CITY OF PORT MOODY

Development Permit No. DP000064

BC TRANSPORTATON FINANCING AUTHORITY 940 BLANSHARD STREET VICTORIA, BC V8W 3E6

- 1. This Development Permit is issued subject to compliance with all applicable City Bylaws, except as specifically varied or supplemented by this Permit.
- 2. This Permit applies to those lands in Port Moody, British Columbia more particularly described below and including all buildings, structures and other development thereon:

CLARKE STREET ROAD ALLOWANCE IN BETWEEN GRANT STREET AND MOODY STREET ADJACENT TO THE SKYTRAIN RAIL LINE AS SHOWN IN THE KEY PLAN IN THE DRAWING PACKAGE INCLUDED IN SCHEDULE "A"

(the "Lands")

- 3. The following plans and documents are made part of this Permit and, notwithstanding any other provision, no works shall be performed upon the Lands covered by this Permit, nor shall any building or structure be erected, constructed, repaired, renovated or sited, that is not in substantial accordance with the following and strictly in accordance with all terms and conditions of this Permit:
- 4. The following requirement is hereby imposed under section 490(1)(c) of the Local Government Act:
 - a) Substantial construction shall commence within two (2) years of the date of the Council resolution authorizing issuance of this Development Permit or the Permit will lapse.
- 5. The following requirements are hereby imposed under sections 489(b), 490(2) and 491(2), (4), (7), and (8) of the Local Government Act:
 - a) The site shall be developed in accordance with the attached plans and documents:
 - EMUP Propulsion Power Upgrades 90% Submission, prepared by Francl Architecture Inc., dated July 19, 2023, pages 1 through 20. and attached as Schedule 'A'

- 6. As conditions of this Development Permit, the following shall be provided to the City of Port Moody for review:
 - a) Final IFC Architectural Drawings
 - b) Final Tree Management Plan
 - c) Final Landscape Plan
 - d) Stormwater Management Plan
 - e) Construction Management Plan
 - f) Erosion and Sediment Control Plan
 - g) Fire Department Access Plan
- 7. As a condition of this permit the owner shall notify the City Arborist upon installation of tree protection fencing, prior to commencement of any construction activities.
- 8. As a condition of this permit the owner will be required to provide full-depth road reconstruction for areas within the road right-of-way disturbed for underground service or mainline addition or abandonment.

Development Permit: Hazardous Lands – Soil Liquefaction

- (a) 60% Geotechnical Design Report Titled "EMUP Propulsion Power Upgrades Project, Moody Centre Station – 60% Geotechnical Design Report, South Coast British Columbia Transportation Authority SNC-Lavalin Project: 675448, on file with the City, prepared SNC Lavalin and dated December 19, 2022. and attached as Schedule B to this permit, and any amendments thereto subsequently reviewed by the City.
- 4. Prior to the commencement of construction the owner will provide the City with an final Geotechnical Design Report which includes a statement confirming that the site is safe for the intended use.

CITY OF PORT MOODY, by its authorized signatories:

Michael Olubiyi Manager of Development Planning (for)

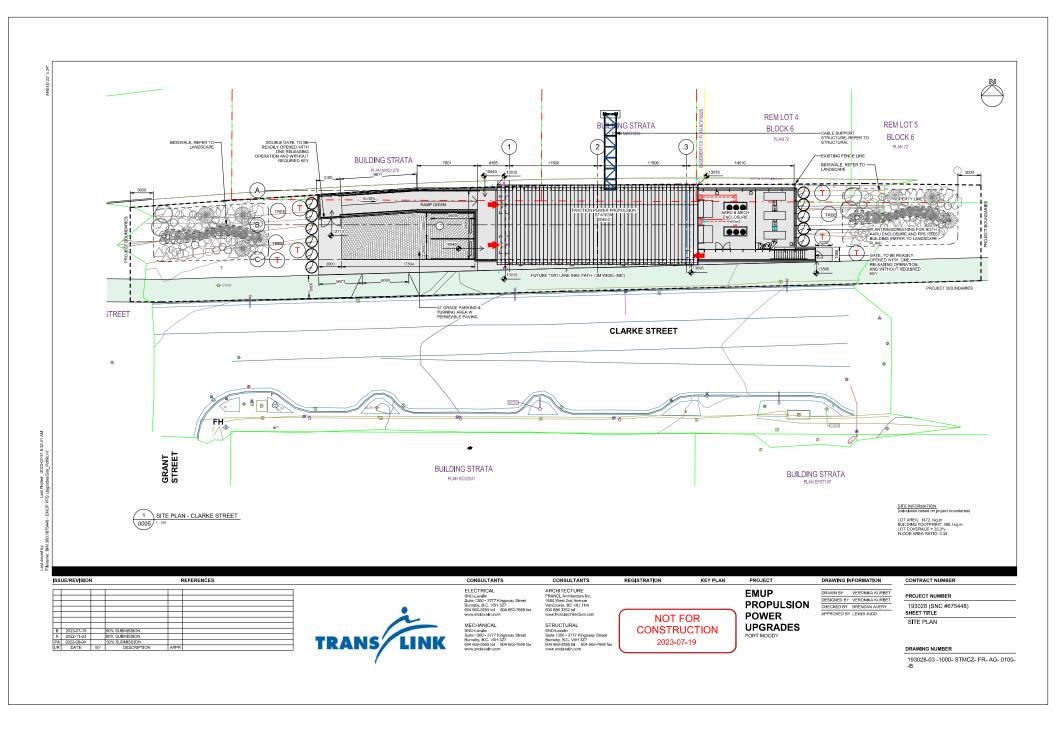
Kate Zanon General Manager of Community Development

