
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www.snc-lavalin.com

MECHANICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snciavalin.com

CONSULTANTS

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
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REGISTRATION

KEY PLAN

PROJECT

EMUP PROPULSION POWER UPGRADES

PORT MOODY

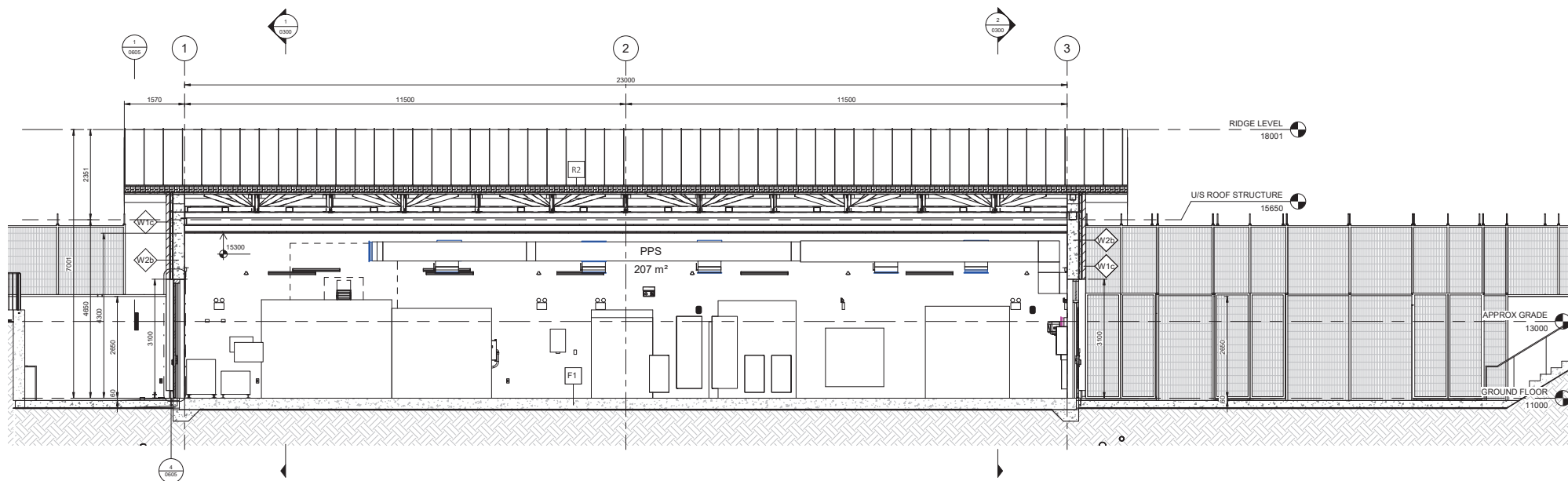
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DRAWN BY	Author
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CONTRACT NUMBER

PROJECT NUMBER	193028 (SNC #675448)
SHEET TITLE	SECTIONS

DRAWING NUMBER

193028-03 -1000- STMCZ- FR- AG- 0301-
A



1 SECTION D
0102 1 : 50

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ISSUE/REVISION			REFERENCES	
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MECHANICAL SNC-Lavalin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 3Z7 604 662-3555 tel 604 662-7688 fax www.snc.lavalin.com	STRUCTURAL SNC-Lavalin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 3Z7 604 662-3555 tel 604 662-7688 fax www.snc.lavalin.com

REGISTRATION KEY PLAN

PROJECT

**EMUP
PROPULSION
POWER
UPGRADES**

PORT MOODY

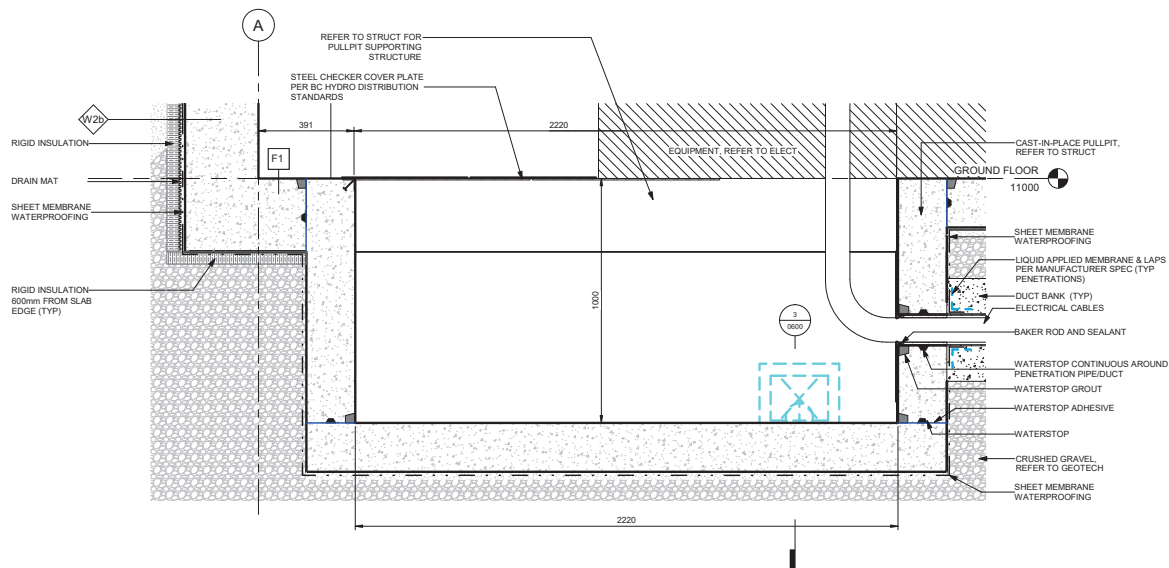
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CONTRACT NUMBER
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SHEET TITLE
SECTIONS

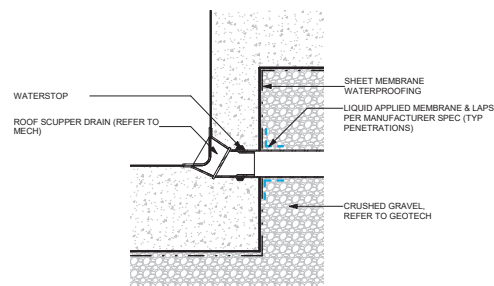
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193028-03 -1000- STMCZ- FR- AG- 0302-

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2 BC HYDRO PULLPIT SECTION
0300 1:10



3 PULLPIT DRAIN SECTION
0600 1:10

ISSUE/REVISION						REFERENCES
A	2022-11-23		80% SUBMISSION			
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Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.sncclavalin.com

MECHANICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.sncclavalin.com

CONSULTANTS

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
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REGISTRATION

KEY PLAN

PROJECT

EMUP PROPULSION POWER UPGRADES

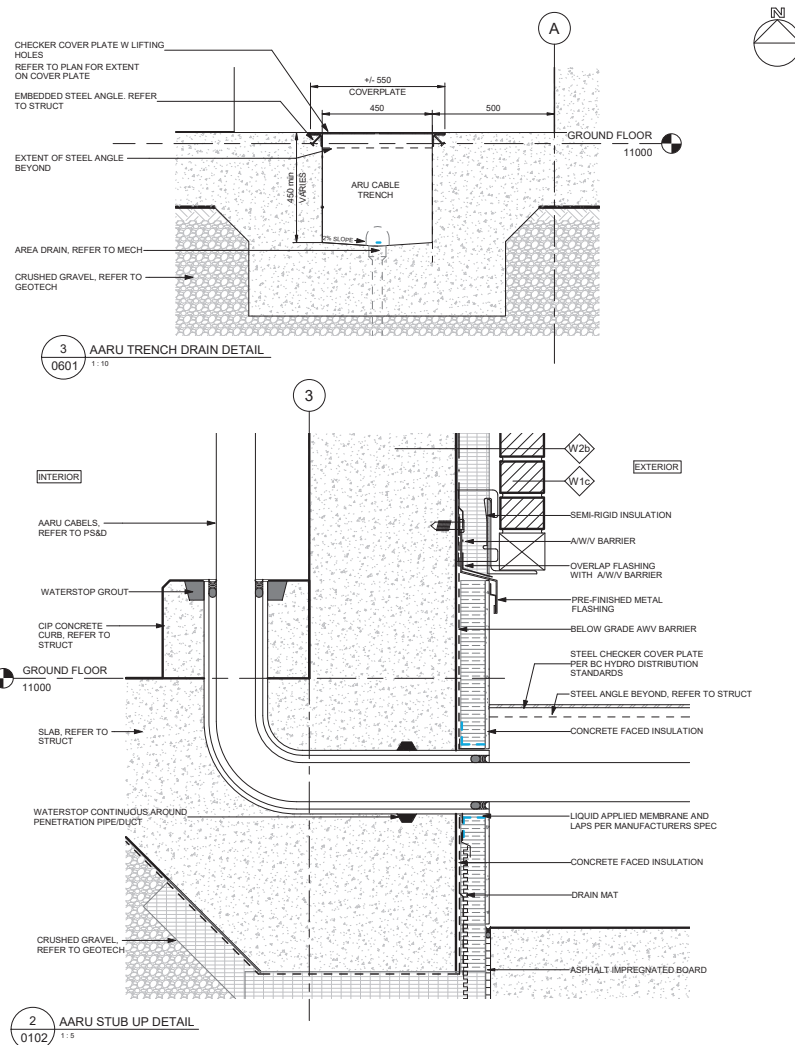
DRAWING INFORMATION	
DRAWN BY	Author
DESIGNED BY	Designer
CHECKED BY	Checker
APPROVED BY	Approver

CONTRACT NUMBER

PROJECT NUMBER	193028 (SNC #675448)
SHEET TITLE	FOUNDATION DETAILS

DRAWING NUMBER

193028 - 03 -1000- STMCZ- FR- AG-
0600- A



ISSUE/REVISION					REFERENCES				
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604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

MECHANICAL
SNC-Lavalin
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Burnaby, B.C. V5H 3Z7
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www.snc-lavalin.com

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ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclichitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
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REGISTRATION

KEY PLAN

PROJECT

EMUP PROPULSION POWER UPGRADES

PORT MOODY

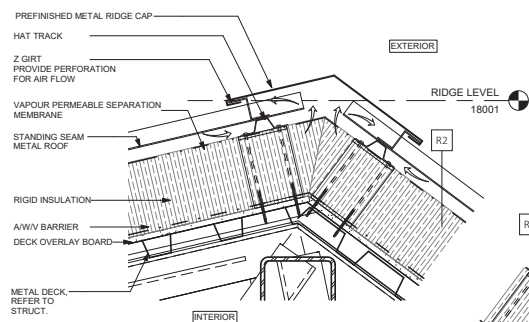
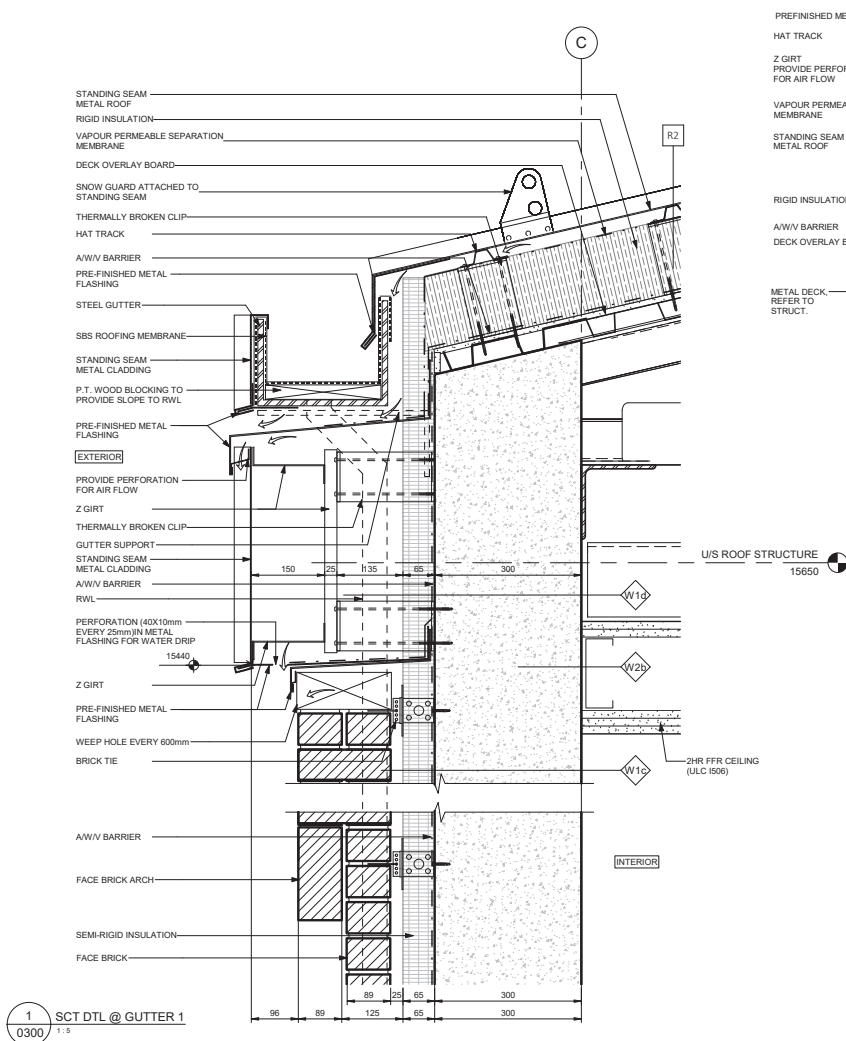
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DESIGNED BY	Designer
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APPROVED BY	Approver

CONTRACT NUMBER

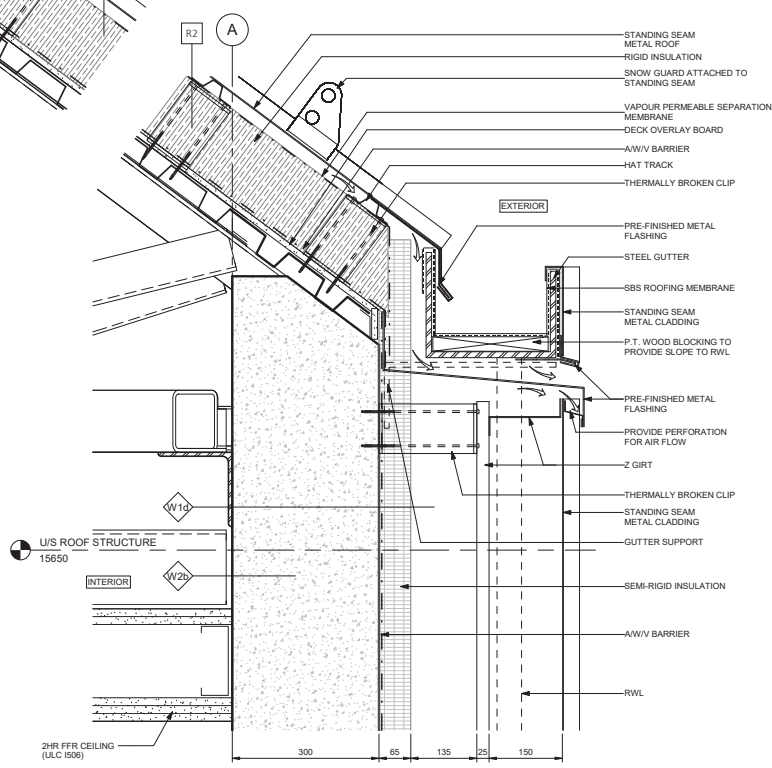
PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
FOUNDATION DETAILS

DRAWING NUMBER

193028 - 03 -1000- STMCZ- FR- AG-
0601- A



2 SGT DTL @ RIDGE
0300 1:5



3 SCT DTL @ GUTTER 2
0300 1:5

ISSUE/REVISION						REFERENCES	
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IR	_DATE	BY		_DESCRIPTION		APPR	



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REGISTRATION KEY PLAN

PROJECT

**EMUP
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POWER
UPGRADES**

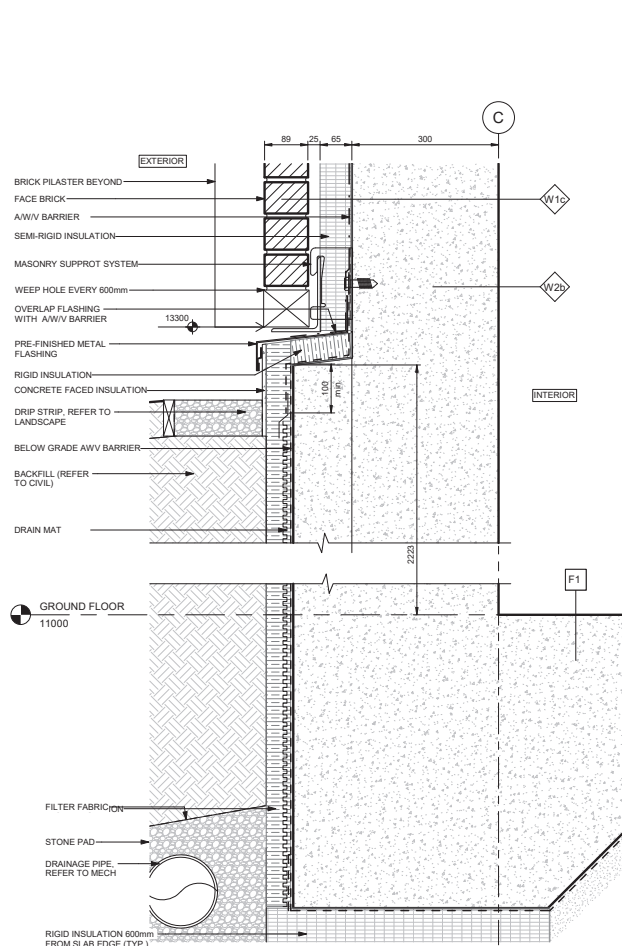
PORT MOODY

DRAWING INFORMATION	
DRAWN BY	Author
DESIGNED BY	Designer
CHECKED BY	Checker
APPROVED BY	Approver

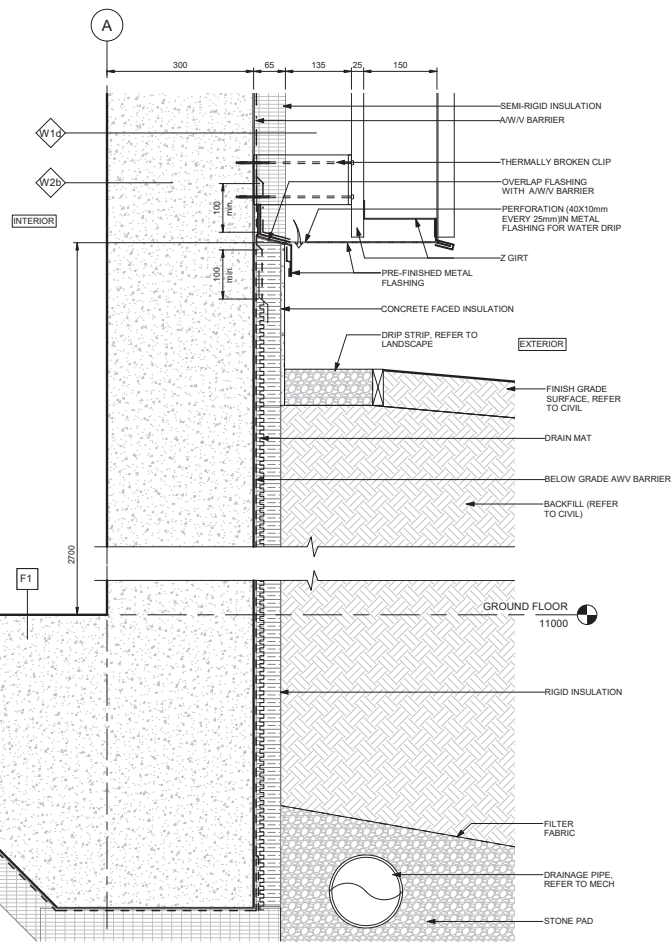
CONTRACT NUMBER
PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
ROOF & WALL DETAILS

DRAWING NUMBER

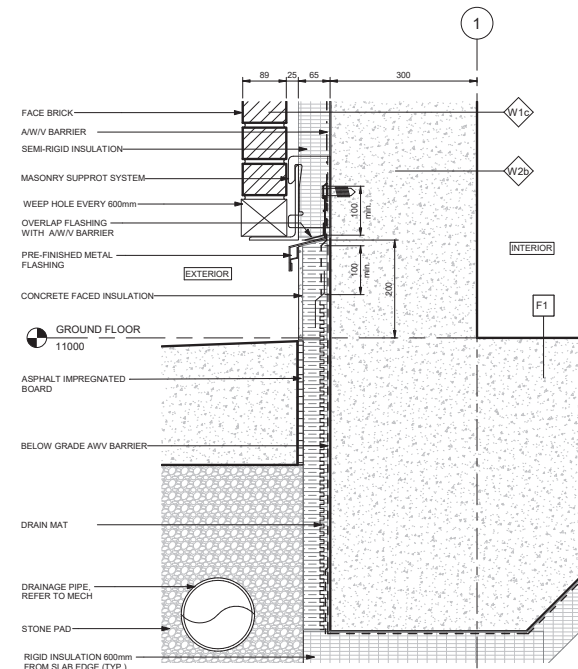
193028 - 03 -1000- STMCZ- FR- AG-
0602- A



1 SCT DTL @ GRAT WITH BRICK CLADDING
0300 1:5



2 SECTION DETAIL @ GRADE WITH METAL CLADDING
0300 1:5



3 SCT DTL @ GRADE
0102 1:5

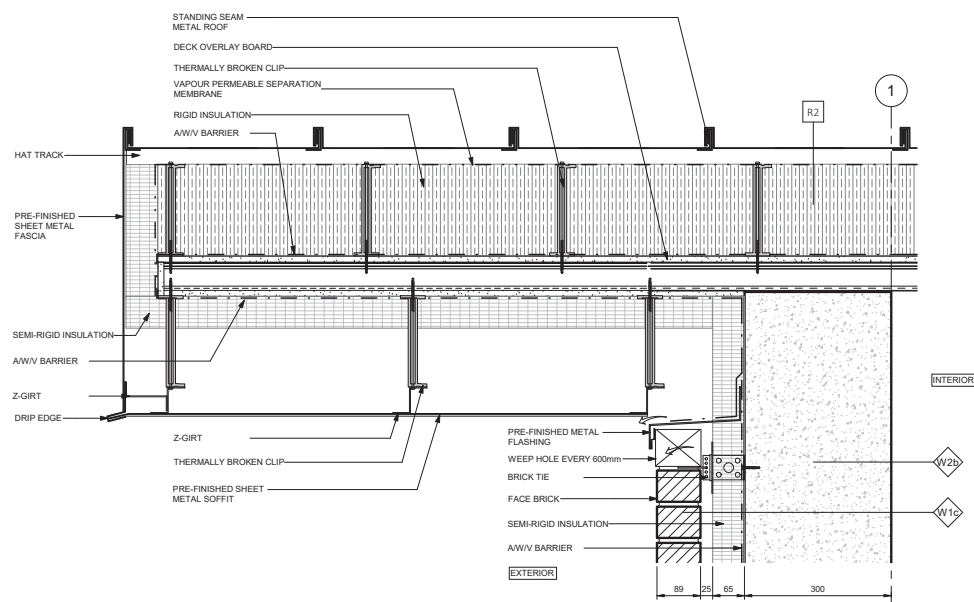
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A	2023-11-23		60% SUBMISSION	
IR#	DATE	BY	DESCRIPTION	APPR



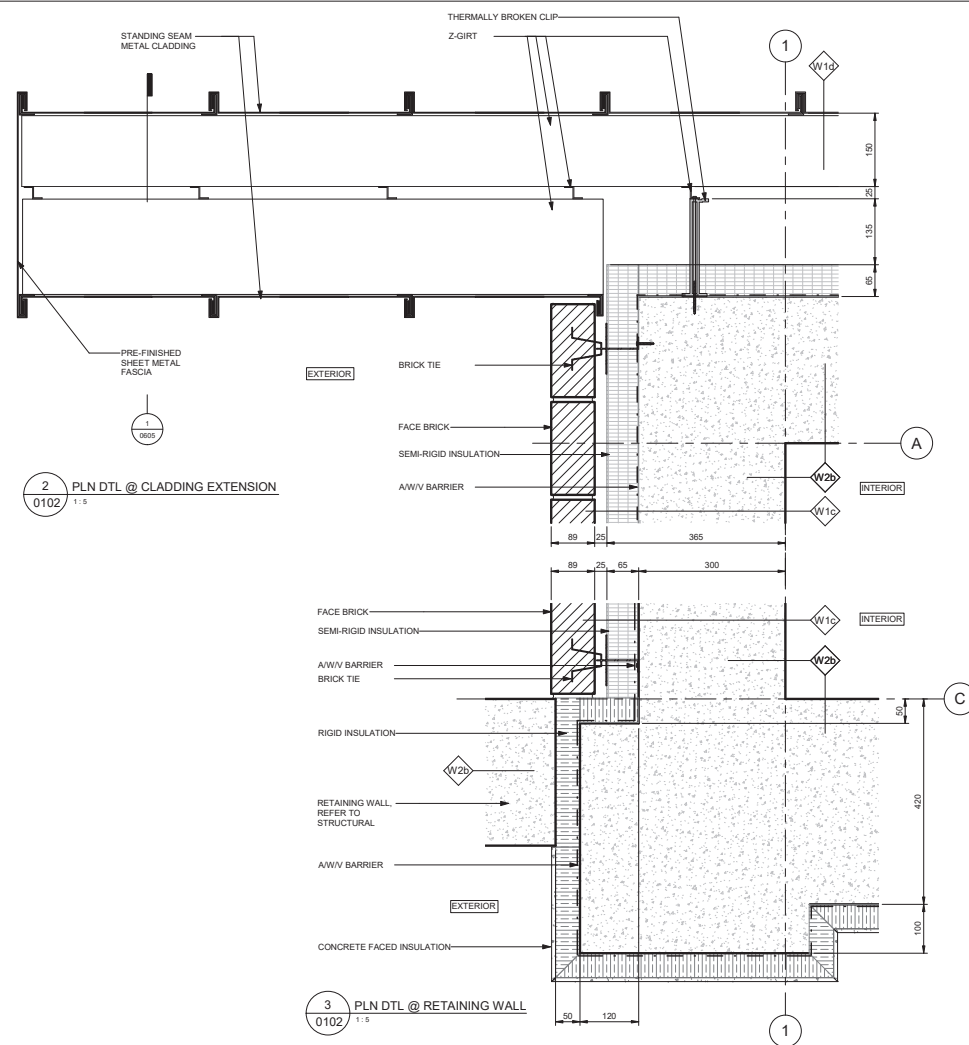
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REGISTRATION	KEY PLAN
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PROJECT	DRAWING INFORMATION	CONTRACT NUMBER
EMUP PROPULSION POWER UPGRADES	DRAWN BY <i>Author</i>	PROJECT NUMBER 193028 (SNC #675448) SHEET TITLE WALL & FOUNDATION DETAILS
	DESIGNED BY <i>Designer</i>	
	CHECKED BY <i>Checker</i>	
	APPROVED BY <i>Approver</i>	
PORT MOODY	DRAWING NUMBER	
	193028 - 03 - 1000- STMCZ- FR- AG- 0803- <i>A</i>	



1 SCT DTL @ ROOF OVERHANG
0301 1:5



3
0102

PLN DTL @ RETAINING WALL
1:5

ISSUE/REVISION				REFERENCES
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www.snc-lavalin.com

MECHANICAL
SNC-Lavalin
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Burnaby, B.C. V5H 3Z7
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CONSULTANTS

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

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REGISTRATION

KEY PLAN

PROJECT

**EMUP
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POWER
UPGRADES**
PORT MOODY

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CHECKED BY	Checker
APPROVED BY	Approver

CONTRACT NUMBER

PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
WALL DETAILS

DRAWING NUMBER

193028 - 03 -1000- STMCZ- FR- AG-
0604- A



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REGISTRATION	KEY PLAN
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PROJECT	DRAWING INFORMATION
EMUP PROPULSION POWER UPGRADES PORT MOODY	DRAWN BY Author
	DESIGNED BY Designer
	CHECKED BY Checker
	APPROVED BY Approver

CONTRACT NUMBER
PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
WALL DETAILS

DRAWING NUMBER
193028 - 03 -1000- STMCZ- FR- AG-
0605- A



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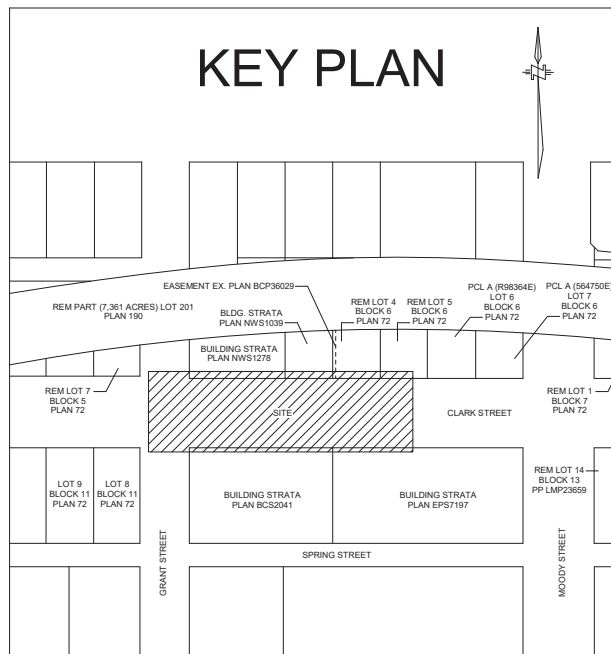
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MECHANICAL SNCL-Lavalin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 3Z7 604 662-3555 tel 604 662-7888 fax www.snclavalin.com	STRUCTURAL SNCL-Lavalin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 3Z7 604 662-3555 tel 604 662-7888 fax www.snclavalin.com

PROJECT	DRAWING INFORMATION	CONTRACT NUMBER
EMUP PROPULSION POWER UPGRADES	DRAWN BY <i>Author</i>	PROJECT NUMBER 193028 (SNC #675448) SHEET TITLE MISC DETAILS
	DESIGNED BY <i>Designer</i>	
	CHECKED BY <i>Checker</i>	
	APPROVED BY <i>Approver</i>	
PORT MOODY	DRAWING NUMBER	
	193028 - 03 -1000- STMCZ- FR- AG- 0806-A	

Expo and Millennium Upgrade Project (EMUP) Propulsion Power Upgrades

STRUCTURAL PORT MOODY

60% SUBMISSION
2022-11-23



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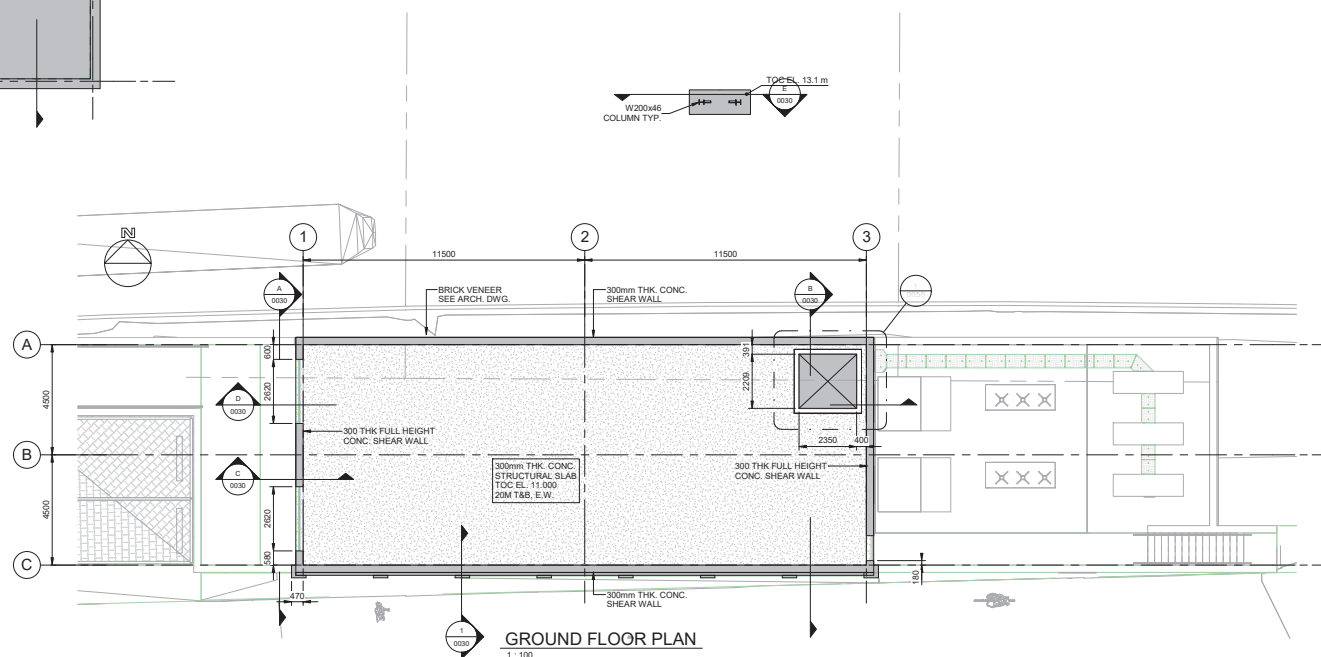
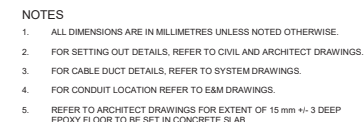


PROVINCE CONTRACT No. 193028	SUB-CONSULTANT No. SNC #675448
DRAWING No. 193028-03 -1000- STMCZ- SL- SG- 0000- A	A

ANSI D 29" x 34"	<div>LOAD BEARING MASONRY</div> <div><div><div>1. FOR ADDITIONAL REQUIREMENTS SEE UNIT MASONRY SPECIFICATION AND STRUCTURAL DRAWINGS.</div><div>2. MASONRY WORK SHALL CONFORM TO CSA S304 AND ITS REFERENCED DOCUMENTS, INCLUDING:<div><div>a. CONCRETE BLOCK TO CAN/CSA-A165.1, TYPE H20/A, UNLESS NOTED OTHERWISE ON SCHEDULE (BASED ON NET AREA)</div><div>b. MORTAR TO CAN/CSA-A179, TYPE 'S' FOR ALL WALLS</div><div>c. GROUT TO CAN/CSA-A179 (20MPa)</div><div>d. MASONRY WIRE REINFORCING TO CSA ASTM A185/A185 M-07</div><div>e. REINFORCING BARS TO CAN/CSA-G30.18 - 400 MPa</div><div>f. JOINT REINFORCING TO CSA G30.14 AND CSA G30.15</div><div>g. CONNECTORS TO CAN/CSA-A370</div><div>h. PRACTICE TO CAN/CSA-A371</div></div></div><div>3. STRUCTURAL DRAWINGS INDICATE ONLY LOAD-BEARING WALLS. DESIGN IS BASED ON ENGINEERING ANALYSIS ACCORDING TO CSA S304.</div><div>4. CONCRETE MASONRY UNITS SHALL CONFORM TO CSA A165 SERIES - 04. COMPRESSIVE STRENGTH FOR CMU SHALL BE 20 MPa AT 28 DAYS WITH fm = 10 MPa. MASONRY CONTRACTOR SHALL PROVIDE TO THE ENGINEER WRITTEN CONFIRMATION OF UNIT STRENGTHS PRIOR TO INSTALLATION.</div><div>5. USE ONLY TYPE 'S' MORTAR TO CSA A179-14. NO CALCIUM CHLORIDE IN ANY FORM IS PERMITTED IN THE GROUT OR MORTAR MIXES. MINIMUM STRENGTH OF MORTAR CUBE AT 28 DAYS SHALL BE 20MPa.</div><div>6. MASONRY CONTRACTOR MUST DISCUSS ALL MASONRY CONSTRUCTION WITH THE ENGINEER PRIOR TO COMMENCING WORK.</div><div>7. FILL BLOCK CORES UNDER ALL CONCENTRATED LOADS WITH 20 MPa CONCRETE GROUT TO A DEPTH OF AT LEAST 400 mm MEASURED DOWN FROM THE BEARING.</div><div>8. PROVIDE GALVANIZED HEAVY DUTY LADDER TYPE OR GALVANIZED TRUSS TYPE MASONRY JOINT REINFORCING 3.8 mm DIA (9 GA) IN HORIZONTAL JOINTS EVERY SECOND COURSE (400 mm) UNLESS NOTED OTHERWISE IN DRAWINGS.</div><div>9. PROVIDE LINTELS OVER ALL OPENINGS IN WALLS.</div><div>10. PROVIDE 2-15M CONTINUOUS IN TOP COURSE OF WALL, UNDER BEAM OR JOIST BEARING AND FILL CELLS SOLID WITH 20 MPa CONCRETE GROUT.</div><div>11. LAPS: WIRE REINFORCEMENT<div><div>15M BARS</div><div>15M BARS</div><div>20M BARS</div></div><div><div>-</div><div>-</div><div>-</div></div><div><div>200 mm</div><div>450 mm</div><div>650 mm</div></div></div><div>12. UNLESS NOTED OTHERWISE, PROVIDE 2-15M VERTICAL BARS FULL HEIGHT AT:<div><div>a. ENDS OF WALLS.</div><div>b. EACH CORNER AND AT INTERSECTIONS.</div><div>c. EACH SIDE OF DOORS AND OTHER OPENINGS.</div></div></div><div>13. ALL VERTICAL REINFORCING SHALL BE CONTINUED TO WITHIN 50 mm OF TOP OF THE WALL.</div><div>14. PROVIDE HOOKED DOWELS FROM BOTTOM OF CONCRETE BASE TO MATCH VERTICAL MASONRY WALL REINFORCING. THE CONTRACTOR AND MASONRY CONTRACTOR SHALL ENSURE DOWELS ARE CENTERED ACCURATELY WITH CELLS OVER.</div><div>15. ALL BARS MUST BE CONTINUOUS, PROPERLY LAPPED AT SPICES, AT CORNERS AND INTERSECTIONS. HORIZONTAL REINFORCEMENT SHALL BE BENT AND LAPPED.</div><div>16. PROVIDE CLEANOUTS FOR ALL CELLS TO BE REINFORCED, REPEAT CLEANOUTS ABOVE BOND BEAMS.</div><div>17. FILL ALL CELLS WITH 20 MPa CONCRETE (10 mm AGGREGATE, 180 mm SLUMP) VIBRATE OR PUDDLE TO FILL CELLS COMPLETELY.</div><div>18. FILL CELLS IN 1200 mm LIFTS.</div><div>19. CONTROL JOINTS SHALL BE INSTALLED AT MAXIMUM SPACING OF 6000 mm UNLESS OTHERWISE SHOWN ON DRAWINGS.</div><div>20. CONTROL JOINTS AND EXPANSION JOINTS SHALL BE CONTINUED THROUGH BOND BEAMS UNLESS OTHERWISE SHOWN.</div><div>21. OUTSIDE FACE OF CONCRETE MASONRY UNIT WALLS SHALL BE WATERPROOFED AS PER SPECIFICATIONS AND ARCHITECTURAL DRAWINGS.</div><div>22. NO MASONRY WORK SHALL BE PERMITTED WITH TEMPERATURE BELOW 5° CELSIUS, UNLESS COLD TEMPERATURE CONSTRUCTION PRACTICE IS IMPLEMENTED.</div><div>23. PROVIDE 1200 mm STARTER DOWELS TO MATCH ALL VERTICAL REINFORCING, UNLESS NOTED OTHERWISE.</div><div>24. LOAD BEARING MASONRY IS SHOWN AS ON PLAN.</div><div>25. DO NOT USE EXPANSION ANCHORS IN MASONRY WALL.</div><div>26. INTERLOCK COURSES SHALL BE PROVIDED AT ALL WALL CORNERS AND INTERSECTIONS WITH RUNNING BOND, UNLESS NOTED OTHERWISE ON DRAWINGS.</div></div></div>	<div>STRUCTURAL & MISCELLANEOUS STEEL NOTES</div> <div><div><div>1. STRUCTURAL STEEL DESIGN, FABRICATION AND ERECTION SHALL COMPLY WITH THE LATEST EDITIONS OF GOVERNING CODES, SPECIFICATIONS AND STANDARDS INCLUDING:<div><div>- BC BUILDING CODE (BCBC)</div><div>- CISC CODE OF STANDARD PRACTICE FOR STRUCTURAL STEEL</div><div>- CAN/CSA S16-14 LIMIT STATES DESIGN OF STEEL STRUCTURES</div></div></div><div>2. STRUCTURAL STEEL MATERIALS:<div><div>- NEW, FREE FROM DEFECTS IMPAIRING STRENGTH OR DURABILITY.</div><div>a. STRUCTURAL STEEL SHAPES SHALL CONFORM TO CAN/CSA-G40.20-13/G40.21-13. ALL MATERIALS SHALL BE 350W GRADE UNLESS SHOWN AS 350WT OR OTHERWISE IN DESIGN DRAWINGS. HOLLOW SECTIONS SHALL BE 350W GRADE, CLASS 'C' MATERIAL. PLATES, CHANNELS, ANGLES AND BARS SHALL BE 300W GRADE.</div><div>b. HIGH STRENGTH BOLTS: ASTM A325, TYPE 1.</div><div>c. ANCHOR RODS SHALL BE ASTM A307 FOR GENERAL APPLICATION AND ASTM F1554 GRADE 105 FOR HIGH STRENGTH APPLICATION OR AS OTHERWISE INDICATED ON THE DRAWING.</div><div>d. WELDING RODS CSA-W59-18, E49xx LOW HYDROGEN TYPE.</div></div></div><div>3. THE DESIGN, DETAILING, AND FABRICATION OF MEMBER CONNECTIONS SHALL BE IN ACCORDANCE WITH CURRENT PRACTICES TO CONTROL BRITTLE FRACTURE OF MEMBERS OR ASSEMBLIES.</div><div>4. DESIGN ALL BOLTED STRUCTURAL CONNECTIONS AS BEARING TYPE IN ACCORDANCE WITH CISC STANDARDS USING A MINIMUM OF TWO M20 BOLTS. SMALLER SIZE BOLTS MAY BE USED IN MINOR MEMBER CONNECTIONS AS APPROVED ON SHOP DRAWINGS.</div><div>5. DESIGN BEAM CONNECTIONS IN ACCORDANCE WITH CISC HANDBOOK LIMIT STATE DESIGN. LATEST EDITION, USING THE GREATER OF FOLLOWING BEAM SHEAR PLUS AXIAL LOADS IF SHOWN:<div><div>a. BEAM REACTIONS IF SHOWN ON THE DRAWINGS</div><div>b. HALF THE SHEAR CAPACITY OF THE MEMBER CONNECTED</div><div>c. SPLICE CONNECTIONS SHALL DEVELOP THE FULL DESIGN STRENGTH OF THE MEMBER UNLESS OTHERWISE INDICATED ON THE DRAWINGS.</div></div></div><div>6. USE STANDARD BOLT HOLES IN ACCORDANCE WITH CISC STANDARDS.</div><div>7. CONTACT SURFACES OF CONNECTIONS WHEN ASSEMBLED IN FIELD SHALL BE FREE FROM OIL, LOOSE SCALE, AND ANY FOREIGN MATERIAL THAT WOULD PREVENT THE SOLID SEATING OF THE PARTS.</div><div>8. TEST A MINIMUM OF 10% OF ALL BOLTS, BUT NOT LESS THAN ONE BOLT PER MEMBER PER CONNECTION WITH A HAND TORQUE WRENCH TO CHECK THE PROPER EXECUTION OF BOLTING.</div><div>9. WELD IN ACCORDANCE WITH THE LATEST EDITION OF CSA W59 WELDED STEEL CONSTRUCTION.</div><div>10. ALL WELDING OPERATORS SHALL BE CERTIFIED IN ACCORDANCE WITH CSA W47.1 (DIVISION 1 OR 2.1) QUALIFICATION REQUIREMENTS.</div><div>11. PROVIDE ALL GROUT, AS PER SPECIFICATION, BETWEEN TOP OF CONCRETE AND UNDERSIDE OF BASE PLATE. GROUT DIMENSION WHERE IS SHOWN IS NOMINAL.</div><div>12. COATING:<div><div>a. PAINTING</div><div>ALL STEEL TO BE PAINTED UNLESS NOTED OTHERWISE ON THE DRAWINGS TO SPECIFICATION. PROVIDE TOUCH-UP PAINT TO DAMAGED SURFACES INCLUDING FIELD WELDING AREA.</div><div>b. HOT DIP GALVANIZING</div><div>WHERE NOTED ON DRAWING STEEL SHALL BE HOT DIP GALVANIZED TO CSA G164. REPAIR ALL DAMAGE ON GALVANIZING SURFACES WITH TWO COATS OF ZINCA-ZINC RICH PAINT, INCLUDING FIELD WELDING AREA.</div></div></div><div>13. SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS TO SHOW ALL DETAILS AND MATERIAL SPECIFICATION AND TO BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF BRITISH COLUMBIA FOR CONNECTION DESIGN. THE ENGINEER SEALING SHOP DRAWINGS SHALL BE RESPONSIBLE FOR ALL FIELD REVIEW OF THEIR WORK AND SHALL PROVIDE A SEALED LETTER (SCHEDULE - S) CONFIRMING THAT WORK HAS BEEN COMPLETED IN ACCORDANCE WITH FINAL REVIEWED SHOP DRAWINGS AND ALL CODE REQUIREMENTS.</div><div>14. SHOP AND FIELD INSPECTION AND TESTING OF STEEL FABRICATION, ERECTION AND INSTALLATION TO BE COMPLETED BY AN INDEPENDENT TESTING AND INSPECTION AGENCY RETAINED BY THE OWNER. TESTING AGENCY SHALL BE CERTIFIED TO CSA-W178.</div></div></div>	<div>STEEL DECK</div> <div><div><div>1. DESIGN, FABRICATE AND INSTALL STEEL DECK TO CSA S136 AND CANADIAN SHEET STEEL BUILDING INSTITUTE STANDARDS CSSBI 10M.</div><div>2. STEEL DECK, UNLESS NOTED OTHERWISE ON DRAWINGS, SHALL BE PREFORMED 38mm DEEP MINIMUM 22GA THICKNESS CONFORMING TO ASTM A653/ASTM A653M. STEEL DECK SHALL BE ZINC-COATED CONFORMING TO ASTM A653/ASTM A653M, 275 FOR INTERIOR EXPOSURE, 275 FOR EXTERIOR EXPOSURE AND UNHEATED AREAS. DECK FLUTES SPACED AT 152mm MAXIMUM ON CENTER.</div><div>3. PROVIDE ROOF DECK TYPE P806 COMPOSITE BY CANAM OR APPROVED EQUIVALENT.</div><div>4. FASTEN DECK TO SUPPORTING MEMBER AS FOLLOW UNLESS NOTED OTHERWISE ON ROOF PLAN DRAWING.<div><div>a. STRUCTURAL STEEL: HILTI X-ENP-19, @ 300mm c/c MINIMUM</div></div></div><div>5. FASTEN DECK SIDE LAPS WITH #10 SCREW @ 300mm c/c MINIMUM, UNLESS NOTED OTHERWISE ON ROOF PLAN DRAWING.</div><div>6. DECKING INSTALLERS MUST PROVIDE A VALID TRAINED OPERATOR - HILTI DECK FASTENING SYSTEM CARD. A DIRECT REPRESENTATIVE OF HILTI TO BE ON-SITE DURING INITIAL INSTALLATION TO PROVIDE TRAINING TO INSTALLERS IN PROPER SELECTION AND INSTALLATION PROCEDURES.</div><div>7. PLACE DECK SHEET IN A MINIMUM 3 SPAN LENGTH EXCEPT WHERE OTHERWISE APPROVED.</div><div>8. UNLESS NOTED OTHERWISE, REINFORCE ALL ROOF DECK OPENINGS GREATER THAN 150mm AND LESS THAN 400 mm WITH L38X38X6 EXTENDED AND FASTENED TO MINIMUM TWO FLUTES ON EACH SIDE OF OPENING. FRAME OPENINGS EQUAL TO 400mm OR LARGER WITH MINIMUM L76X76X6 EXTENDED TO MAIN STRUCTURAL MEMBERS OR AS SHOWN ON THE DRAWINGS.</div><div>9. SUBMIT DECK SHOP DRAWINGS TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL SHOW ALL DETAILS AND MATERIAL SPECIFICATIONS.</div></div></div> <div>FIELD REVIEWS</div> <div><div><div>1. THE CONTRACTOR'S SUPERINTENDENT IS TO PRE-INSPECT THE WORK TO CONFIRM WORK IS COMPLETED AS PER DOCUMENTS AND PROVIDE THE ENGINEER'S REPRESENTATIVE WITH A MINIMUM OF 72 HOURS NOTICE IN ACCORDANCE WITH BCBC 2018 AND MUNICIPAL BYLAWS FOR ROUTINE INSPECTIONS OF:<div><div>a. GENERAL SITE CONDITIONS PRIOR TO FORMING.</div><div>b. REINFORCING STEEL AND POUR CONDITIONS PRIOR TO EACH CONCRETE POUR.</div><div>c. STRUCTURAL STEEL FRAMING PRIOR TO BEING COVERED.</div><div>d. UPON PROJECT COMPLETION.</div></div></div><div>2. THE GEOTECHNICAL ENGINEER SHALL REVIEW FINAL PLANS AND INSPECT THE SITE PREPARATION WORK TO CONFIRM THAT THE SOIL CONDITIONS ARE CONSISTENT WITH DESIGN ASSUMPTIONS AND DESIGN RECOMMENDATIONS. INSPECTION TO INCLUDE THE FOLLOWING:<div><div>a. BASE MATERIALS FOR CONFIRMATION OF ASSUMED SOIL BEARING.</div><div>b. TESTING FOR COMPACTION OF ANY STRUCTURAL FILL REQUIRED UNDER FOOTINGS AND GRADE SLAB.</div><div>c. BACKFILL AROUND / UNDER GROUND SERVICES.</div></div></div><div>3. THE GEOTECHNICAL ENGINEER TO BE NOTIFIED MINIMUM 2-WEEKS PRIOR TO ANY FOOTING POURS FOR INSPECTION AS NOTED ABOVE.</div><div>4. REINSPECTION REQUIRED BY THE ENGINEER DUE TO INCOMPLETE WORK AND /OR DEFICIENCIES FROM PREVIOUS INSPECTIONS, SHALL BE AT THE EXPENSE OF THE CONTRACTOR.</div><div>5. ALL WORK SHALL BE MADE ACCESSIBLE FOR INSPECTION. FAILURE TO GIVE REQUIRED NOTIFICATION AND ACCESSIBILITY MAY RESULT IN THE ENGINEER REQUESTING THE REMOVAL AND REPLACEMENT OF THE WORK AT THE CONTRACTOR'S EXPENSE.</div><div>6. REVIEW OF THE WORK, OR ANY PORTION THEREOF, BY THE ENGINEER SHALL NOT IN ANY WAY RELIEVE THE CONTRACTOR OF HIS RESPONSIBILITY AND OBLIGATION TO COMPLY WITH THE CONTRACT DRAWINGS AND SPECIFICATIONS.</div></div></div>
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				ELECTRICAL SNC-Lavalin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 3Z7 604 662-3555 tel 604 662-7688 fax www.snc-lavalin.com	ARCHITECTURE FRANCL Architecture Inc. 1684 West 2nd Avenue Vancouver, BC V6J 1H4 604 688 3252 tel www.franclarchitecture.com			EMUP PROPULSION POWER UPGRADES PORT MOODY	DRAWN BY	A.CHEUNG	193028 (SNC #675448) SHEET TITLE GENERAL NOTES - SHEET 2
									DESIGNED BY	C.FRANCIS	
									CHECKED BY	M.HAN	
									APPROVED BY	K.BRIGNALL	
A	2022-11-23	JV	60% SUBMISSION	WZ	MECHANICAL SNC-Lavalin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 3Z7 604 662-3555 tel 604 662-7688 fax www.snc-lavalin.com	STRUCTURAL SNC-Lavalin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 3Z7 604 662-3555 tel 604 662-7688 fax www.snc-lavalin.com					DRAWING NUMBER
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SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