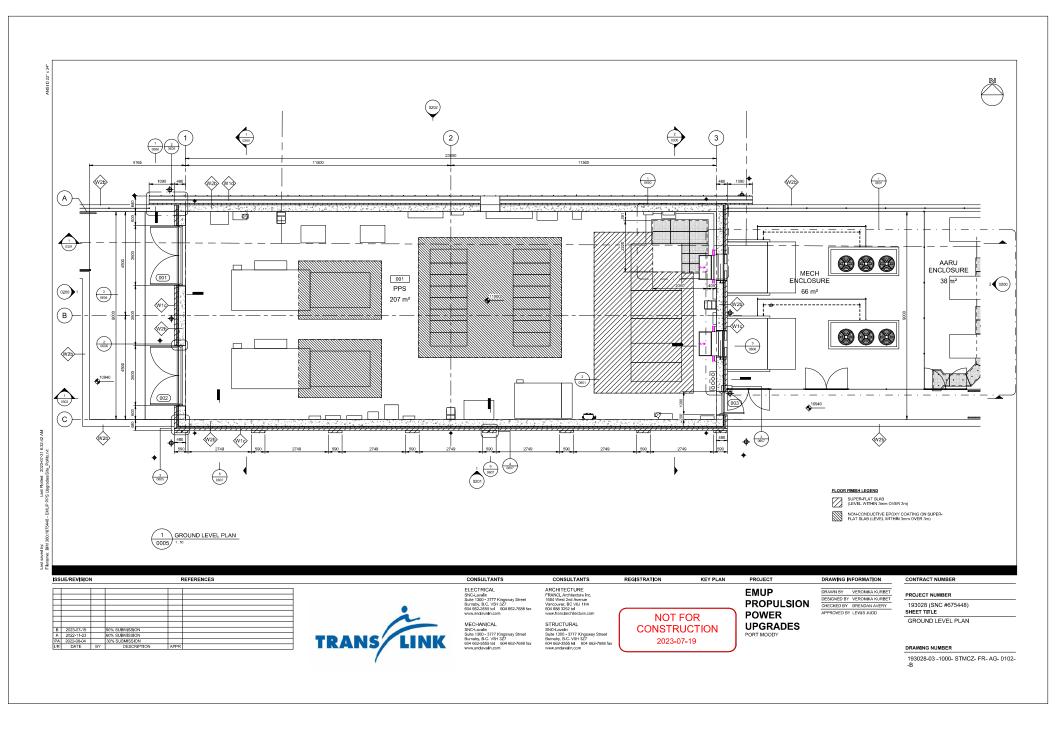
Pursuant to City of Port Moody Development Permit Delegation Bylaw, 2015, No. 3032

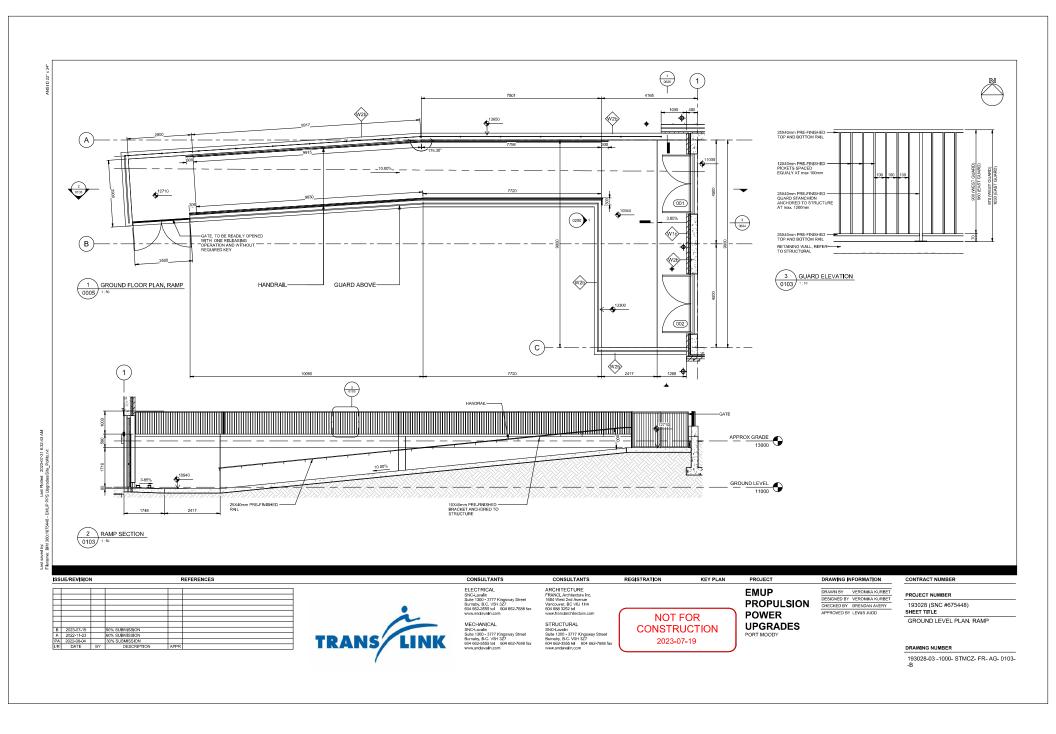
Dated on the _____ of _____, 2024.

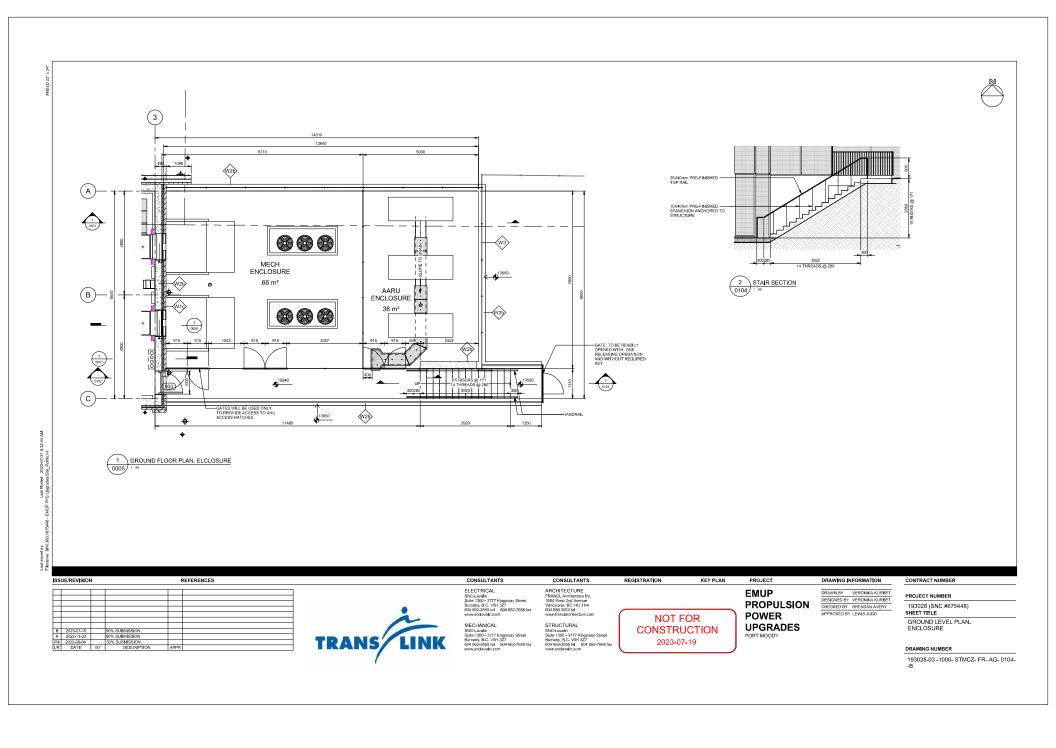
SCHEDULE A

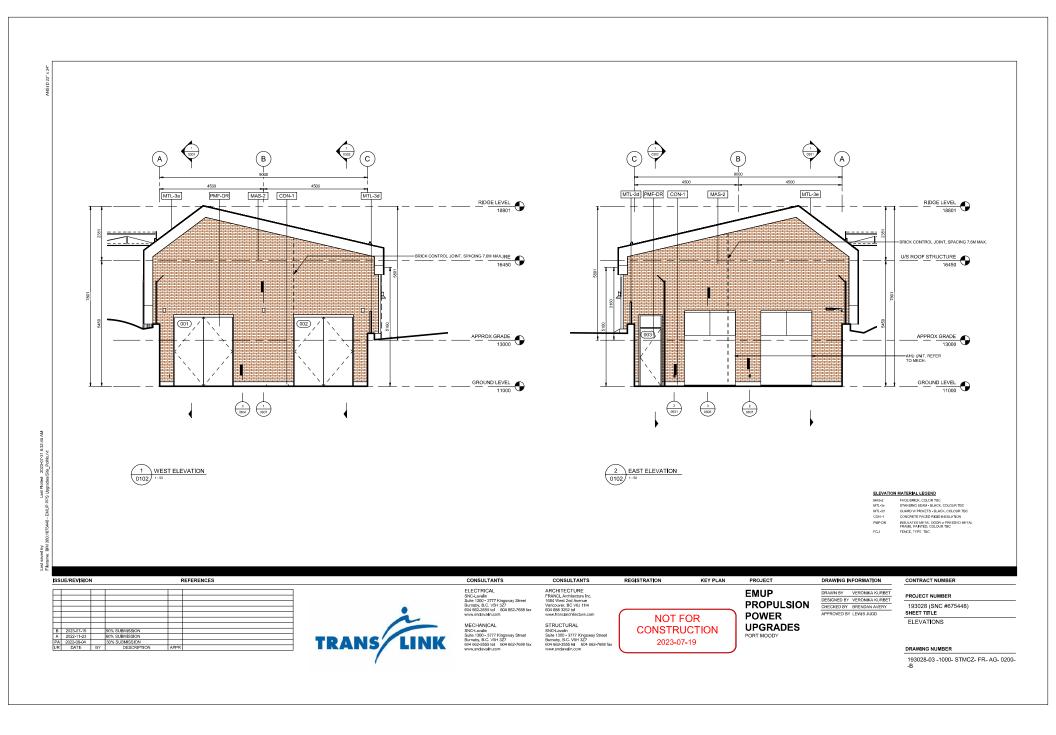
Architectural Drawings

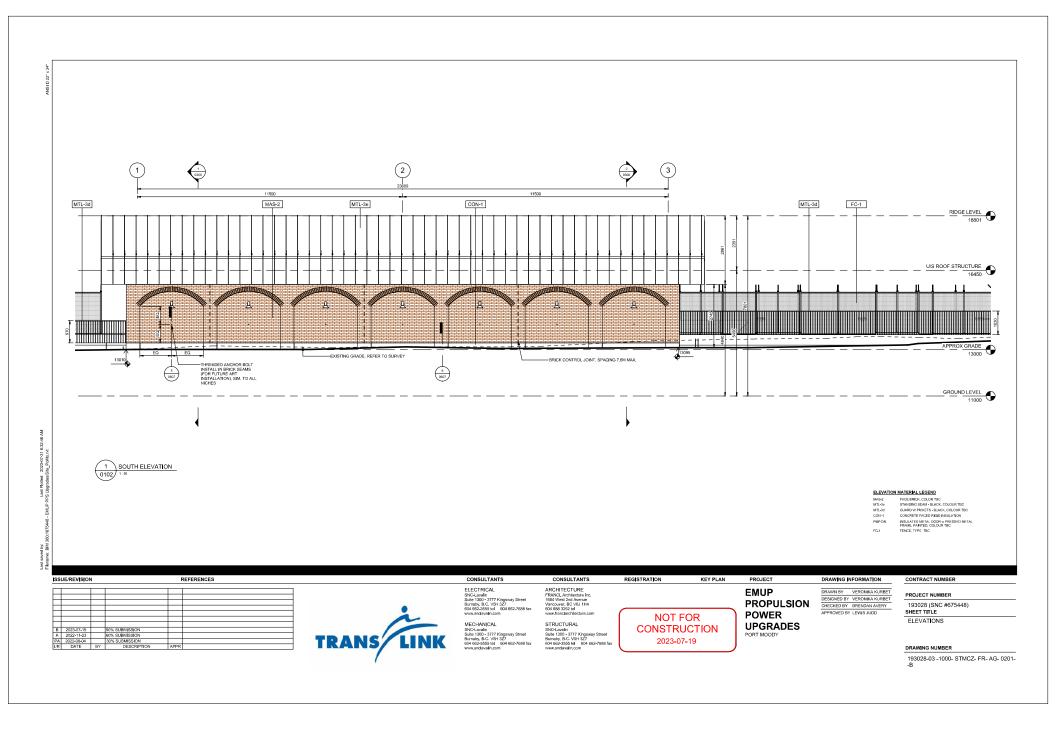


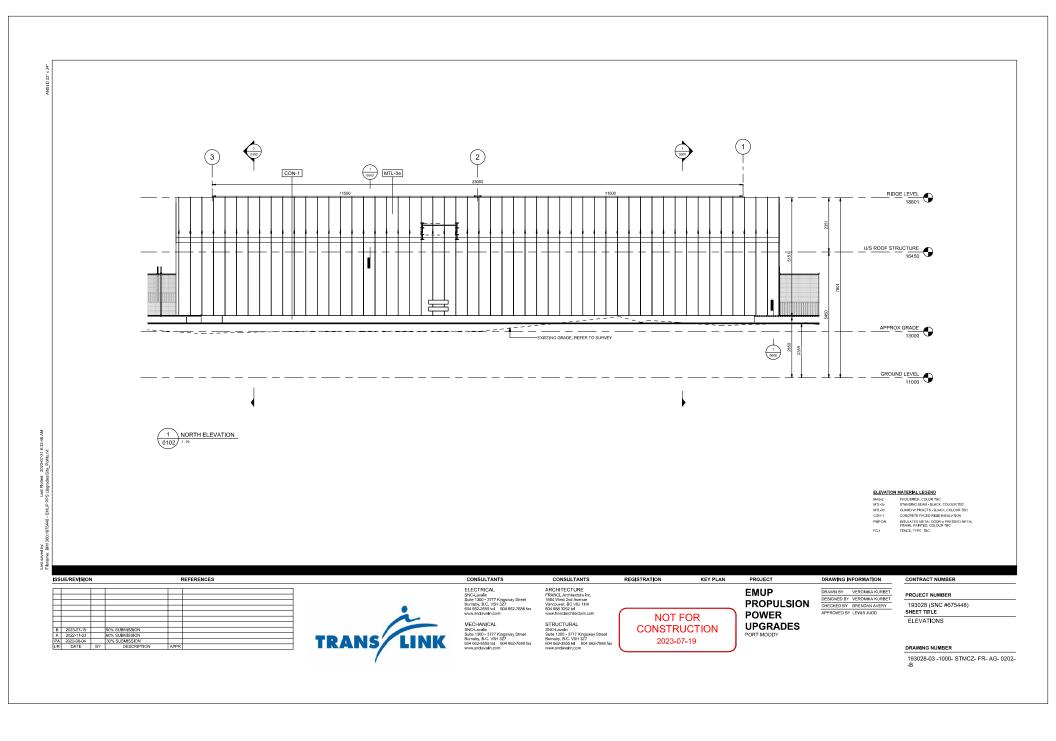


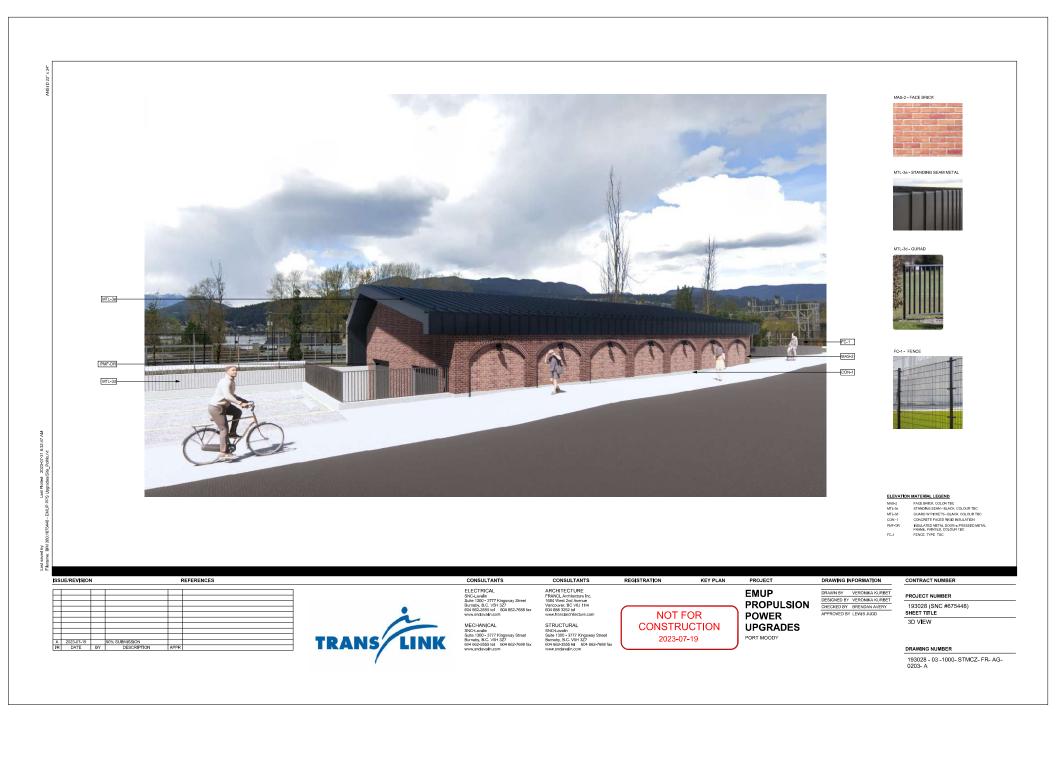
