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ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

STRUCTURAL
SNC-Lavalin
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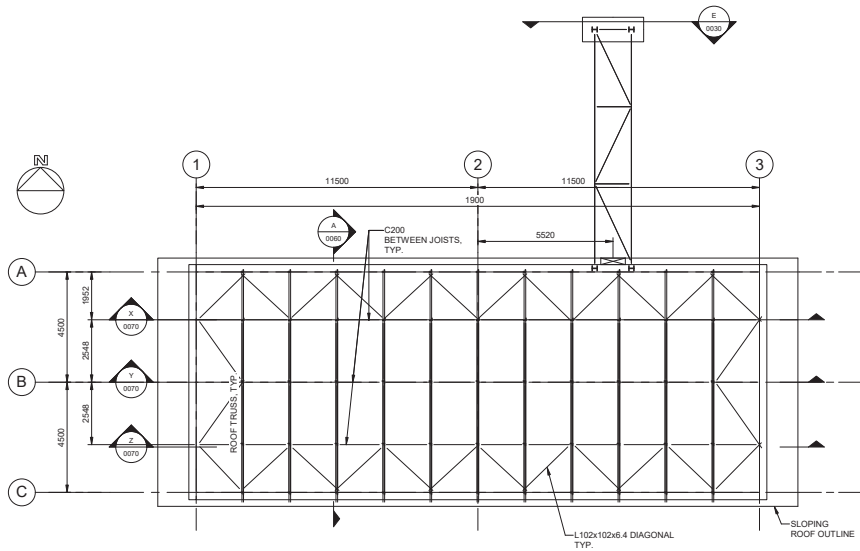
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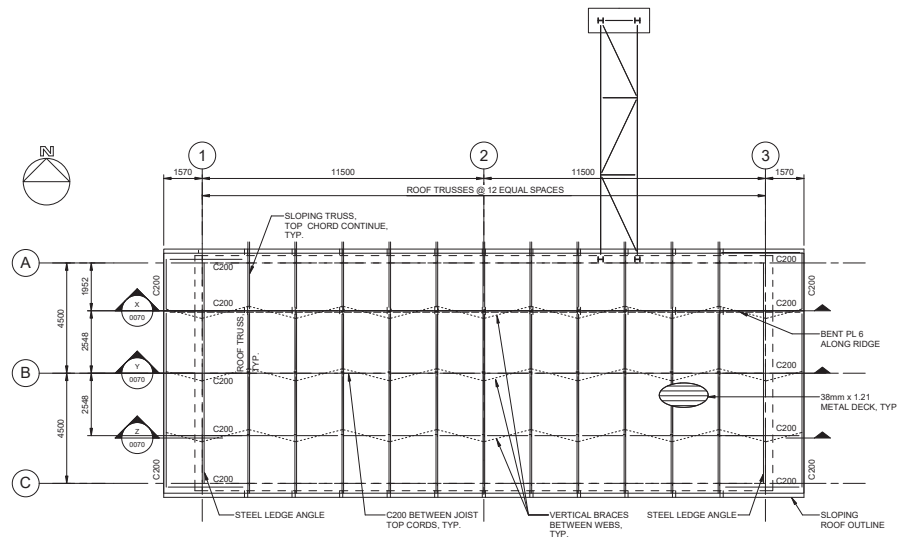
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ROOF JOIST BOTTOM CHORD PLAN

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ROOF TRUSS FRAMING PLAN

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Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

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PROJECT	DRAWING INFORMATION	CONTRACT NUMBER
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	DESIGNED BY C.FRANCIS	
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	APPROVED BY K.BRIGNALL	
DRAWING NUMBER		193028 - 03 -1000- STMCZ- SL- SG- 0020-A



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Suite 1300 - 3777 Kingsway Street
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604 662-3555 tel 604 662-7688 fax
www.sncclavalin.com

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ARCHITECTURE
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604 688 3252 tel
www.franclarchitecture.com

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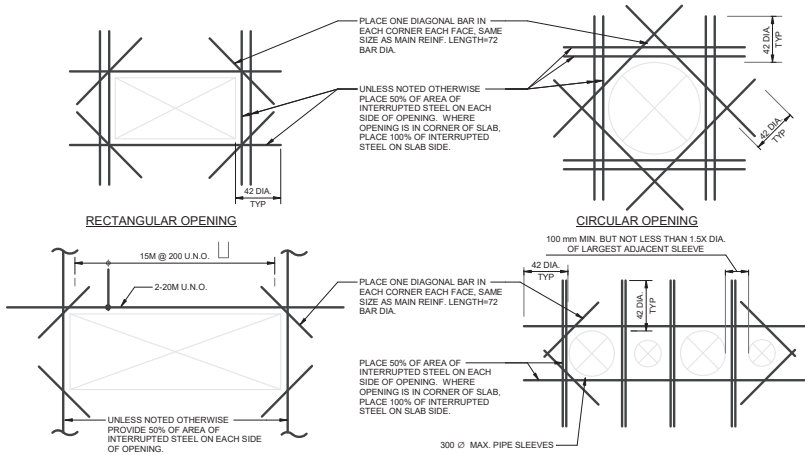
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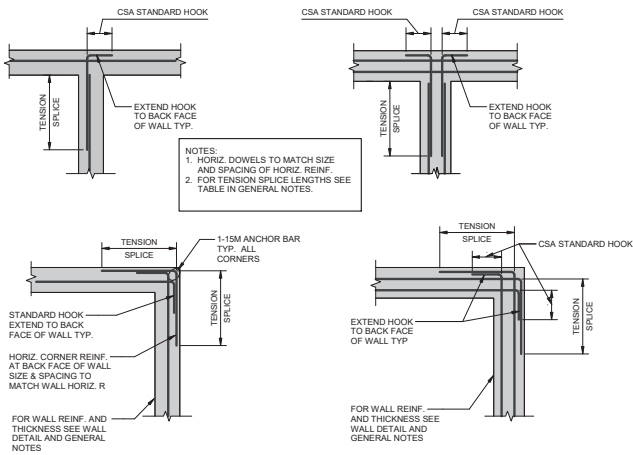
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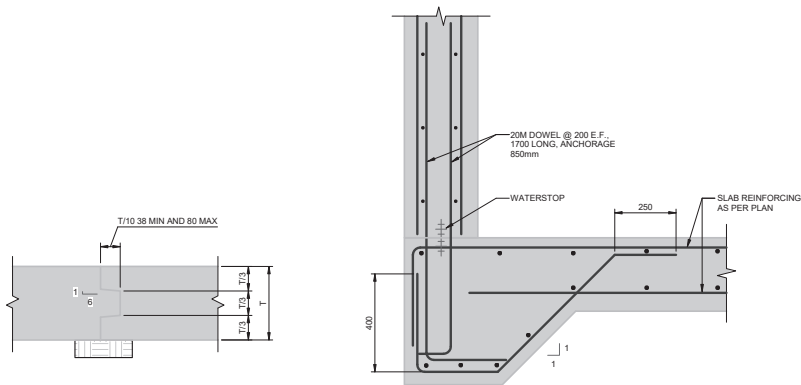
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HORIZONTAL REINFORCEMENT AT WALL CORNERS U.N.O.

1 WALLS AND SLABS ADDITIONAL REINFORCEMENT TO OPENINGS

2 HORIZONTAL REINFORCEMENT AT WALL CORNERS U.N.O.



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604 662-3555 tel 604 662-7688 fax
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1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

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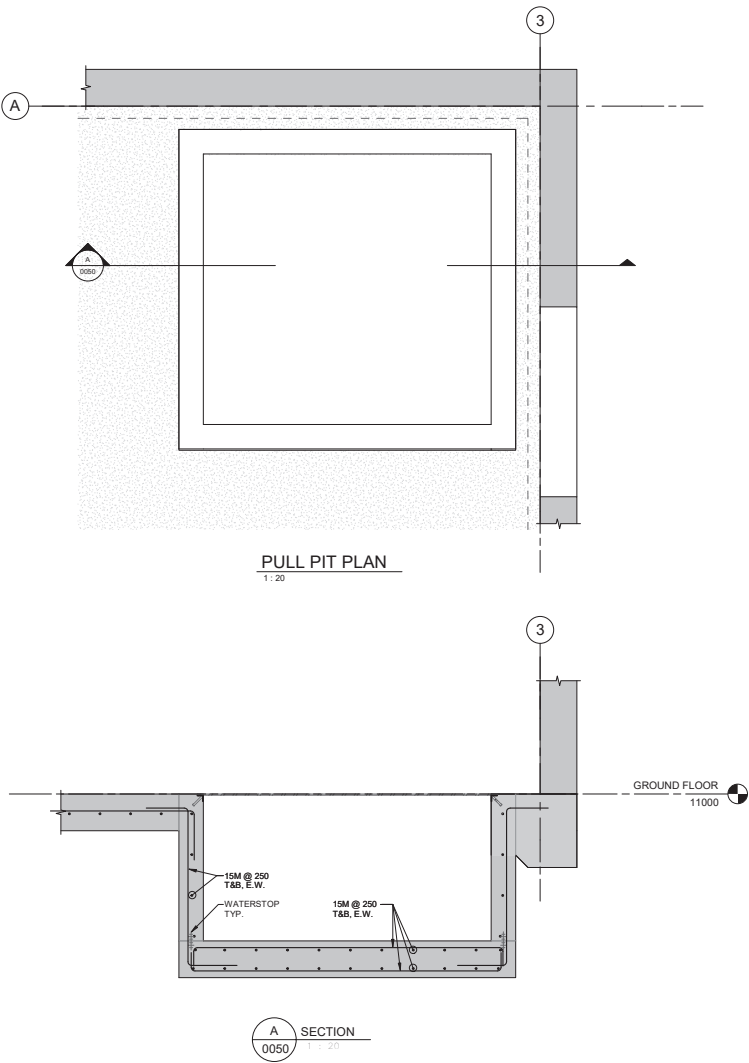
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Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franclarchitecture.com

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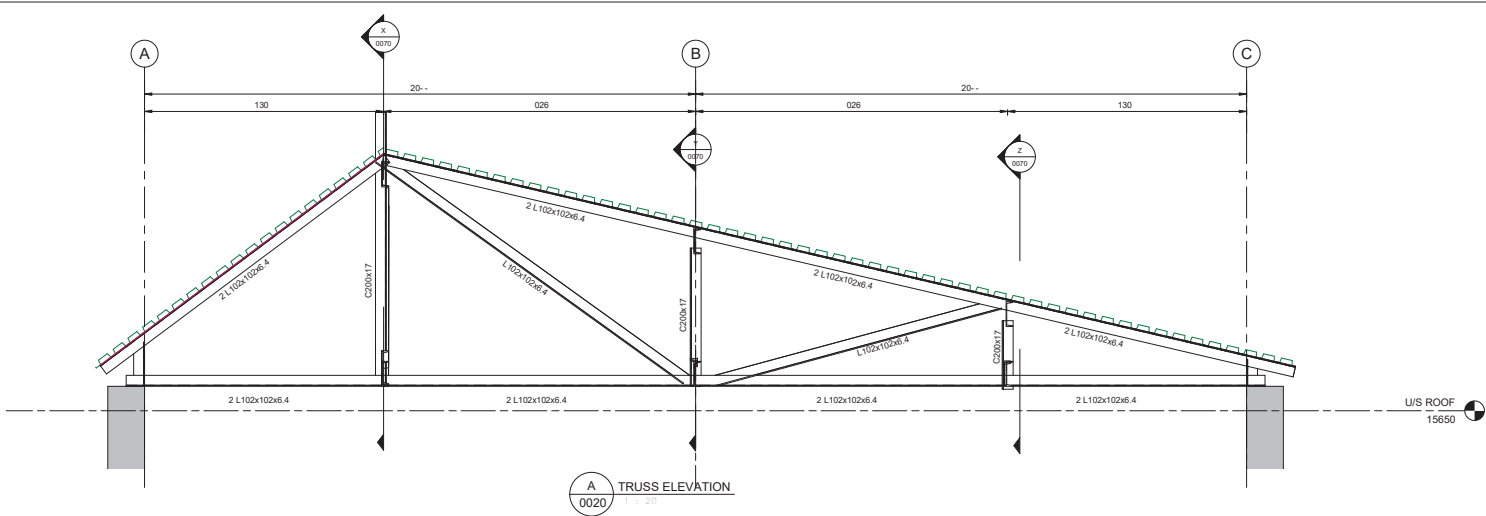
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www.snc-lavalin.com