1. EAST VIEW



2. SOUTH-EAST VIEW



3. WEST VIEW



4. NORTH-EAST VIEW

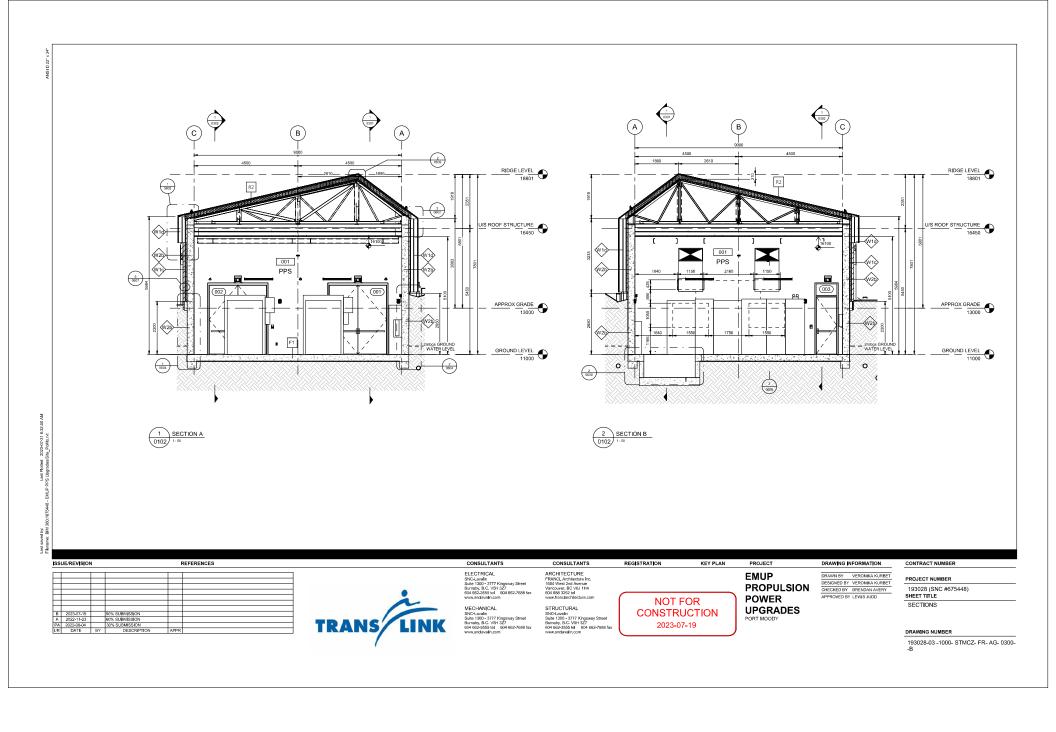


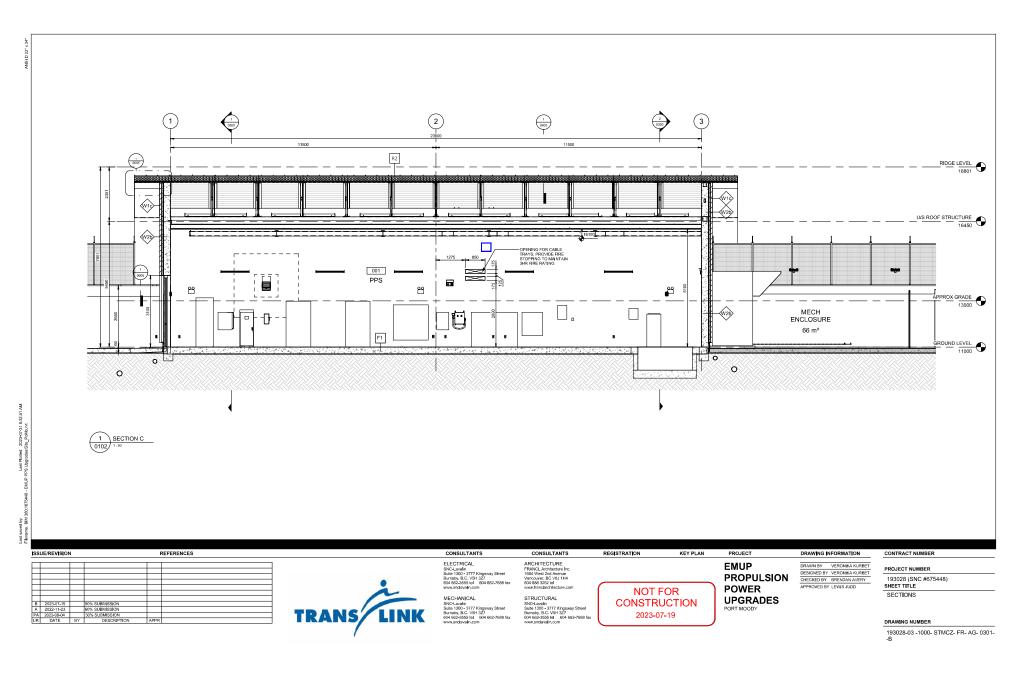
5. NORTH-WEST VIEW

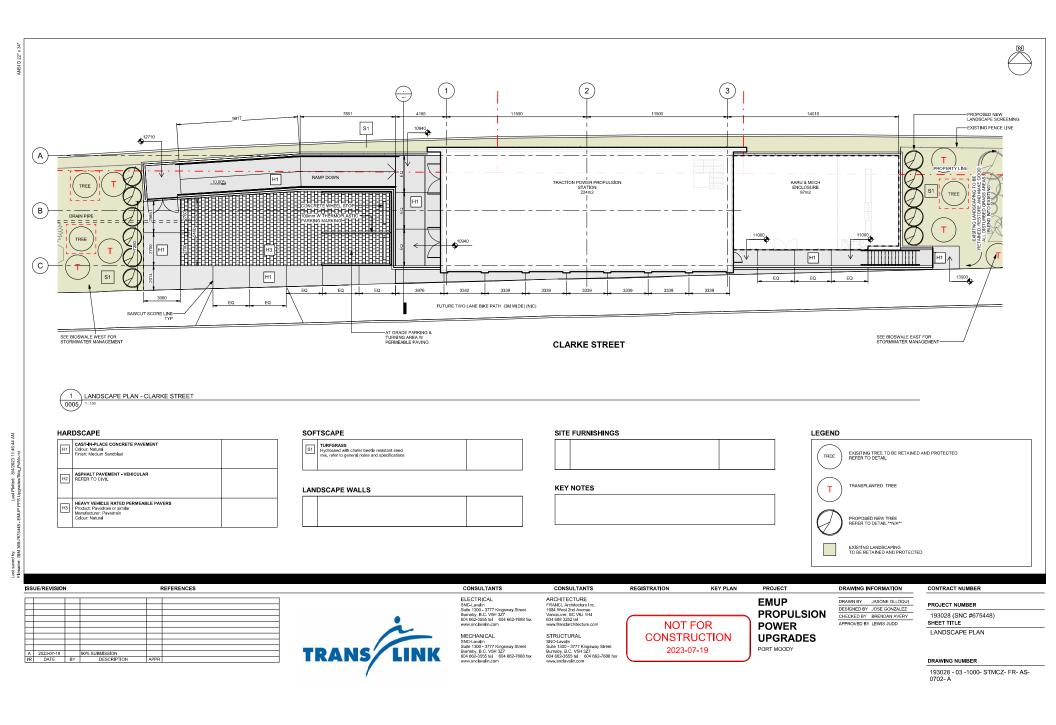


CONTEXT PLAN

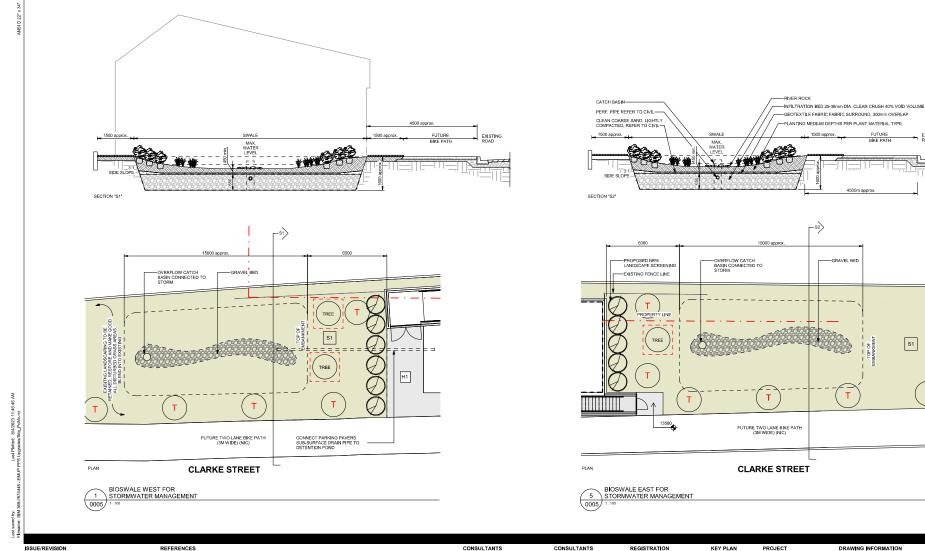


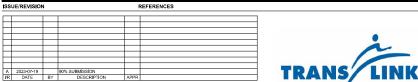






ROAD





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MECHANICAL



STRUCTURAL SNC-Lavajin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 3Z7 604 662-7555 tel 604 662-7688 fax www.snclavalin.com

NOT FOR CONSTRUCTION 2023-07-19

PROJECT	DRAWING INFORMATION
EMUP	DRAWN BY JASONE OLLOQU
PROPULSION	DESIGNED BY JOSE GONZALEZ CHECKED BY BRENDAN AVERY
POWER	APPROVED BY LEWIS JUDD
UPGRADES	