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www.snc-lavalin.com

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604 688 3252 tel
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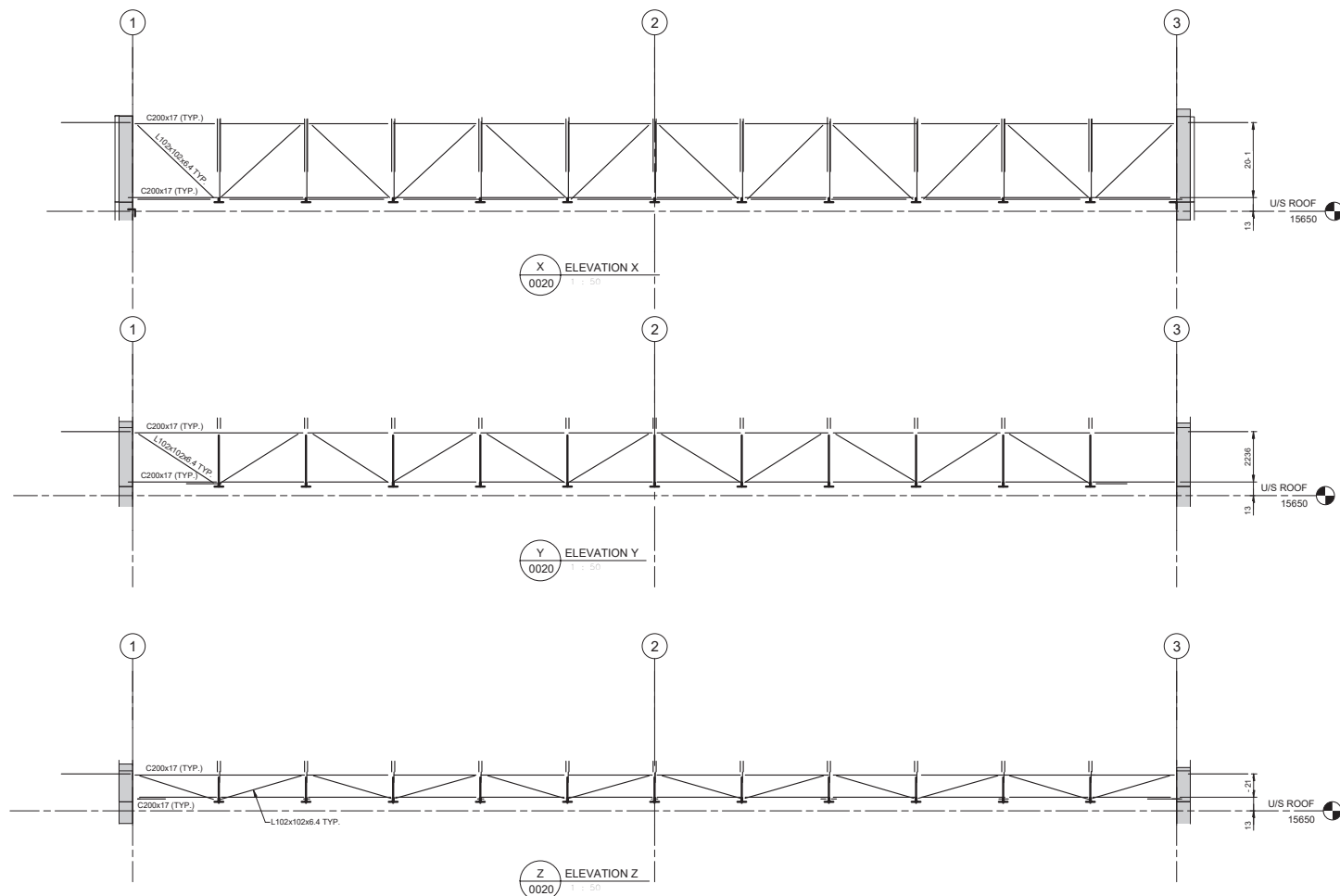
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PROPULSION
POWER
UPGRADES
PORT MOODY

DRAWING INFORMATION
DRAWN BY A.CHEUNG
DESIGNED BY C.FRANCIS
CHECKED BY M.HAN
APPROVED BY K.BRIGNALL

CONTRACT NUMBER
PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE
ROOF TRUSS ELEVATIONS AND
DETAILS - SHEET 1

DRAWING NUMBER
193028 - 03 -1000- STMCZ- SL- SG- 0060-
A



ISSUE/REVISION						REFERENCES	
A	3	'22	JV	03% BUMP BESSIN		WZ	
4A	3	'85.3	CF	13% BUMP BESSIN		KM	
BR	DATE	M'		DESCRIB TION		A44R	



CONSULTANTS

ELECTRICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

MECHANICAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

CONSULTANTS

ARCHITECTURE
FRANCL Architecture Inc.
1684 West 2nd Avenue
Vancouver, BC V6J 1H4
604 688 3252 tel
www.franciararchitecture.com

STRUCTURAL
SNC-Lavalin
Suite 1300 - 3777 Kingsway Street
Burnaby, B.C. V5H 3Z7
604 662-3555 tel 604 662-7688 fax
www.snc-lavalin.com

REGISTRATION

KEY PLAN

PROJECT

EMUP PROPULSION POWER UPGRADES

PORT MOODY

DRAWING INFORMATION	
DRAWN BY	A.CHEUNG
DESIGNED BY	C.FRANCIS
CHECKED BY	M.HAN
APPROVED BY	K.BRIGNALL

CONTRACT NUMBER

PROJECT NUMBER
193028 (SNC #675448)
SHEET TITLE

ROOF TRUSS ELEVATIONS AND
DETAILS - SHEET 2

DRAWING NUMBER

193028 - 03 -1000- STMCZ- SL- SG- 0070-
A

Appendix II




Borehole/CPT Logs





UNIT	LAYER	LITHOLOGIC UNIT	DESCRIPTION	INFERRED CONSISTENCY
	Fill	-	Varies	Varies
	Debris fan, marine and shoreline deposits	Salish Sediments	Sand with variable silt, gravel, occasional cobbles, organic layers and wood debris	Loose to compact
	Marine deposits	Capilano Sediments	Silt and clay with lenses of sand and gravel; may contain organic layers and wood debris	Soft to firm
			Interlayered silt and sand with variable gravel; may contain organic layers and wood debris	Compact to dense
	Glaciomarine deposits	Capilano Sediments	Sand and silt with variable clay, gravel and occasional cobbles / boulders	Compact to very dense
			Sand and gravel with occasional cobbles / boulders	Compact to very dense
	Till-like deposits	Vashon Sediments	Sand and silt with variable gravel and occasional cobbles / boulders; may contain water-bearing lenses of sand and gravel	Very dense

- Sonic Testhole
- Auger Testhole
- Cone Penetration Testhole
- Mud Rotary Testhole
- Becker Hammer Testhole

-  ODEX Testhole
-  Previous Testhole (position approx.)
-  Piezometric Level

(1) Figures derived from CADD files provided from the Golder Associates Ltd. report submitted to CH2M Hill Canada Ltd. and the ELRT project entitled "ELRT Project - Surface Alignment Fault Geotechnical Report", dated January 09, 2013. The inferred stratigraphy shown on these figures is based solely on EBA's own evaluation of the available testhole information.

(2) Stratigraphy is known only at the locations of the testholes. Actual geology and stratigraphy between testholes may vary from that shown on this drawing.

(3) Stratigraphy in testhole logs at locations offset from the alignment centerline may vary from that shown on the profile.

(4) Lithologic unit references from GSC Map 1484A.

HORIZONTAL SCALE 1:2000
20 0 20 40 80

VERTICAL SCALE 1:500
5 0 5 10 20



EGRT CONSTRUCTION



eba
A TETRA TECH COMPANY

**EVERGREEN LINE
RAPID TRANSIT PROJECT**

SECTION 330 - AT-GRADE GUIDEWAY
TESTHOLE LOCATION AND SOIL PROFILE

PROJECT NO.	V13103058
OFFICE	EBA-VANC

	DWN	
	RL	
	DATE	
	Jan 24, 201	

Figure 2b

PROJECT No.: 08-1411-0115

RECORD OF BOREHOLE: BH09-305

SHEET 1 OF 4

LOCATION: 2708 & 2710 Clarke Street, Port Moody

N: 5458164.77 E: 510899.29

Survey Provided by: Mason Surveys Inc.

DRILLING DATE: July 2 to 3, 2009

DRILLING CONTRACTOR: Foundex Explorations Ltd.

DATUM: Local

SAMPLER HAMMER, 64kg; DROP, 762mm

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE		SAMPLES		DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m				HYDRAULIC CONDUCTIVITY, k, cm/s				ADDITIONAL LAB. TESTING	STANDPIPE INSTALLATION AND WATER LEVELS										
		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	BLOWS/0.3m	SHEAR STRENGTH				WATER CONTENT PERCENT													
								20		40		60				80		10 ⁻⁶		10 ⁻⁵		10 ⁻⁴		10 ⁻³	
								Cu, kPa		nat V. rem V.		+ ⊕				Q - ● U - ○		Wp		W		WI		NP - Non-Plastic	
								20	40	60	80	20	40	60	80										
0		Ground Surface		12.12																					
		DRAIN ROCK. [FILL]		0.05																					
	Vacuumed	Inferred, moist, dark brown, silty SAND, some gravel, contains cobbles and organics/topsoil. [FILL]			1	GS							○			○									
1				10.90	2	GS							○			○									
				1.22																					
2		Inferred, moist, brown, silty SAND, some gravel, contains cobbles, rootlets and topsoil.																							
				9.68																					
				2.44																					
3		Inferred, loose, moist to wet, brown to grey SAND, some silt and gravel, contains cobbles.																							
	HT-1000 Truck Mounted Rotary Drill Rig Mud Rotary			8.31	3	51 DO	5						○												
				3.81																					
4					4	51 DO	23						○			M									
5			- at 4.88m, cobble.																						
		- at 5.18m, cobble.																							
		Compact, moist to wet, brown to grey, gravelly SAND, some silt, contains silt layers (25 mm thick) and cobbles.																							
				5.11																					
				7.01	6	51 DO	12						○			M									
8		Compact to loose, wet, grey, silty SAND to sandy SILT, trace gravel.																							
				3.58																					
				8.53																					
9		- at 8.53 to 9.45m, trace black organics.																							
		Loose, wet, grey, silty SAND to sandy SILT, trace clay.											○												
					7	51 DO	7																		
10																									
		CONTINUED NEXT PAGE																							

DEPTH SCALE

1 : 50



LOGGED: L.W.

CHECKED: W.L.

Project Co-ordinates to UTM NAD 83 Zone 10 Conversion: Project North * 0.999596150; Project East * 0.999596150

PROJECT No.: 08-1411-0115

RECORD OF BOREHOLE: BH09-305

SHEET 2 OF 4

LOCATION: 2708 & 2710 Clarke Street, Port Moody

N: 5458164.77 E: 510899.29

Survey Provided by: Mason Surveys Inc.

DRILLING DATE: July 2 to 3, 2009

DRILLING CONTRACTOR: Foundex Explorations Ltd.

DATUM: Local

SAMPLER HAMMER, 64kg; DROP, 762mm

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE		SAMPLES		DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m				HYDRAULIC CONDUCTIVITY, k, cm/s				ADDITIONAL LAB. TESTING	STANDPIPE INSTALLATION AND WATER LEVELS		
		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	BLOWS/0.3m	SHEAR STRENGTH				WATER CONTENT PERCENT					
								20 40 60 80		nat V. + Q - rem V. ⊕ ⊖ U - ○		10 ⁻⁶ 10 ⁻⁵ 10 ⁻⁴ 10 ⁻³					
								SHEAR STRENGTH Cu, kPa				Wp — ○ — Wl NP - Non-Plastic					
							20 40 60 80										
10	HT-1000 Truck Mounted Rotary Drill Rig Mud Rotary	Soft to firm, wet, grey SILTY CLAY to CLAY, contains sandy silt and silty sand layers (25 to 50mm).		-2.06 10.08	8	51 DO WH									MH		
11																	
12					9A	76 TO											
12					9B	51 DO	6										
13		Firm, wet, grey CLAYEY SILT, some fine sand, contains soft, grey, silty clay and silty sand layers.		-0.99 13.11	10	76 TO									H, C, LV		
14																	
15					11	51 DO	5										
16																	
17		Loose to compact, wet, grey, silty SAND to sandy SILT, trace gravel.		-4.65 16.76	12	51 DO	14										
18																	
19		Compact to dense, wet, grey SAND and GRAVEL, trace silt, contains cobbles. - at 19.50m, cobble.		-6.32 18.44	13A	51 DO	8/150mm										
20					13B	51 DO	46										
		CONTINUED NEXT PAGE															

DEPTH SCALE

1 : 50



LOGGED: L.W.

CHECKED: W.L.

Project Co-ordinates to UTM NAD 83 Zone 10 Conversion: Project North * 0.999596150; Project East * 0.999596150

File:Z:\ACTIVE\YEAR 2008\141108-1411-0115 (EVERGREEN PROJECT)\GINT08-1411-0115 FACTUAL REPORT GPJ Output Form\BOREHOLE (AUTO) Template\BC REGION TEMPLATE BETA 1.GDT Library\BC REGION LIBRARY GLE KCapes 4/10/10

PROJECT No.: 08-1411-0115

RECORD OF BOREHOLE: BH09-305

SHEET 3 OF 4

LOCATION: 2708 & 2710 Clarke Street, Port Moody

N: 5458164.77 E: 510899.29

Survey Provided by: Mason Surveys Inc.

DRILLING DATE: July 2 to 3, 2009

DRILLING CONTRACTOR: Foundex Explorations Ltd.

DATUM: Local

SAMPLER HAMMER, 64kg; DROP, 762mm

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE		SAMPLES			DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m				HYDRAULIC CONDUCTIVITY, k, cm/s				ADDITIONAL LAB. TESTING	STANDPIPE INSTALLATION AND WATER LEVELS	
		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	BLOWS/0.3m										
								SHEAR STRENGTH Cu, kPa		nat V. + Q - ● rem V. ⊕ U - ○		WATER CONTENT PERCENT Wp ———— W WI NP - Non-Plastic					
								20	40	60	80	10 ⁻⁶	10 ⁻⁵	10 ⁻⁴			10 ⁻³
20	HT-1000 Truck Mounted Rotary Drill Rig Mud Rotary	- at 20.72m, cobble.															
21				14	51 DO	48										M	
22		Compact to dense, wet, grey SAND and GRAVEL, trace silt, contains cobbles. (continued)															
23		- at 22.25m, cobble.															
24				-10.74 22.86													
25					15	51 DO	52									M	
26		- at 25.60 to 26.21m, sand and gravel layer. Dense, moist to wet, grey SAND, trace gravel and silt.															
27					16	51 DO	52										
28																	
29					-16.84 28.96												
30		Dense, moist to wet, grey SAND and GRAVEL, some silt, contains cobbles.															
		CONTINUED NEXT PAGE				17	51	55									

DEPTH SCALE

1 : 50



LOGGED: L.W.

CHECKED: W.L.

Project Co-ordinates to UTM NAD 83 Zone 10 Conversion: Project North * 0.999596150; Project East * 0.999596150

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PROJECT No.: 08-1411-0115

RECORD OF BOREHOLE: BH09-305

SHEET 4 OF 4

LOCATION: 2708 & 2710 Clarke Street, Port Moody

N: 5458164.77 E: 510899.29

Survey Provided by: Mason Surveys Inc.

DRILLING DATE: July 2 to 3, 2009

DATUM: Local

DRILLING CONTRACTOR: Foundex Explorations Ltd.

SAMPLER HAMMER, 64kg; DROP, 762mm

DEPTH SCALE METRES	BORING METHOD	SOIL PROFILE		SAMPLES		DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m				HYDRAULIC CONDUCTIVITY, k, cm/s				STANDPIPE INSTALLATION AND WATER LEVELS	
		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	BLOWS/0.3m	SHEAR STRENGTH Cu, kPa		nat V. + Q - ● rem V. ⊕ U - ○		WATER CONTENT PERCENT Wp ———— WI			
								20	40	60	80	10 ⁻⁶	10 ⁻⁵		10 ⁻⁴
30	HT-1000 Truck Mounted Rotary Drill Rig Mud Rotary	- at 30.18m, cobble.			17	DO 51 DO	55								
31		Dense, moist to wet, grey SAND and GRAVEL, some silt, contains cobbles. (continued)													
32		- at 32.00m, cobble.			-19.89 32.00										
33		Very dense, moist, grey, sandy SILT, some gravel, contains cobbles.			18	51 DO	>100								
34		End of Borehole.													Completed borehole grouted to surface.
35															
36															
37															
38															
39															
40															

DEPTH SCALE

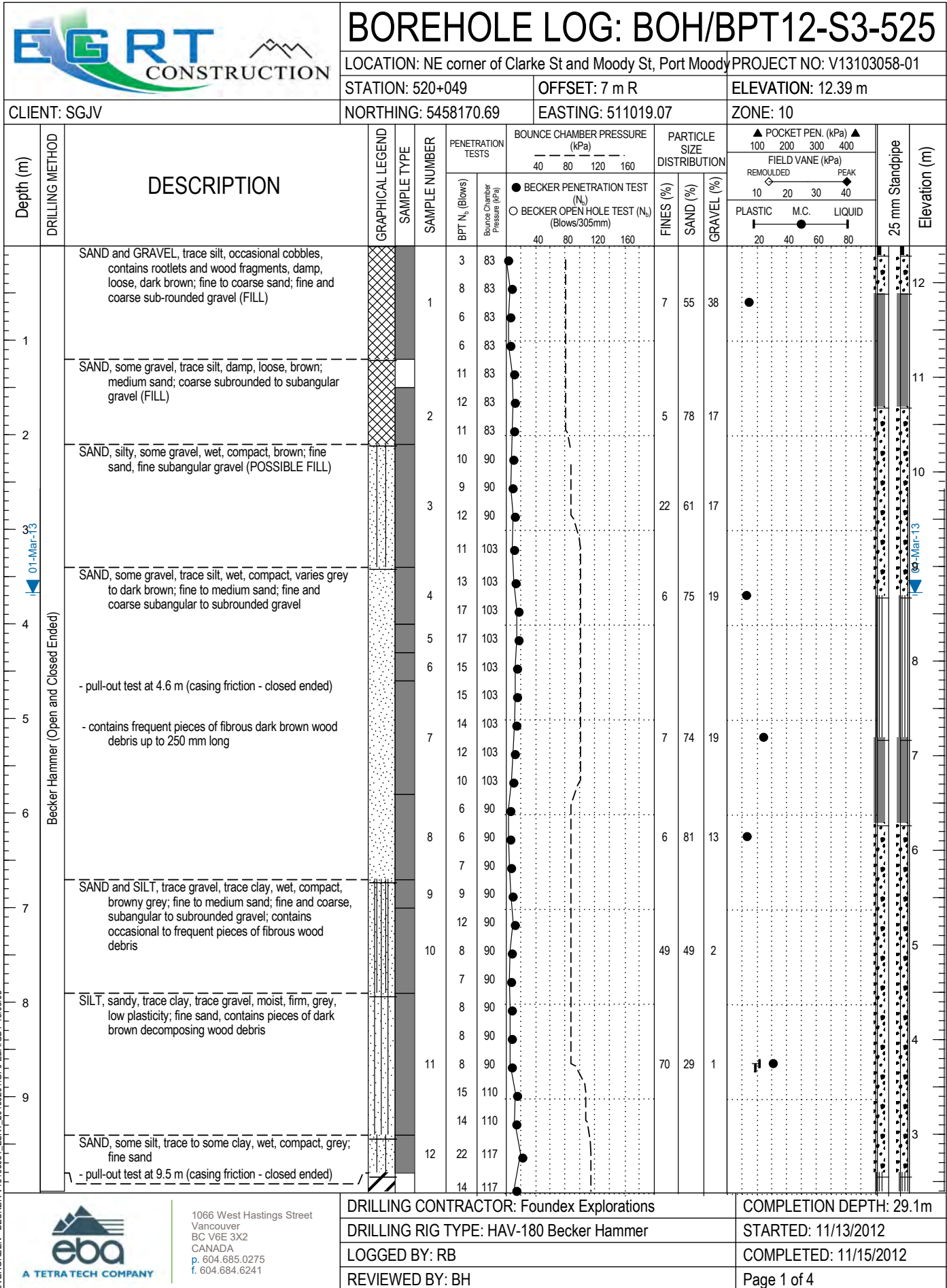
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LOGGED: L.W.

CHECKED: W.L.