PORT MOODY

PROJECT NUMBER 193028 (SNC #675448) SHEET TITLE

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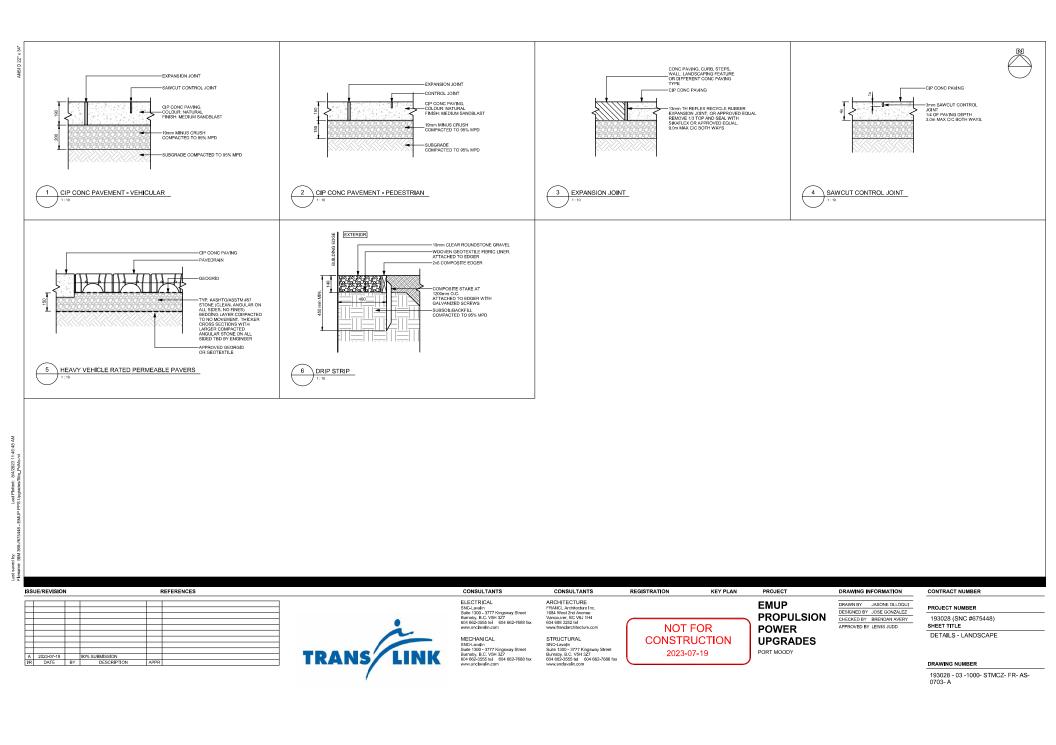
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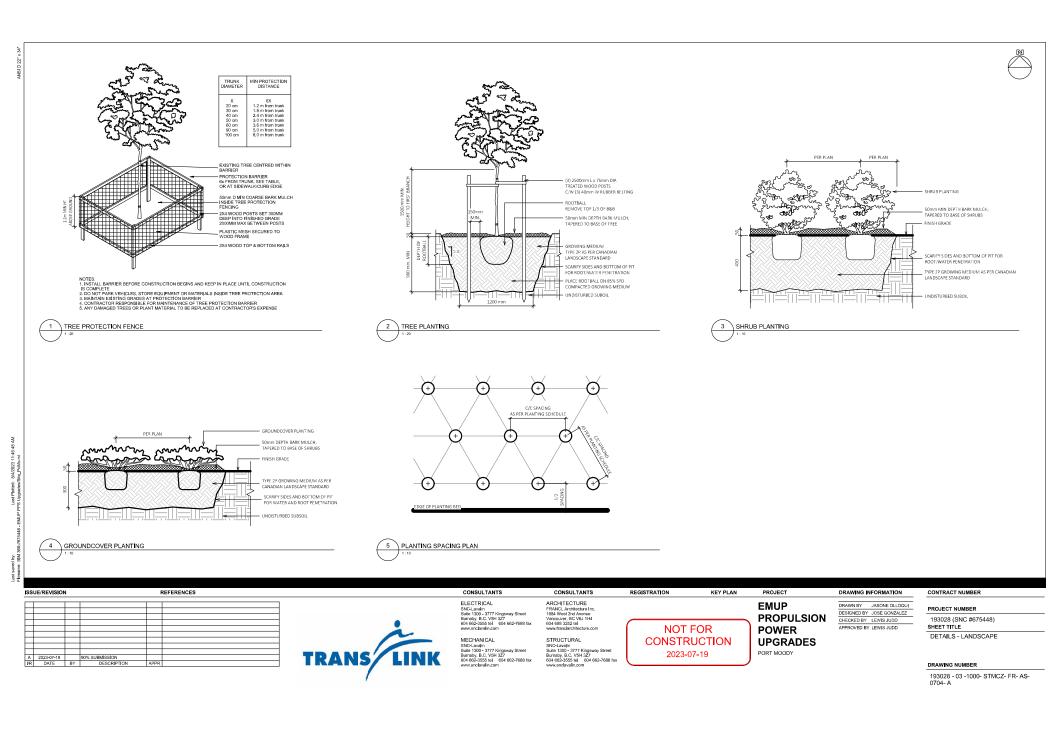
SWALES

DRAWING NUMBER

CONTRACT NUMBER

193028 - 03 -1000- STMCZ- FR- AS-0702b- A





SCHEDULE B

Geotechnical Design Report



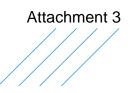
EMUP Propulsion Power Upgrades Project

Moody Centre Station – 60% Geotechnical Design Report

South Coast British Columbia Transportation Authority

December 19, 2022 SNC-Lavalin Project: 675448 Internal Reference: 193028-03-2000-STMCZ-SL-BJ-0001> Draft Rev. A





Signature Page

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Reviewed By:

Mohsen Nicksiar, PhD, P.Eng. Geotechnical Operations Manager – Lower Mainland

Geoscience & Materials Practice Engineering Services Canada



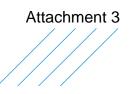


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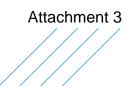


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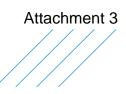
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- I. Drawings
- II. Borehole/CPT Logs
- III. NBCC 2020 Seismic Hazard Parameters
- IV. Liquefaction Triggering Assessment

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1 Introduction

SNC-Lavalin Inc. (SNC-Lavalin) has been retained by South Coast British Columbia Transportation Authority (SCBCTA, or TransLink) to provide geotechnical design services for the SkyTrain Expo and Millennium Line Propulsion Power Upgrades (EMUP) project. This project consists of upgrades to multiple SkyTrain stations located in Vancouver, Burnaby, Surrey, Coquitlam and Port Moody, British Columbia (BC). The locations and scope of the upgrades are generally variable between the SkyTrain stations that are being improved however, in some cases, a part of the upgrade includes a new Propulsion Power System (PPS) building.

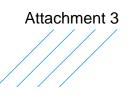
The scope of this 60% design report is to review the geotechnical components of the Moody Centre SkyTrain Station PPS and provide geotechnical recommendations for the design and construction of the PPS elements.

The following tasks were performed as a part of this 60% design report:

- Compile and review existing information pertinent to this project site, including:
 - Available geotechnical reports provided by TransLink and SNC-Lavalin's internal documents for nearby projects, including:
 - EBA Engineering Consultants Ltd., Evergreen Line Rapid Transit Project Combined Moody Centre Station and At-Grade Guideway (Section 340) provided for SNLC-Lavalin Construction (Western) Inc. (SLCW), August 2013.
 - EBA Engineering Consultants Ltd., Evergreen Line Rapid Transit Project At-Grade Guideway Structures (Section 330B) provided for SNLC-Lavalin Construction (Western) Inc. (SLCW), January 2014.
 - Aerial photos, topographic maps, ortho-imagery, utilities mapping and other relevant drawings and datasets; and
 - Published geological maps and technical papers for the general project area.
- Provide design seismic hazard parameters based on the newly published NBCC 2020.
- Provide foundation design parameters and recommendations for select foundation types and construction methodologies.
- Provide general construction recommendations including for site preparation, frost penetration, subgrade preparation, backfilling, site drainage, excavations, dewatering.
- Provide recommendation for the new utility design construction.