Project Co-ordinates to UTM NAD 83 Zone 10 Conversion: Project North * 0.999596150; Project East * 0.999596150





BOREHOLE LOG: BOH/BPT12-S3-525

LOCATION: NE corner of Clarke St and Moody St, Port Moody PROJECT NO: V13103058-01

STATION: 520+049

OFFSET: 7 m R

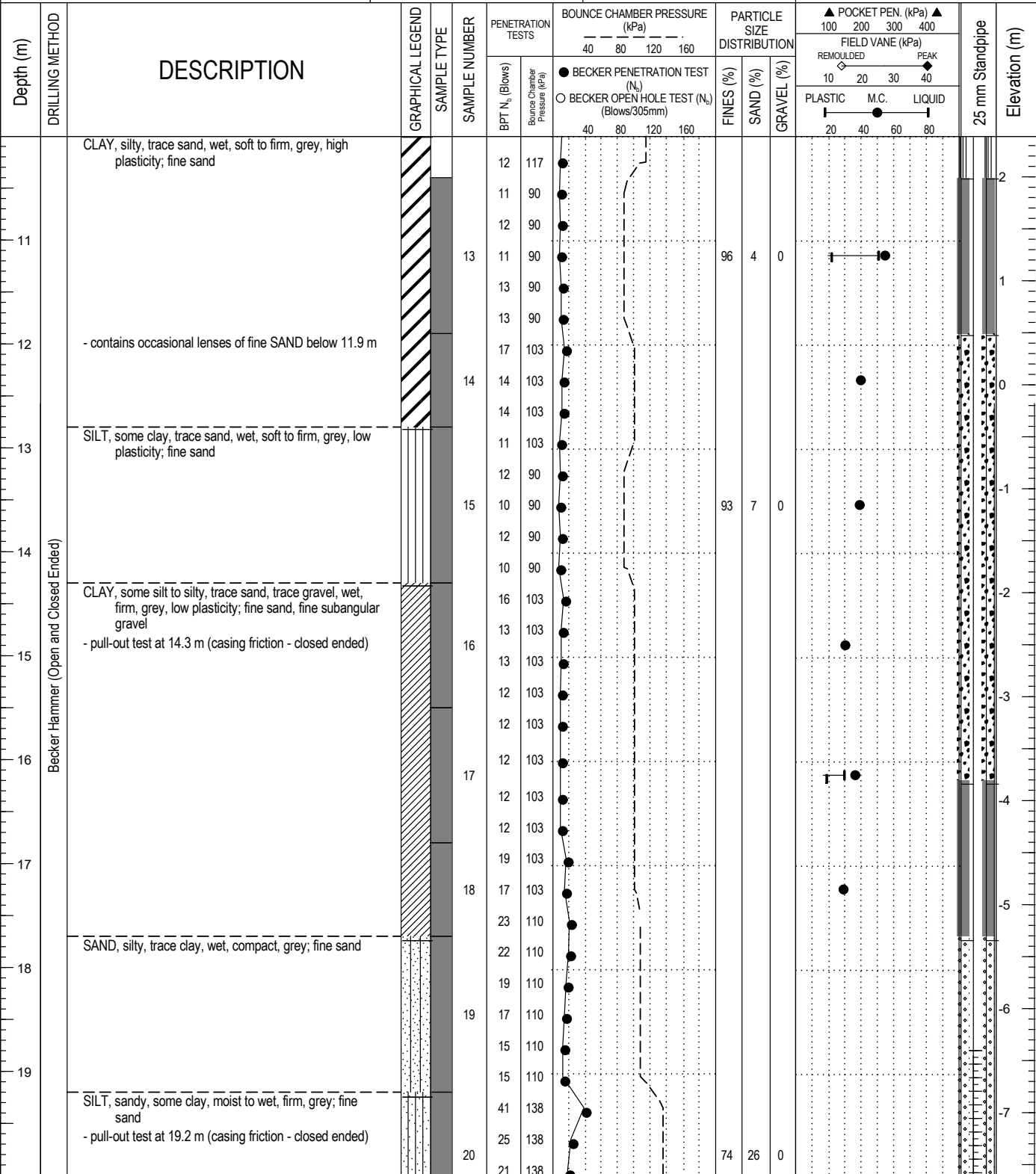
ELEVATION: 12.39 m

CLIENT: SGJV

NORTHING: 5458170.69

EASTING: 511019.07

ZONE: 10


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 BC V6E 3X2
 CANADA
 p. 604.685.0275
 f. 604.684.6241

DRILLING CONTRACTOR: Foundex Explorations

DRILLING RIG TYPE: HAV-180 Becker Hammer

LOGGED BY: RB

REVIEWED BY: BH

COMPLETION DEPTH: 29.1m

STARTED: 11/13/2012

COMPLETED: 11/15/2012

Page 2 of 4

EGRT CONSTRUCTION		BOREHOLE LOG: BOH/BPT12-S3-525										
CLIENT: SGJV		LOCATION: NE corner of Clarke St and Moody St, Port Moody				PROJECT NO: V13103058-01						
STATION: 520+049		OFFSET: 7 m R				ELEVATION: 12.39 m						
NORTHING: 5458170.69		EASTING: 511019.07				ZONE: 10						
Depth (m)	DRILLING METHOD	DESCRIPTION	GRAPHICAL LEGEND	SAMPLE TYPE	SAMPLE NUMBER	PENETRATION TESTS	BOUNCE CHAMBER PRESSURE (kPa)	PARTICLE SIZE DISTRIBUTION	POCKET PEN. (kPa)	FIELD VANE (kPa)	25 mm Standpipe	Elevation (m)
						BPT N ₆₀ (Blows) Bounce Chamber Pressure (kPa)	40 80 120 160 BECKER PENETRATION TEST (N ₆₀) BECKER OPEN HOLE TEST (N ₆₀) (Blows/305mm)	FINES (%) SAND (%) GRAVEL (%)	100 200 300 400 REMOULDED PEAK PLASTIC M.C. LIQUID			
21		- trace clay below 20.4 m			21	24 138						-8
22		End of Becker open hole drilling at 21.6 m depth (target depth reached). - Standpipe piezometer installed in borehole as indicated. - pull-out test at 22.3 m (casing friction - closed ended)				21 138						
23						27 138						
24						24 124						
25						20 117						
26						23 117						
27						24 117						
28						38 131						
29						34 138						
30						29 159						
31						37 159						
32						91 159						
33						91 159						
34						75 159						
35						67 152						
36						60 152						
37						59 152						
38						63 145						
39						52 145						
40						40 145						
41						37 145						
42						53 145						
43						83 159						
44						55 152						
45						29 138						
46						31 138						
47						65 145						
48						66 145						
49						166 145						
50						204 159						
29.1	Becker Hammer (Open and Closed Ended)	End of Testhole at 29.1 m depth (refusal). - Standpipe piezometer installed in testhole as indicated. - Estimates of the soil consistency were determined from BPT blowcounts, drill rig performance, and visual classification of recovered samples. These										

LOCATION: NE corner of Clarke St and Moody St, Port Moody PROJECT NO: V13103058-01

OFFSET: 7 m R

ZONE: 10

- estimates are based on engineering judgement and are subjective.
- Moisture content values for cohesionless soils may not be representative of in-situ conditions due to drilling disturbance.
- Fines content values may not be representative of in-situ conditions due to drilling disturbance.
- Reported blowcounts are uncorrected field values.
- Becker open blow count values should not be relied upon for design and are provided for relative comparison only.



1066 West Hastings Street
Vancouver
BC V6E 3X2
CANADA
p. 604.685.0275
f. 604.684.6241

COMPLETION DEPTH: 29.1m

STARTED: 11/13/2012

COMPLETED: 11/15/2012

Page 4 of 4

EGRT CONSTRUCTION		BOREHOLE LOG: BH12-03								
CLIENT: SNC-Lavalin		LOCATION: Clarke St. @ Grant St., Port Moody			PROJECT NO: V13103058-01					
		STATION: 519+815		OFFSET: 15 m R		ELEVATION: 11.7 m				
		NORTHING: 5458152		EASTING: 510787		ZONE: 10				
Depth (m)	DRILLING METHOD	DESCRIPTION	PENETRATION TESTS		BOUNCE CHAMBER PRESSURE (kPa)	PARTICLE SIZE DISTRIBUTION			Backfill	Elevation (m)
			BPT N_6 (Blows)	Bounce Chamber Pressure (kPa)	40 80 120 160	FINES (%)	SAND (%)	GRAVEL (%)		
					● BECKER PENETRATION TEST (N_6) ○ BECKER OPEN HOLE TEST (N_6) (Blows/305mm)				▲ POCKET PEN. (kPa) ▲ 100 200 300 400 FIELD VANE (kPa) REMOULDED PEAK 10 20 30 40 PLASTIC M.C. LIQUID 10 20 30 40	
1	Hydro-Vac	NO SAMPLE RECOVERY - VACUUM EXCAVATED								11
2										10
3										9
4		Becker hammer drilling (closed ended) to 32.0 m depth - no soil samples recovered	2	110						8
5			9	110						7
6			12	110						6
7			15	110						5
8			12	110						4
9			11	110						3
10			12	110						2
11			8	79						
12			6	79						
13			8	79						
14			6	79						
15			9	79						
16			14	79						
17			17	90						
18			19	93						
19			17	93						
20			13	93						
21			9	79						
22			8	79						
23			9	79						
24			17	93						
25			15	107						
26			20	107						



BOREHOLE LOG: BH12-03

LOCATION: Clarke St. @ Grant St., Port Moody

PROJECT NO: V13103058-01

STATION: 519+815

OFFSET: 15 m R

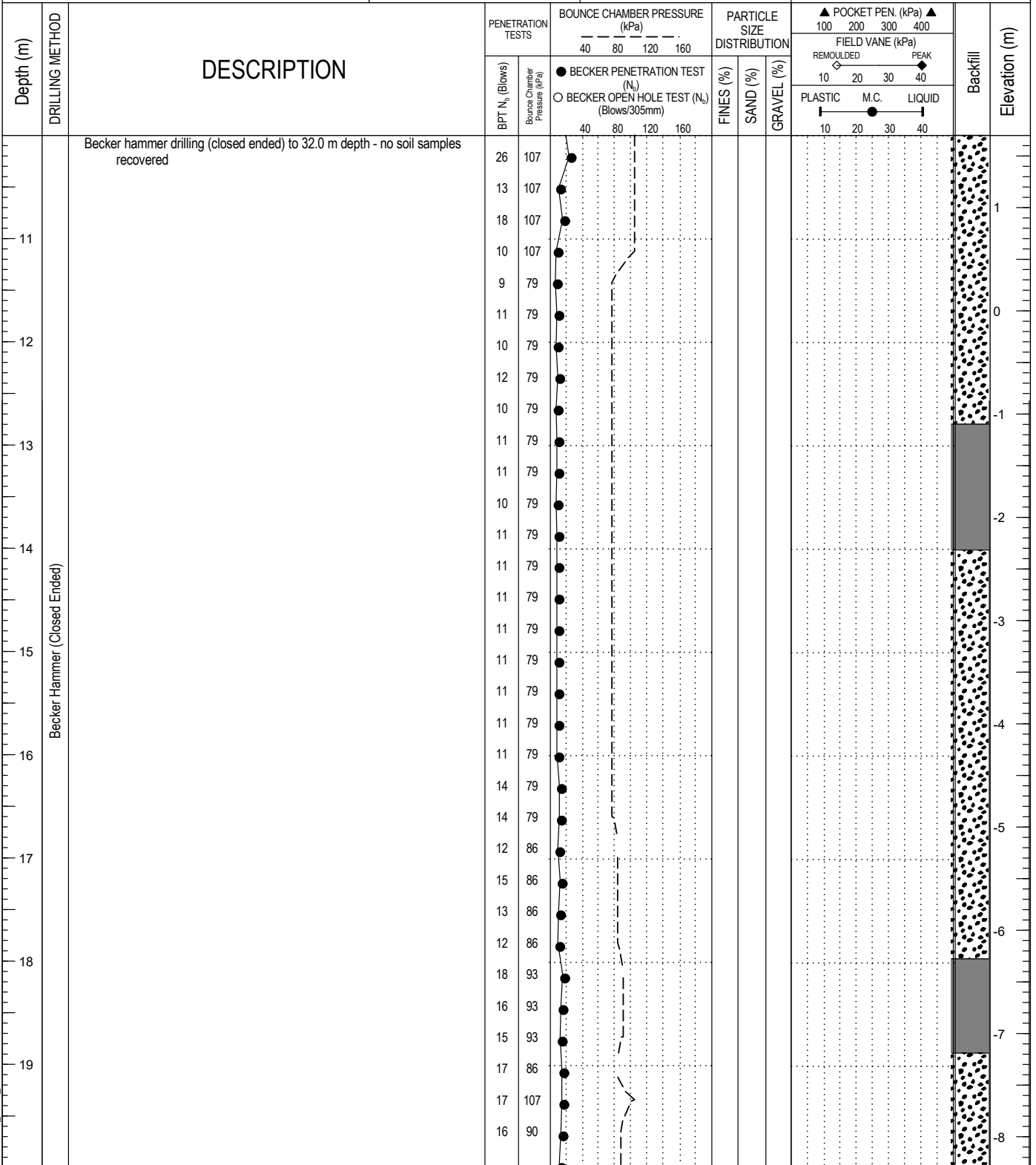
ELEVATION: 11.7 m

CLIENT: SNC-Lavalin

NORTHING: 5458152

EASTING: 510787

ZONE: 10



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CANADA
p. 604.685.0275
f. 604.684.6241

DRILLING CONTRACTOR: Foundex Explorations

COMPLETION DEPTH: 32m

DRILLING RIG TYPE: HAV-180 Becker Hammer

STARTED: 4/24/2012

LOGGED BY: AT

COMPLETED: 4/24/2012

REVIEWED BY: BH

Page 2 of 4



BOREHOLE LOG: BH12-03

LOCATION: Clarke St. @ Grant St., Port Moody

PROJECT NO: V13103058-01

STATION: 519+815

OFFSET: 15 m R

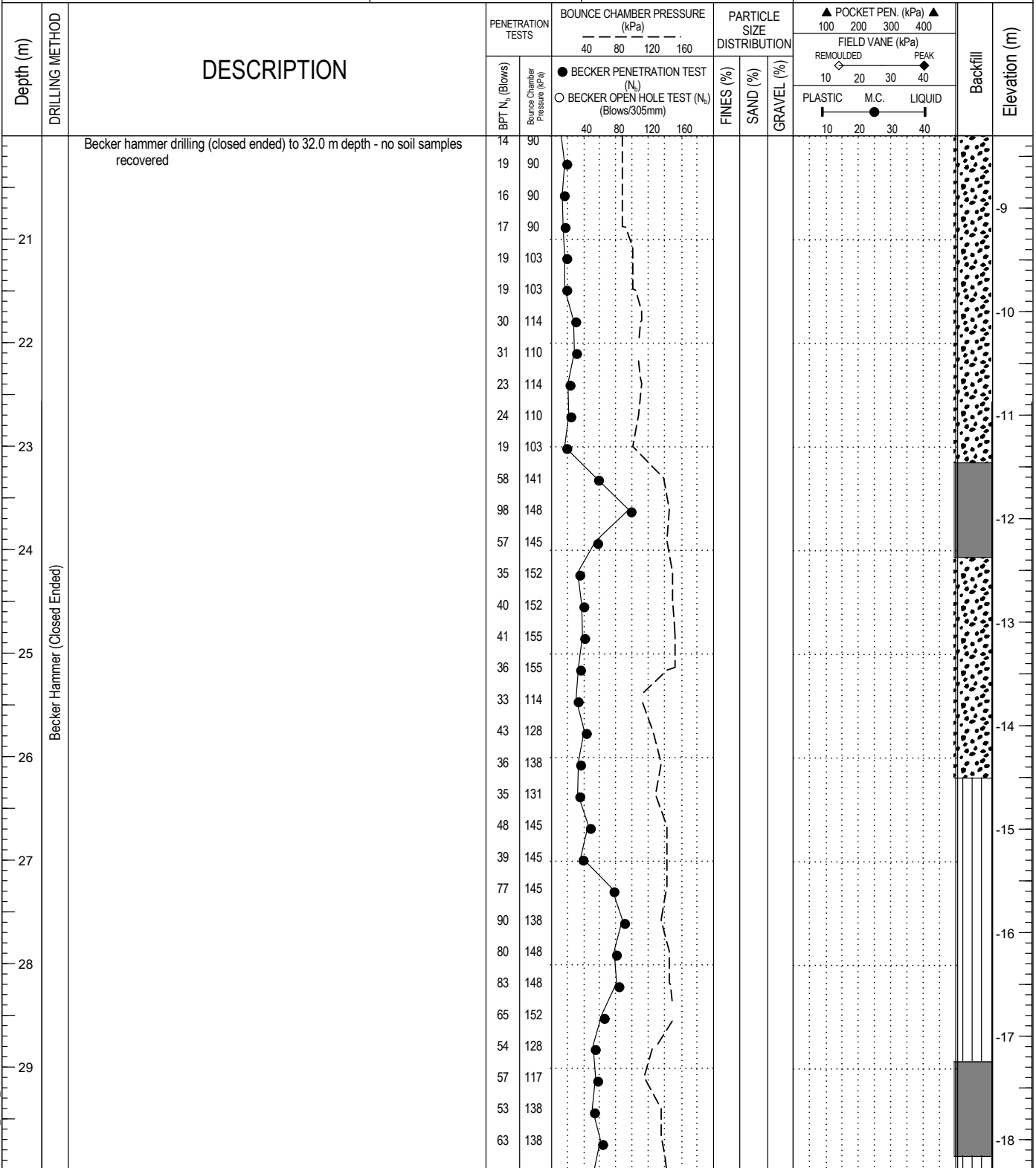
ELEVATION: 11.7 m

CLIENT: SNC-Lavalin

NORTHING: 5458152

EASTING: 510787

ZONE: 10



EVERGREEN - BECKER V13103031 ELRT 20130201.GPJ EBA.GDT 13/03/08



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Vancouver
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CANADA
p. 604.685.0275
f. 604.684.6241

DRILLING CONTRACTOR: Foundex Explorations

COMPLETION DEPTH: 32m

DRILLING RIG TYPE: HAV-180 Becker Hammer

STARTED: 4/24/2012

LOGGED BY: AT

COMPLETED: 4/24/2012

REVIEWED BY: BH

Page 3 of 4



BOREHOLE LOG: BH12-03

LOCATION: Clarke St. @ Grant St., Port Moody

PROJECT NO: V13103058-01

STATION: 519+815

OFFSET: 15 m R

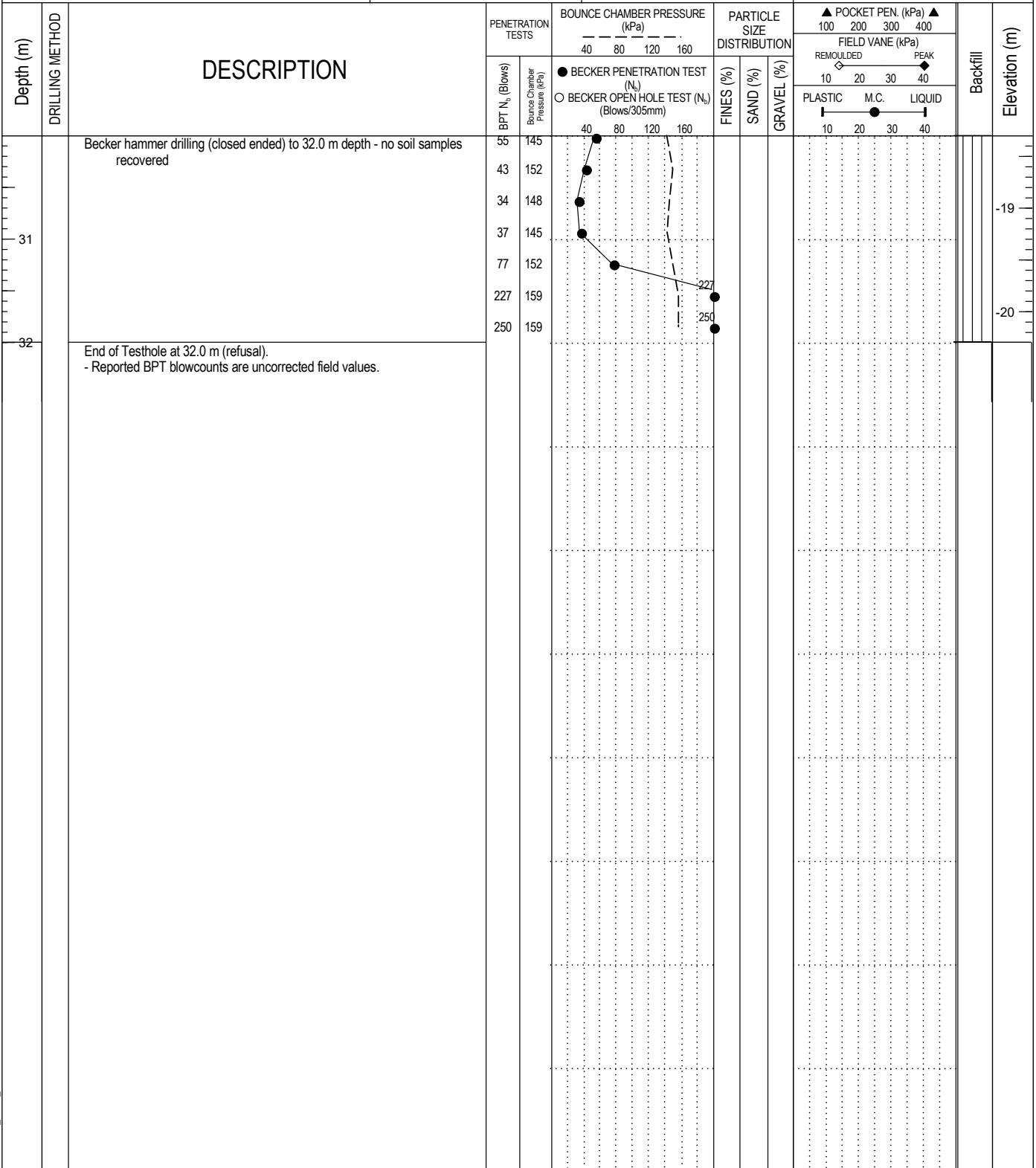
ELEVATION: 11.7 m

CLIENT: SNC-Lavalin

NORTHING: 5458152

EASTING: 510787

ZONE: 10



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CANADA
p. 604.685.0275
f. 604.684.6241

DRILLING CONTRACTOR: Foundex Explorations

COMPLETION DEPTH: 32m

DRILLING RIG TYPE: HAV-180 Becker Hammer

STARTED: 4/24/2012

LOGGED BY: AT

COMPLETED: 4/24/2012

REVIEWED BY: BH

Page 4 of 4



BOREHOLE LOG: MR12-S3-524

LOCATION: Clarke Street between Grant St and Mary St

PROJECT NO: V13103058-01

STATION: 519+780

OFFSET: 15 m R

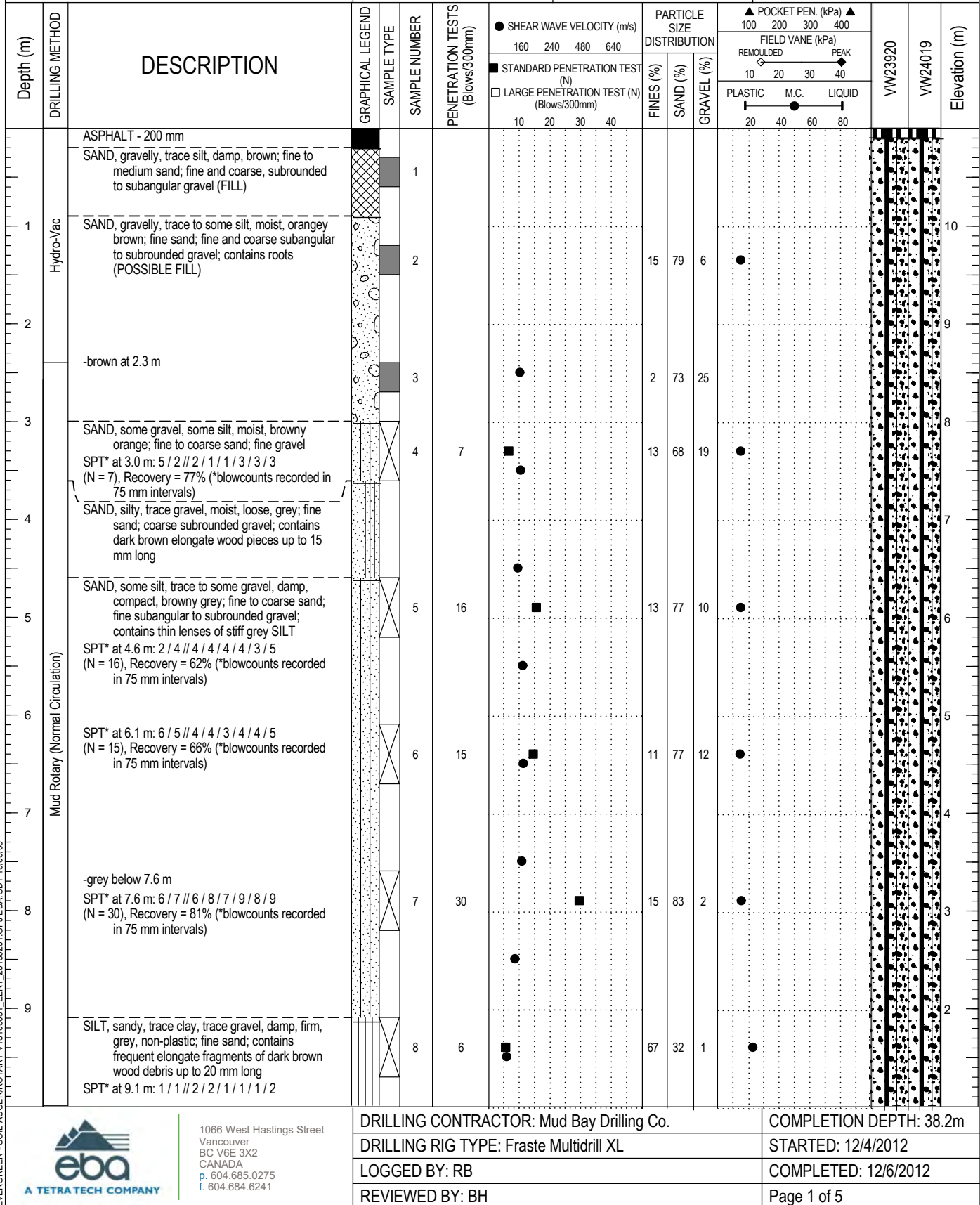
ELEVATION: 11 m

CLIENT: SGJV

NORTHING: 5458150

EASTING: 510753

ZONE: 10



EVERGREEN - SOIL AUGER/ROTARY V13103031 ELRT 20130201.GPJ EBA GDT 130308



1066 West Hastings Street
Vancouver
BC V6E 3X2
CANADA
p. 604.685.0275
f. 604.684.6241

DRILLING CONTRACTOR: Mud Bay Drilling Co.

COMPLETION DEPTH: 38.2m

DRILLING RIG TYPE: Fraste Multidrill XL


STARTED: 12/4/2012

LOGGED BY: RB

COMPLETED: 12/6/2012

REVIEWED BY: BH

Page 1 of 5

			BOREHOLE LOG: MR12-S3-524										
CLIENT: SGJV			LOCATION: Clarke Street between Grant St and Mary St				PROJECT NO: V13103058-01						
STATION: 519+780			OFFSET: 15 m R				ELEVATION: 11 m						
NORTHING: 5458150			EASTING: 510753				ZONE: 10						
Depth (m)	DRILLING METHOD	DESCRIPTION	GRAPHICAL LEGEND	SAMPLE TYPE	SAMPLE NUMBER	PENETRATION TESTS (Blows/300mm)	● SHEAR WAVE VELOCITY (m/s) 160 240 480 640	■ STANDARD PENETRATION TEST (N) □ LARGE PENETRATION TEST (N) (Blows/300mm) 10 20 30 40	PARTICLE SIZE DISTRIBUTION FINES (%) SAND (%) GRAVEL (%)	▲ POCKET PEN. (kPa) 100 200 300 400 FIELD VANE (kPa) REMOULDED PEAK 10 20 30 40 PLASTIC M.C. LIQUID 20 40 60 80	VW23920	VW24019	Elevation (m)
11	Mud Rotary (Normal Circulation)	(N = 6), Recovery = 87% (*blowcounts recorded in 75 mm intervals)											
		ORGANIC SILT, some clay to clayey, trace sand, trace gravel, trace shells, moist, firm, grey, high plasticity; fine sand, fine subangular gravel		ST1									0
12		Nilcon vane test at 11.7 m								ND			-1
		Inferred SILT, some clay, trace sand, moist, firm, grey, low plasticity		ST2									-2
13		Nilcon vane test at 13.3 m								ND			-3
		CLAY, silty, trace sand, trace gravel, moist, firm, grey, low plasticity; fine to medium sand; fine subangular gravel		ST3									-4
14		Nilcon vane test at 14.8 m								ND			-5
15													-6
16													-7
17			SILT, sandy, some clay, moist, stiff, grey, low plasticity; fine sand SPT* at 16.5 m: 9 / 6 / 4 / 2 / 2 / 1 / 2 / 2 (N = 9), Recovery = 116% (*blowcounts recorded in 75 mm intervals)		9	9							
18		SILT, some sand, some clay, damp, stiff, non-plastic, grey; fine sand SPT* at 18 m: 3 / 3 / 3 / 3 / 2 / 2 / 3 / 2 (N = 10), Recovery = 100% (*blowcounts recorded in 75 mm intervals)		10	10			84 16 0					-9
19													-10

EVERGREEN - SOIL AUGER/ROTARY V13103031 ELRT 20130201.GPJ EBA GDT 130308



1066 West Hastings Street
Vancouver
BC V6E 3X2
CANADA
p. 604.685.0275
f. 604.684.6241

DRILLING CONTRACTOR: Mud Bay Drilling Co.
DRILLING RIG TYPE: Fraste Multidril XL
LOGGED BY: RB
REVIEWED BY: BH

COMPLETION DEPTH: 38.2m
STARTED: 12/4/2012
COMPLETED: 12/6/2012
Page 2 of 5



BOREHOLE LOG: MR12-S3-524

LOCATION: Clarke Street between Grant St and Mary St

PROJECT NO: V13103058-01

STATION: 519+780

OFFSET: 15 m R

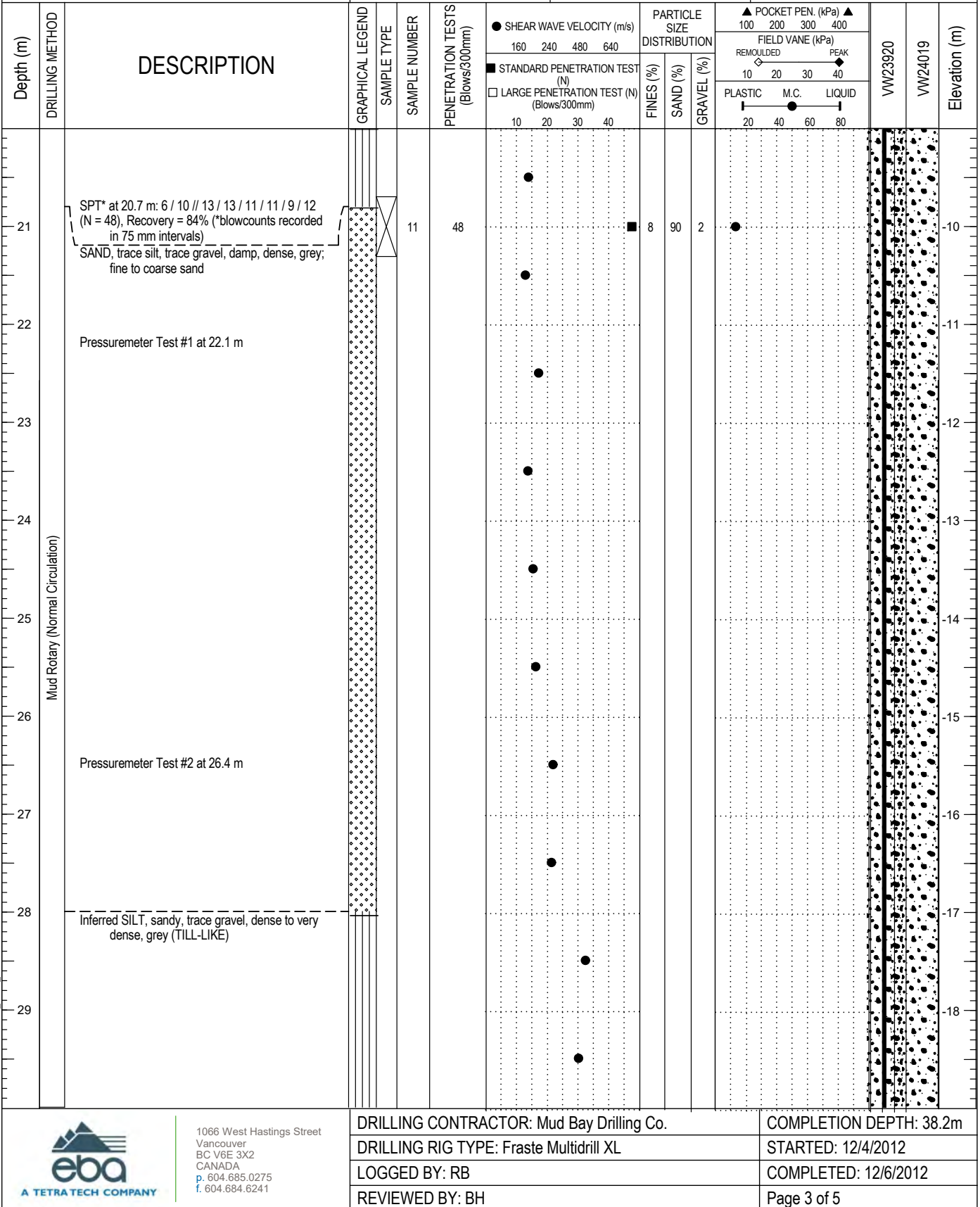
ELEVATION: 11 m

CLIENT: SGJV

NORTHING: 5458150

EASTING: 510753

ZONE: 10



EVERGREEN - SOIL AUGER/ROTARY V13103031 ELRT 20130201.GPJ EBA GDT 13/03/08


 1066 West Hastings Street
 Vancouver
 BC V6E 3X2
 CANADA
 p. 604.685.0275
 f. 604.684.6241

DRILLING CONTRACTOR: Mud Bay Drilling Co.

COMPLETION DEPTH: 38.2m

DRILLING RIG TYPE: Fraste Multidril XL

STARTED: 12/4/2012

LOGGED BY: RB

COMPLETED: 12/6/2012

REVIEWED BY: BH

Page 3 of 5



BOREHOLE LOG: MR12-S3-524

LOCATION: Clarke Street between Grant St and Mary St

PROJECT NO: V13103058-01

STATION: 519+780

OFFSET: 15 m R

ELEVATION: 11 m

CLIENT: SGJV

NORTHING: 5458150

EASTING: 510753

ZONE: 10

Depth (m)	DRILLING METHOD	DESCRIPTION	GRAPHICAL LEGEND	SAMPLE TYPE	SAMPLE NUMBER	PENETRATION TESTS (Blows/300mm)	SHEAR WAVE VELOCITY (m/s)			STANDARD PENETRATION TEST (N)			LARGE PENETRATION TEST (N)			PARTICLE SIZE DISTRIBUTION			POCKET PEN. (kPa)			FIELD VANE (kPa)	REMOULDED	PEAK	PLASTIC	M.C.	LIQUID	VW23920	VW24019	Elevation (m)
							160	240	480	640	10	20	30	40	FINES (%)	SAND (%)	GRAVEL (%)	100	200	300	400									
							SHEAR WAVE VELOCITY (m/s)			STANDARD PENETRATION TEST (N)			LARGE PENETRATION TEST (N)			PARTICLE SIZE DISTRIBUTION			POCKET PEN. (kPa)											
							SHEAR WAVE VELOCITY (m/s)			STANDARD PENETRATION TEST (N)			LARGE PENETRATION TEST (N)			PARTICLE SIZE DISTRIBUTION			POCKET PEN. (kPa)											

31	Mud Rotary (Normal Circulation)																														-20
32																															-21
33																															-22
34																															-23
35																															-24
36																															-25
37																															-26
38																															-27


SPT* at 38.1 m: 60 / 93 // - / - / - / - / -
(N > 50), Recovery = 158% (*blowcounts recorded in 75 mm intervals)

End of Testhole at 38.2 m (target depth reached).

- Testhole grouted to surface with 63.5 mm diameter Sch. 40 PVC casing and vibrating-wire piezometers installed at 12.0 and 36.4 m.

- Estimates of the soil consistency were determined from overburden stress, drill rig performance, insitu test results and visual classification of recovered samples. These

DRILLING CONTRACTOR: Mud Bay Drilling Co.												COMPLETION DEPTH: 38.2m											
DRILLING RIG TYPE: Fraste Multidrill XL												STARTED: 12/4/2012											
LOGGED BY: RB												COMPLETED: 12/6/2012											
REVIEWED BY: BH												Page 4 of 5											



A TETRA TECH COMPANY

1066 West Hastings Street
Vancouver
BC V6E 3X2
CANADA
p. 604.685.0275
f. 604.684.6241



1066 West Hastings Street
 Vancouver
 BC V6E 3X2
 CANADA
 p. 604.685.0275
 f. 604.684.6241

DRILLING CONTRACTOR: Mud Bay Drilling Co.

COMPLETION DEPTH: 38.2m

DRILLING RIG TYPE: Fraste Multidril XL

STARTED: 12/4/2012

LOGGED BY: RB

COMPLETED: 12/6/2012

REVIEWED BY: BH

Page 4 of 5

LOCATION: Clarke Street between Grant St and Mary St

STATION: 519+780

ELEVATION: 11 m

NORTHING: 5458150

EASTING: 510753

ZONE: 10

Depth (m)	DRILLING METHOD	DESCRIPTION	GRAPHICAL LEGEND	SAMPLE TYPE	SAMPLE NUMBER	PENETRATION TESTS (Blows/300mm)	SHEAR WAVE VELOCITY (m/s)				PARTICLE SIZE DISTRIBUTION			POCKET PEN. (kPa)		VW23920	VW24019	Elevation (m)
							160	240	480	640				FIELD VANE (kPa)				
											FINES (%)	SAND (%)	GRAVEL (%)	REMOULDED	PEAK			
														10	20			
							STANDARD PENETRATION TEST (N)			LARGE PENETRATION TEST (N) (Blows/300mm)			PLASTIC M.C. LIQUID					
							10 20 30 40						20 40 60 80					
		estimates are based on engineering judgement and are subjective. - SPT blowcounts were recorded in 75 mm (3") intervals. - Reported blowcounts are uncorrected field values. - SPT recovery is based on the length of the recovered sample compared to the distance driven. - Moisture content values for cohesionless soils may not be representative of in-situ conditions due to drilling disturbance. - Borehole location, collar elevation and chainage/offset are approximate.																


GRAHAM

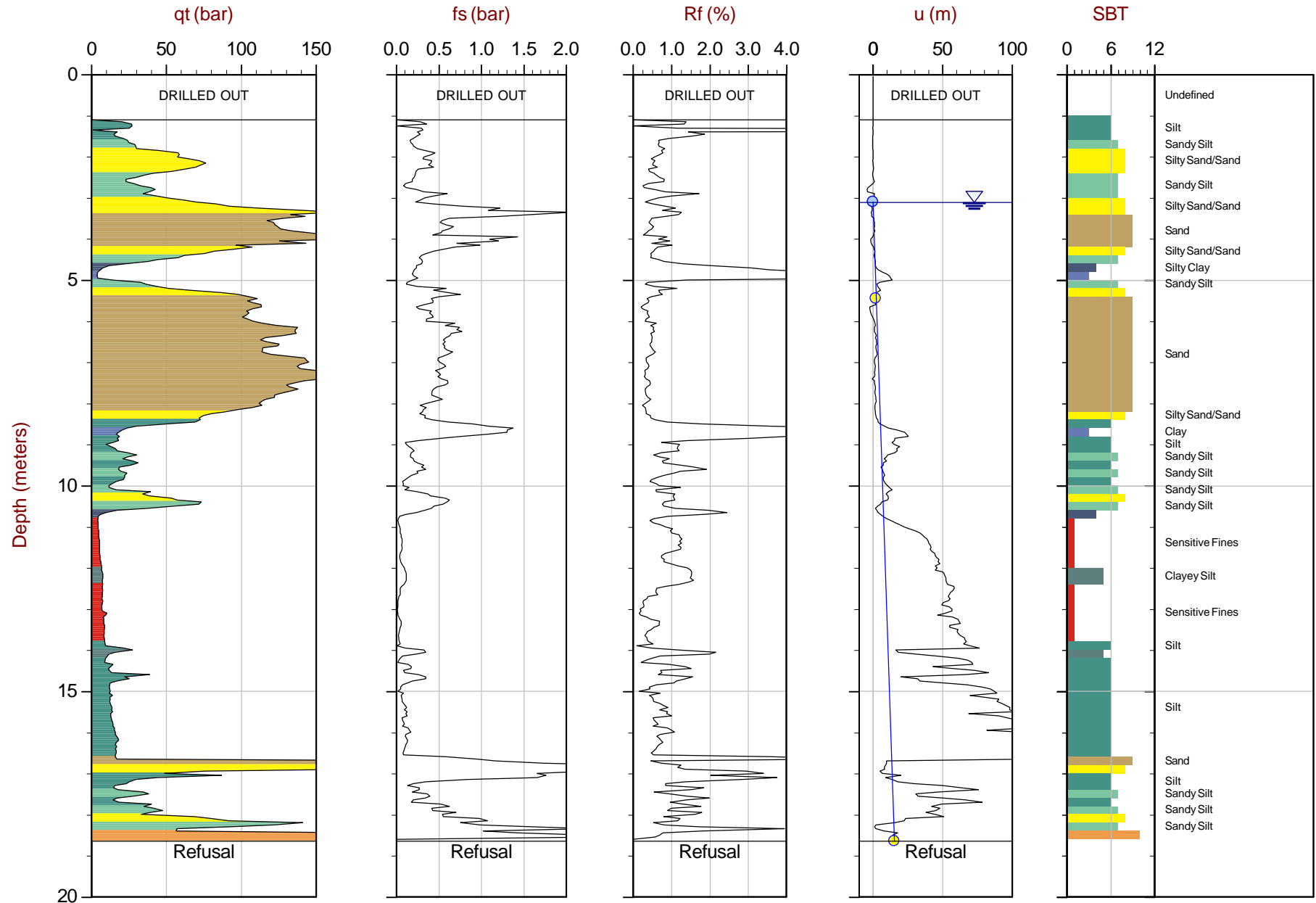
Job No: 13-02006

Date: 05:01:13 12:47

Site: Clarke St., Port Moody, BC

Sounding: CPT13-619+800

Cone: 351:T1500F15U500



Max Depth: 18.650 m / 61.19 ft
 Depth Inc: 0.050 m / 0.164 ft
 Avg Int: 0.200 m

File: 13-02006_CP619800.COR
 Unit Wt: SBT Chart Soil Zones
 Overplot Item: ● Ueq ● Assumed Ueq — Hydrostatic Line

SBT: Lunne, Robertson and Powell, 1997
 Coords: UTM10N N: 5458361m E: 510668m
 Page No: 1 of 1

The reported coordinates were acquired from hand-held GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.


GRAHAM

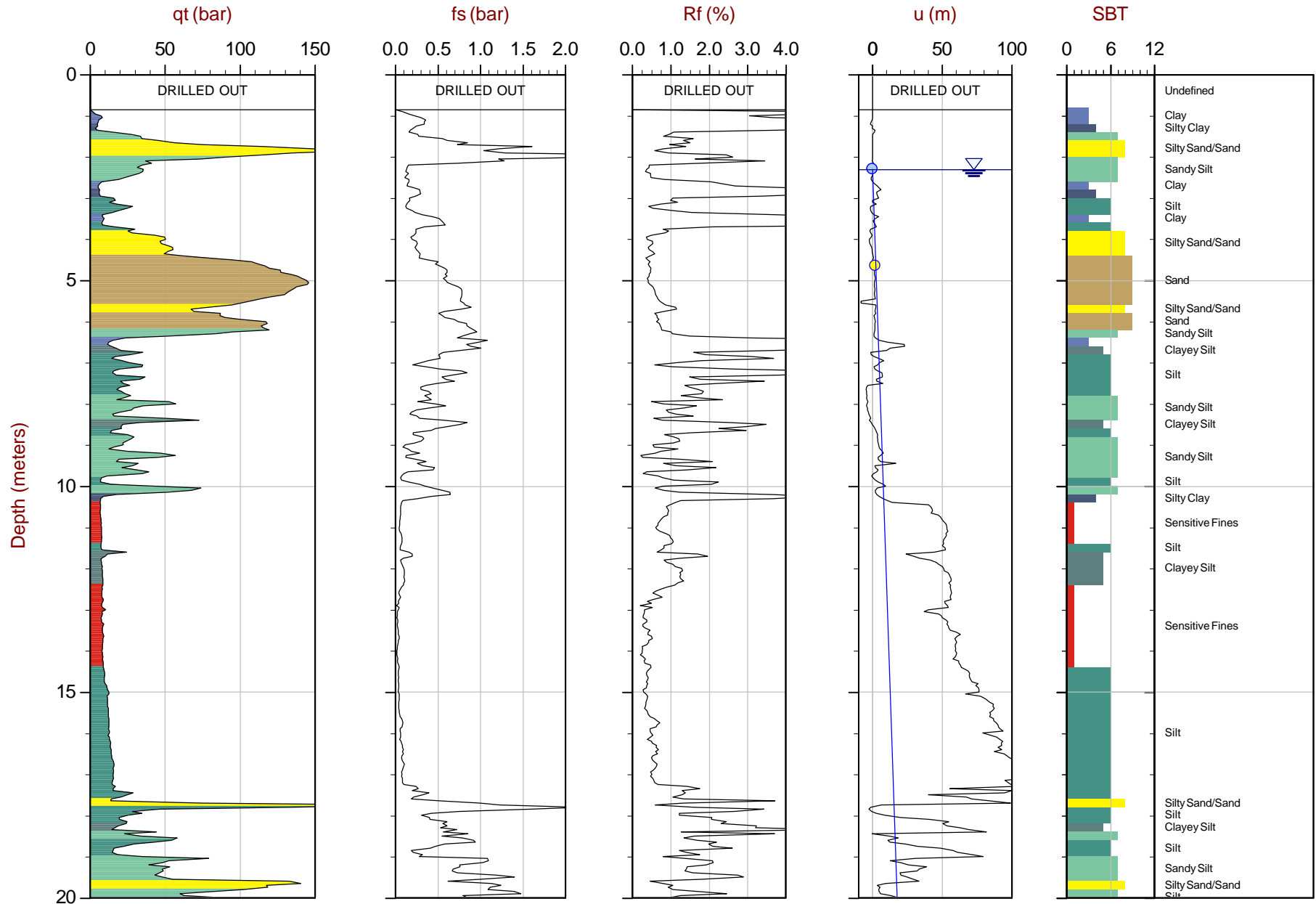
Job No: 13-02006

Date: 05:01:13 14:57

Site: Clarke St., Port Moody, BC

Sounding: CPT13-619+880

Cone: 351:T1500F15U500



Max Depth: 21.400 m / 70.21 ft

Depth Inc: 0.050 m / 0.164 ft

Avg Int: 0.200 m

File: 13-02006_CP619880.COR

Unit Wt: SBT Chart Soil Zones

Overplot Item: ● Ueq ● Assumed Ueq — Hydrostatic Line

SBT: Lunne, Robertson and Powell, 1997

Coords: UTM10N N:5458366m E:510751m

PageNo: 1 of 2

The reported coordinates were acquired from hand-held GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.


GRAHAM

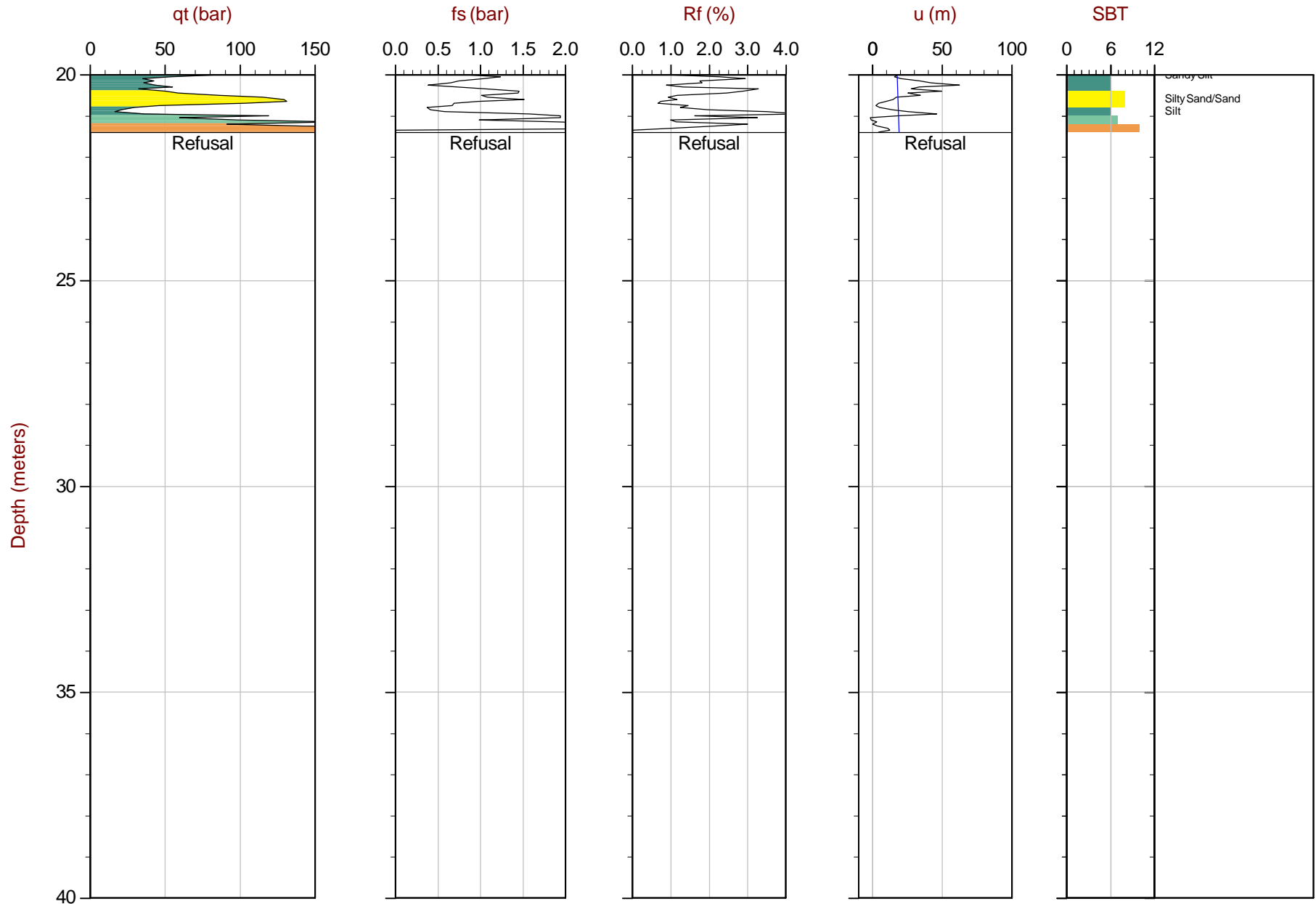
Job No: 13-02006

Date: 05:01:13 14:57

Site: Clarke St., Port Moody, BC

Sounding: CPT13-619+880

Cone: 351:T1500F15U500



Max Depth: 21.400 m / 70.21 ft
 Depth Inc: 0.050 m / 0.164 ft
 Avg Int: 0.200 m

File: 13-02006_CP619880.COR
 Unit Wt: SBT Chart Soil Zones
 Overplot Item: ● Ueq ● Assumed Ueq — Hydrostatic Line

SBT: Lunne, Robertson and Powell, 1997
 Coords: UTM 10N N: 5458366m E: 510751m
 Page No: 2 of 2

The reported coordinates were acquired from hand-held GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.


GRAHAM

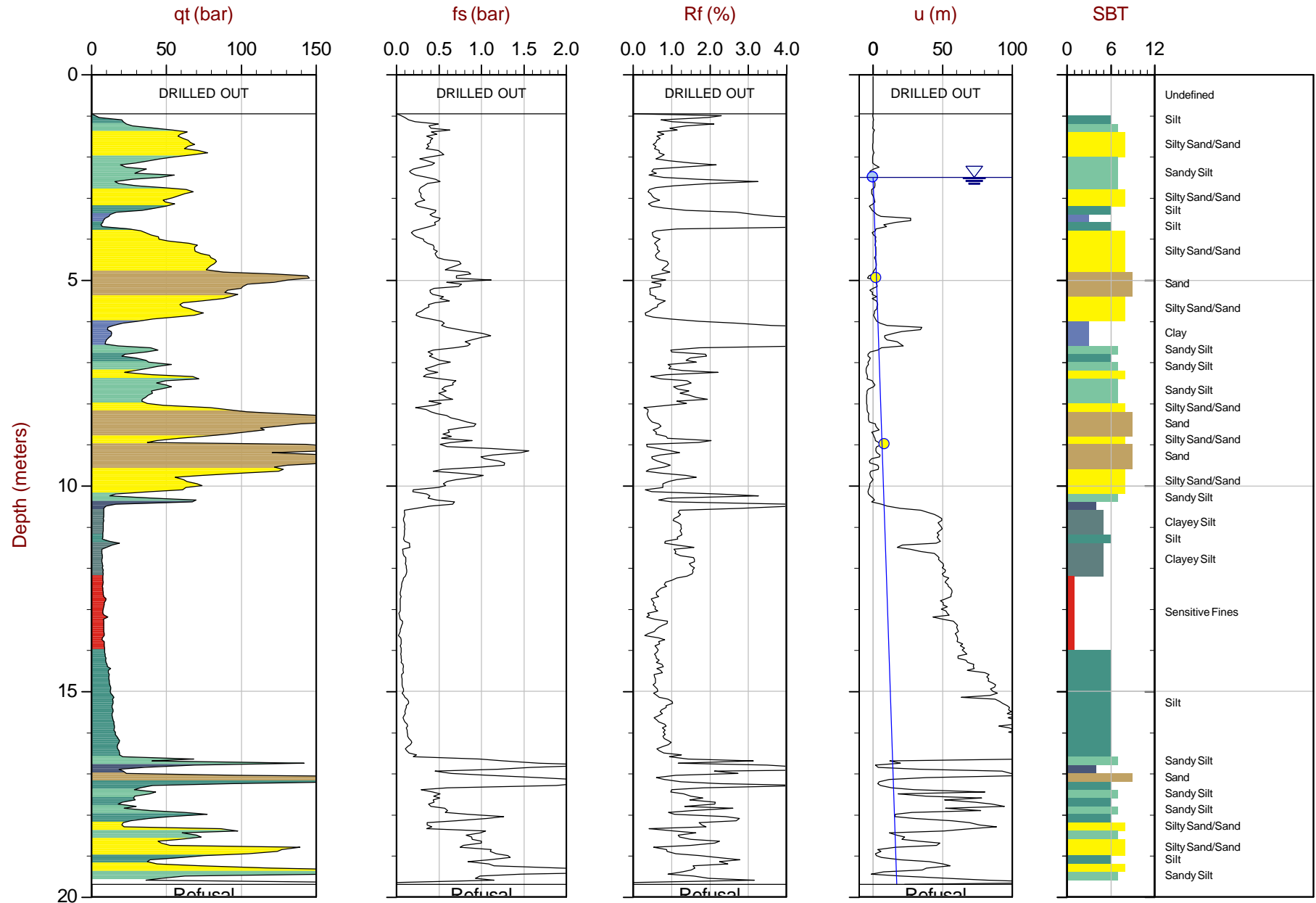
Job No: 13-02006

Date: 05:02:13 08:58

Site: Clarke St., Port Moody, BC

Sounding: CPT13-619+900

Cone: 351:T1500F15U500



Max Depth: 19.700 m / 64.63 ft

Depth Inc: 0.050 m / 0.164 ft

Avg Int: 0.200 m

File: 13-02006_CP619900.COR

Unit Wt: SBT Chart Soil Zones

Overplot Item:



SBT: Lunne, Robertson and Powell, 1997

Coords: UTM 10N N: 5458354m E: 510763m

Page No: 1 of 1

The reported coordinates were acquired from hand-held GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.


GRAHAM

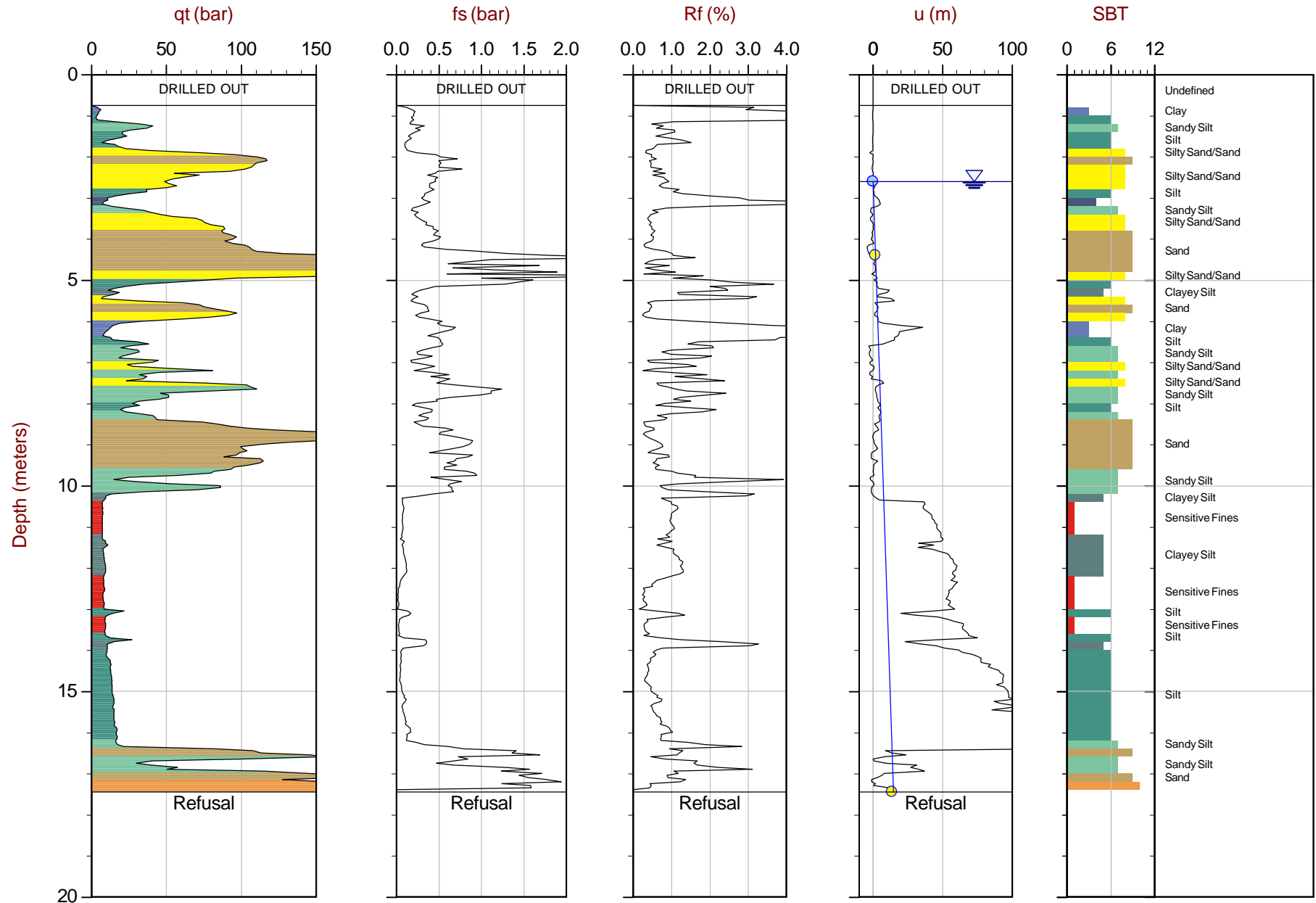
Job No: 13-02006

Date: 05:02:13 10:42

Site: Clarke St., Port Moody, BC

Sounding: CPT13-619+920

Cone: 351:T1500F15U500



Max Depth: 17.450 m / 57.25 ft

Depth Inc: 0.050 m / 0.164 ft

Avg Int: 0.200 m

File: 13-02006_CP619920.COR

Unit Wt: SBT Chart Soil Zones

Overplot Item:



— Hydrostatic Line

SBT: Lunne, Robertson and Powell, 1997

Coords: UTM 10N N: 5458357m E: 510788m

Page No: 1 of 1

The reported coordinates were acquired from hand-held GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.


GRAHAM

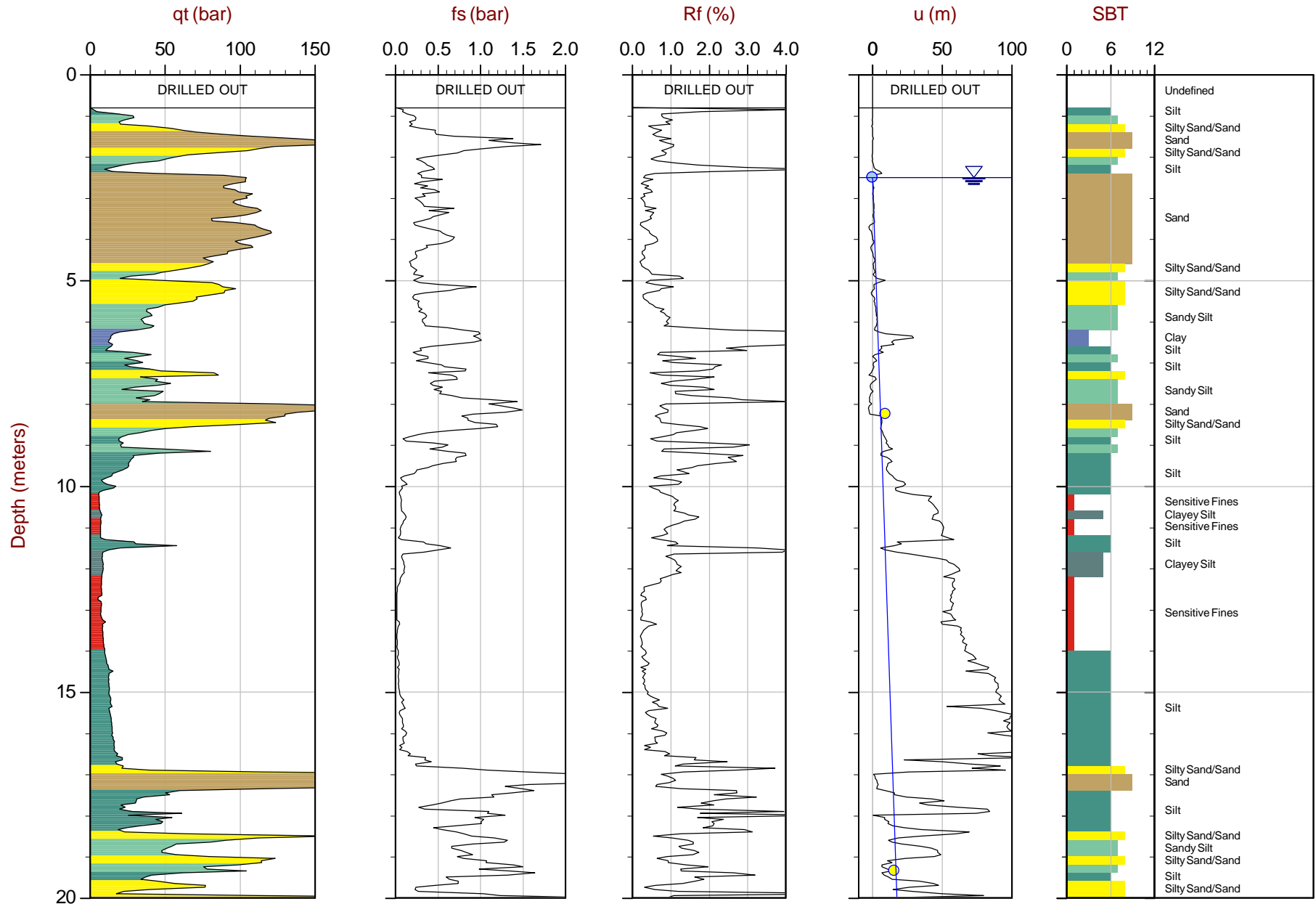
Job No: 13-02006

Date: 05:02:13 16:01

Site: Clarke St., Port Moody, BC

Sounding: CPT13-619+961

Cone: 351:T1500F15U500



Max Depth: 20.900 m / 68.57 ft

Depth Inc: 0.050 m / 0.164 ft

Avg Int: 0.200 m

File: 13-02006_CP619961.COR

Unit Wt: SBT Chart Soil Zones

Overplot Item: ● Ueq ● Assumed Ueq — Hydrostatic Line

SBT: Lunne, Robertson and Powell, 1997

Coords: UTM 10N N: 5458359m E: 510826m

Page No: 1 of 2

The reported coordinates were acquired from hand-held GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.

Site: Clarke St., Port Moody, BC



The reported coordinates were acquired from hand-held GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.


GRAHAM

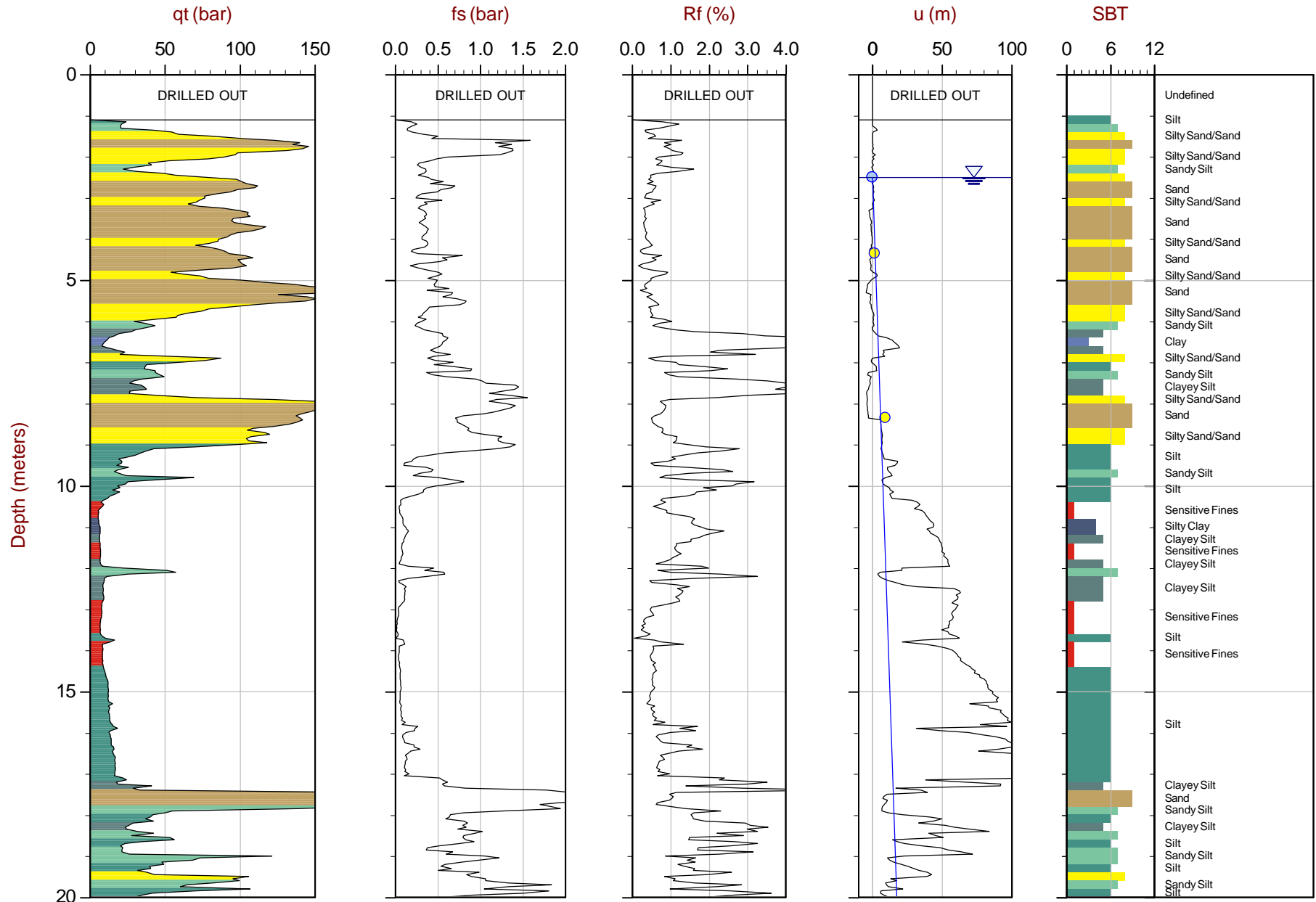
Job No: 13-02006

Date: 05:02:13 14:05

Site: Clarke St., Port Moody, BC

Sounding: CPT13-619+980

Cone: 351:T1500F15U500



Max Depth: 23.350 m / 76.61 ft

Depth Inc: 0.050 m / 0.164 ft

Avg Int: 0.200 m

File: 13-02006_CP619980.COR

Unit Wt: SBT Chart Soil Zones

Overplot Item: ● Ueq ● Assumed Ueq — Hydrostatic Line

SBT: Lunne, Robertson and Powell, 1997

Coords: UTM 10N N: 5458365m E: 510846m

Page No: 1 of 2

The reported coordinates were acquired from hand-held GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.



The reported coordinates were acquired from hand-held GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.


GRAHAM

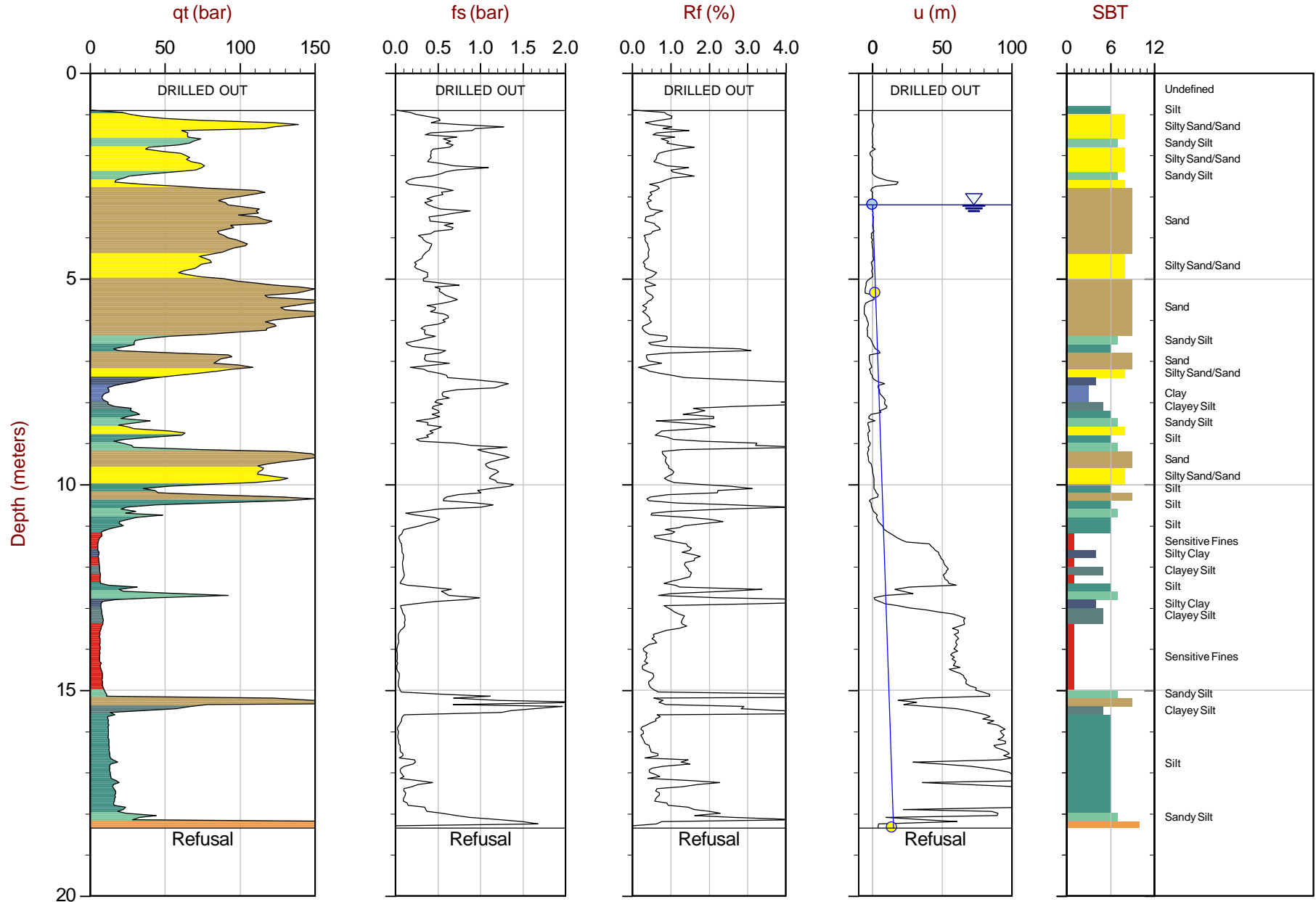
Job No: 13-02006

Date: 05:02:13 12:14

Site: Clarke St., Port Moody, BC

Sounding: CPT13-620+000

Cone: 351:T1500F15U500



Max Depth: 18.350 m / 60.20 ft

Depth Inc: 0.050 m / 0.164 ft

Avg Int: 0.200 m

File: 13-02006_CP620000.COR

Unit Wt: SBT Chart Soil Zones

Overplot Item: ● Ueq ● Assumed Ueq — Hydrostatic Line

SBT: Lunne, Robertson and Powell, 1997

Coords: UTM 10N N: 5458367m E: 510872m

Page No: 1 of 1

The reported coordinates were acquired from hand-held GPS equipment and are only approximate locations. The coordinates should not be used for design purposes.

Appendix III

NBCC 2020 Seismic Hazard Parameters



Government
of CanadaGouvernement
du Canada[Canada.ca](#) > [Natural Resources Canada](#) > [Earthquakes Canada](#)

2020 National Building Code of Canada Seismic Hazard Tool



This application provides seismic values for the design of buildings in Canada under Part 4 of the National Building Code of Canada (NBC) 2020 as prescribed in Article 1.1.3.1. of Division B of the NBC 2020.

Seismic Hazard Values

User requested values

Code edition	NBC 2020
Site designation X_S	X_D
Latitude (°)	49.278
Longitude (°)	-122.852

Please select one of the tabs below.

NBC 2020

Additional Values

Plots

API

Background Information

**The NBC 5% damped spectral acceleration values can be viewed in the NBC tab.
Additional hazard values for your site can be found below.**

The 5%-damped spectral acceleration ($S_a(T)$, where T is the period, in s) and peak ground acceleration (PGA) values are given in units of acceleration due to gravity (g , 9.81 m/s^2). Peak ground velocity (PGV) is given in m/s. Probability is expressed in terms of percent (%) exceedance in 50 years.

By default, all probabilities for the user-specified site designation are shown. Other site designations can be selected from the respective drop-down menu in the table. In low hazard regions, a minimum value of 0.001g for $T \leq 2.0$ s and of 0.0001g for $T > 2.0$ s is assigned. Further information on the calculation of seismic hazard is provided in the *Background Information* tab.

Site Designation	Probability	$S_a(0.05)$	$S_a(0.1)$	$S_a(0.2)$	$S_a(0.3)$	$S_a(0.5)$	$S_a(1.0)$	$S_a(2.0)$	$S_a(5.0)$	$S_a(10.0)$	PGA	PGV
XD ▼	All ▼											
X_D	2	0.606	0.868	1.02	1.12	1.06	0.753	0.46	0.142	0.0541	0.468	0.697
X_D	2.5	0.556	0.801	0.953	1.05	0.982	0.689	0.412	0.121	0.0448	0.435	0.629
X_D	3.5	0.489	0.703	0.851	0.931	0.866	0.599	0.345	0.0942	0.0338	0.387	0.538
X_D	5	0.423	0.61	0.75	0.819	0.753	0.51	0.282	0.0725	0.0254	0.339	0.45
X_D	7	0.365	0.53	0.661	0.717	0.654	0.433	0.23	0.057	0.0197	0.295	0.378
X_D	10	0.311	0.452	0.57	0.618	0.555	0.36	0.183	0.0442	0.015	0.253	0.31
X_D	14	0.263	0.385	0.491	0.529	0.469	0.298	0.145	0.0346	0.0115	0.216	0.254
X_D	20	0.216	0.319	0.412	0.44	0.386	0.238	0.112	0.0262	0.00849	0.178	0.203
X_D	30	0.167	0.247	0.325	0.345	0.298	0.179	0.0811	0.0184	0.00572	0.139	0.153
X_D	40	0.133	0.199	0.265	0.281	0.24	0.141	0.0625	0.0138	0.00412	0.112	0.121

[Download CSV](#)

← Go back to the [seismic hazard calculator form](#)

Date modified: 2021-04-06

Appendix IV

Liquefaction Triggering Assessment

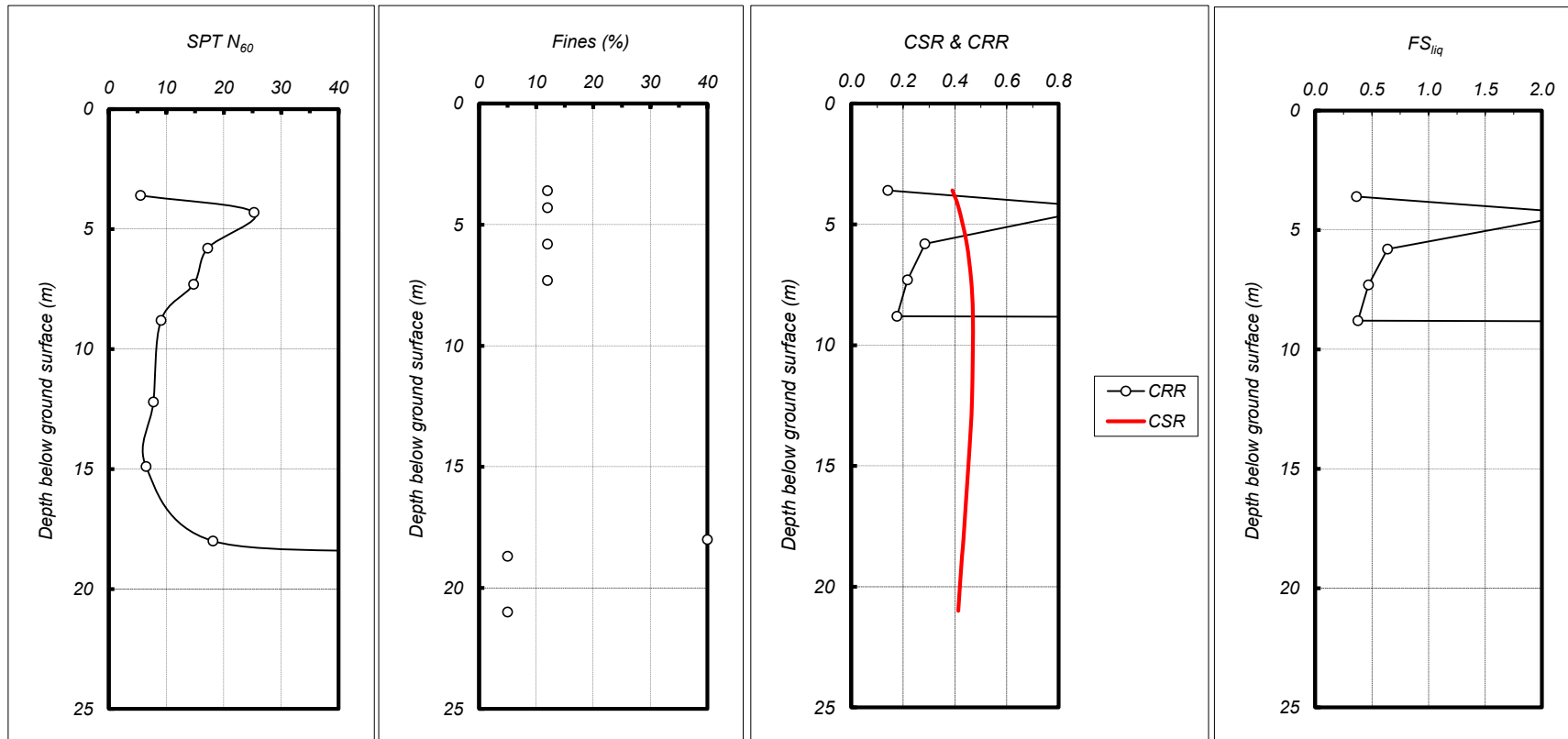


2022-12-15



Project: Moody Centre PPS
 Borehole: BH09-305
 Station: 0

Input parameters:
 Peak ground accel (g) = 0.47
 Earthquake magnitude, M = 7.1
 Water table depth (m) = 2
 Average γ above water table (kN/m³) = 18
 Average γ below water table (kN/m³) = 18
 Hammer Energy Ratio = 74





GeoLogismiki
Geotechnical Engineers
Merarhias 56
<http://www.geologismiki.gr>

SPT BASED LIQUEFACTION ANALYSIS REPORT

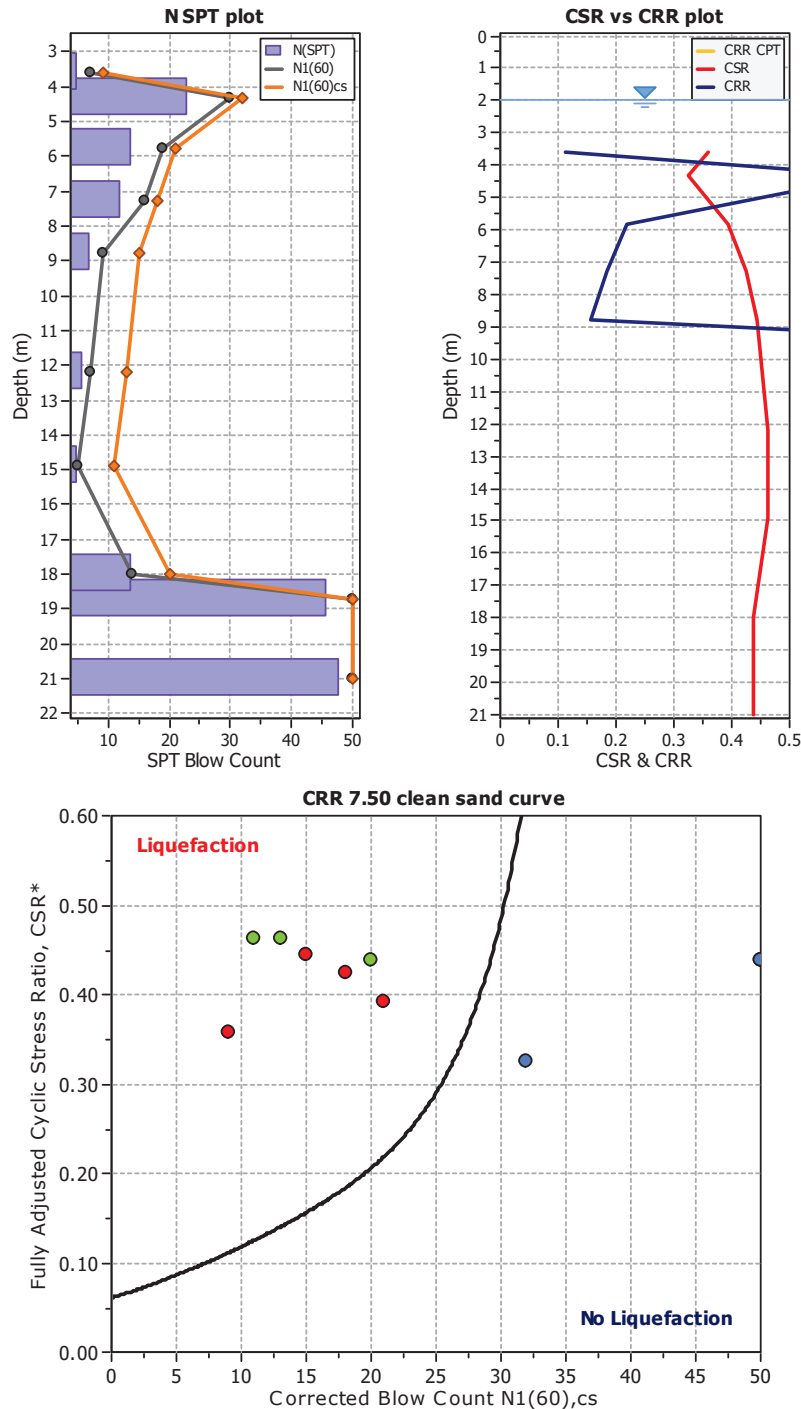
Project title : Moody Centre PPS Building

Location : Port Moody, BC

Borehole Name : BH09-305

:: Input parameters and analysis properties ::

Analysis method:	Idriss & Boulanger 2014	G.W.T. (in-situ):	2.00	EQ site conditions:	Same as initial
Fines correction method:	Idriss & Boulanger 2014	G.W.T. (earthq.):	2.00		
Sampling method:	Standard Sample	Earthquake magnitude M_w :	7.10		
Borehole diameter:	65 mm to 115 mm	Peak ground acceleration:	0.47		
Rod length:	1.50	SPT results rounding mode:	Nearest		
Hammer energy ratio:	1.23				



:: Cyclic Stress Ratio fully adjusted (CSR*) numeric results ::

No	Depth (m)	Weight (kN/m ³)	u ₀ (kPa)	σ _v (kPa)	Ext. Load (kPa)	σ' _v (kPa)	r _d	CSR	K _σ	MSF _{max}	MSF	CSR*
1	3.60	18.00	15.70	64.80	0.00	49.10	0.97	0.390	1.06	1.17	1.02	0.358
2	4.30	18.00	22.56	77.40	0.00	54.84	0.96	0.414	1.10	2.12	1.16	0.325
3	5.80	18.00	37.28	104.40	0.00	67.12	0.94	0.446	1.06	1.53	1.07	0.392
4	7.30	18.00	51.99	131.40	0.00	79.41	0.91	0.462	1.03	1.42	1.06	0.424
5	8.80	18.00	66.71	158.40	0.00	91.69	0.89	0.470	1.01	1.32	1.04	0.445
6	12.20	17.00	100.06	216.20	0.00	116.14	0.83	0.472	0.99	1.26	1.04	2.000
7	14.90	17.00	126.55	262.10	0.00	135.55	0.78	0.463	0.97	1.21	1.03	2.000
8	18.00	18.50	156.96	319.45	0.00	162.49	0.73	0.439	0.94	1.49	1.07	2.000
9	18.70	18.50	163.83	332.40	0.00	168.57	0.72	0.433	0.85	2.20	1.17	0.439
10	21.00	18.50	186.39	374.95	0.00	188.56	0.68	0.416	0.81	2.20	1.17	0.438

Abbreviations

Depth: Depth from free surface where SPT was performed (m) during eq.
u₀: Water pressure at test point (kPa) during eq.
σ_v: Total overburden pressure at test point (kPa) during eq.
σ'_v: Effective overburden pressure based on GWT during earthquake (kPa) during eq.
r_d: Nonlinear shear mass factor
CSR: Cyclic Stress Ratio
MSF: Effective overburden stress factor
K_σ: Magnitude Scaling Factor
CSR*: CSR fully adjusted

:: Cyclic Resistance Ratio (CRR) numeric results ::

No	Depth (m)	Fines %	u ₀ (kPa)	σ _v (kPa)	σ' _v (kPa)	N _{SPT}	C _N	C _R	C _B	C _S	C _E	N ₁₍₆₀₎	Δ(N ₁) ₆₀	N _{1(60),cs}	CRR _{7.5}	F.S.
1	3.60	12.00	15.70	64.80	49.10	5	1.49	0.85	1.00	1.00	1.23	7	2.07	9	0.111	0.31
2	4.30	12.00	22.56	77.40	54.84	23	1.24	0.85	1.00	1.00	1.23	30	2.07	32	0.644	1.98
3	5.80	12.00	37.28	104.40	67.12	14	1.19	0.95	1.00	1.00	1.23	19	2.07	21	0.219	0.56
4	7.30	12.00	51.99	131.40	79.41	12	1.12	0.95	1.00	1.00	1.23	16	2.07	18	0.184	0.43
5	8.80	55.00	66.71	158.40	91.69	7	1.05	1.00	1.00	1.00	1.23	9	5.61	15	0.156	0.35
6	12.20	80.00	100.06	216.20	116.14	6	0.93	1.00	1.00	1.00	1.23	7	5.54	13	4.000	2.00
7	14.90	80.00	126.55	262.10	135.55	5	0.86	1.00	1.00	1.00	1.23	5	5.54	11	4.000	2.00
8	18.00	40.00	156.96	319.45	162.49	14	0.81	1.00	1.00	1.00	1.23	14	5.58	20	4.000	2.00
9	18.70	5.00	163.83	332.40	168.57	46	0.87	1.00	1.00	1.00	1.23	50	0.00	50	607.527	2.00
10	21.00	5.00	186.39	374.95	188.56	48	0.85	1.00	1.00	1.00	1.23	50	0.00	50	607.527	2.00

Abbreviations

Depth: Depth from free surface where SPT was performed (m)
Weight: Soil unit weight from previous test point to current (kN/m³)
u₀: Water pressure at test point (kPa)
σ_v: Total overburden pressure at test point (kPa)
σ'_v: Effective overburden pressure based on in situ GWT (kPa)
N_{SPT}: Number of blows count in the field (blows/30 cm)
C_N: Overburden pressure factor
C_E: Energy ratio factor
C_B: Borehole diameter factor
C_R: Rod length factor
C_S: Sampling method factor
N₁₍₆₀₎: Number of blows corrected for 60% energy
ΔN_{1(60),cs}: Fines correction
N_{1(60),cs}: Number of blows corrected for 60% energy and fines
CRR_{7.5}: Cyclic Resistance Ratio for M_w 7.50
F.S.: Factor of safety against liquefaction



SNC • LAVALIN

SNC-Lavalin Inc.
Suite 1100 - 745 Thurlow Street
Vancouver, British Columbia, Canada V6E 0C5
t. 604.515.5151
www.snclavalin.com