Note, the environmental aspect of the project is beyond the scope of this report.





2 Site and Project Description

The Moody Centre SkyTrain Station is situated in the north of Barnet Highway and south of Murray Street, between Williams and Hugh Streets in Port Moody, B.C. The station and surrounding area are shown in Figure 2.1.



Figure 2.1: Moody Centre SkyTrain Station (image obtained from Google Earth)

TransLink has proposed that the PPS building at Moody Centre Station to be located north of Clarke Street, between Grant and Moody Streets (See Appendix I, Drawing No. 193028-03 -1000-STMCZ- FR- AG- 0001- A).

The proposed upgrades will include construction of slab foundations to support the PPS building. The development of the PPS site will include the construction of a new paved road, new buried utilities, and parking spaces.

Figure 2.2 shows the proposed layout of the PPS building relative to nearby landmarks.





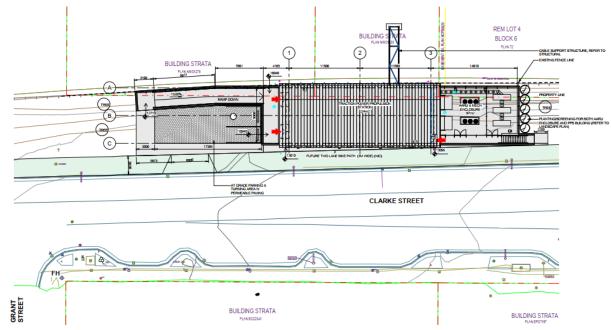
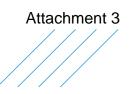


Figure 2.2: Proposed PPS Building Location. (Reference Drawing 193028-03 -1000- STMCZ- FR-AG- 0100-A)

Preliminary drawings showing some design elements in plan view can be found in the Drawings section at the end of this report. A map showing the locations of historical boreholes can be found in Appendix II.





3 Historical Document Review

3.1 General

As a part of the geotechnical design process, SNC-Lavalin reviewed the available geotechnical reports and as-build drawings provided by TransLink. The following documents were provided to us:

- Available geotechnical reports provided by SNC-Lavalin's internal documents for nearby projects.
- EBA Engineering Consultants Ltd., Evergreen Line Rapid Transit Project Combined Moody Centre Station and At-Grade Guideway (Section 340) provided for SNLC-Lavalin Construction (Western) Inc. (SLCW), August 2013.
- EBA Engineering Consultants Ltd., Evergreen Line Rapid Transit Project At-Grade Guideway Structures (Section 330B) provided for SNLC-Lavalin Construction (Western) Inc. (SLCW), January 2014.
- Published geological maps and technical papers for the general project area.

The geotechnical information found in these historical reports has been compiled and summarised in this section. The below subsurface geology should be considered as a summary only; further information can be found in the individual historical reports (Appendix II).

- Fill: loose to compact, grading from sandy silt to silty sand to gravelly sand and sand with trace of silt, scattered cobbles and boulders, and organics including wood debris. The fill is up to 3.5 m thick in the vicinity of Kyle Creek.
- Debris fan, marine and shoreline deposits (Salish Sediments): These deposits were formed by debris floods and flows sourced from the gullies south of the alignment, but with some interlayering of finer-grained marine and shoreline sediments. The deposits vary in thickness from 2 m to 10 m and generally consist of loose to compact interbeds of sand, silty sand, silt and gravel. Occasional cobbles and organic layers including peat and wood debris are also known to occur in these deposits.
- Marine deposits (Capilano Sediments): These deposits vary in thickness from 3 m to 13 m and generally consist of soft to stiff silt and clay with zones of sandy silt and thin lenses of sand and gravel overlying loose to compact interlayered silt and sand. Shell fragments, wood debris, wood fibers and organic layers are also present.
- Glaciomarine Deposits (Capilano Sediments): These deposits vary in thickness from 5 m to 14 m and generally consist of a compact to very dense assemblage of sand, gravel, silt and some clay with occasional cobbles and boulders. In some areas these deposits have a similar texture to till-like soils but have generally not been overridden by glacial ice, and therefore, tend to less dense than the underlying Vashon Sediments.
- Till-like deposits (Vashon Sediments): These deposits generally consist of massive, very dense silty sand to sandy silt with variable gravel and occasional cobbles and boulders. Based on local experience and the results of previous site investigations these deposits may also contain lenses/seams of silt or water-bearing sand and gravel. The depth to these deposits ranges from about 15 m to 30 m below the existing ground surface.

Around the footprint of the proposed PPS building (100m from each side), there are four boreholes and seven CPT soundings from EBA 2013 and 2014 reports (See Appendix II). Table 3-1 summarized the depth of selected boreholes, CPTs, lab and In-situ tests performed in each borehole.



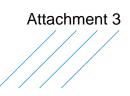


ВН/СРТ	Year of drilling/CPT sounding	Approximate Distance from the footprint of PPS Building (m)	Depth (m)	Performed Lab Tests	In-situ Testing
BH09-305	2009	At the footprint	33.30	Moisture Content, Index limits, Organic Contents	SPT
BOH/BPT12-S3-525	2012	100	29.10	Moisture Content, Index limits, Organic Contents	BPT
BOH12-03	2012	60	32.00	-	BPT
MR12-S3-524	2012	100	38.20	Moisture Content, Index limits, Cyclic Direct Simple Shear (CDSS)	SPT, Downhole, Field Vane
CP13-619-800	2013	80	18.65	-	-
CP13-619-880	2013	At the footprint	21.40	-	-
CP13-619-900	2013	At the footprint	19.70	-	-
CP13-619-920	2013	At the footprint	17.45	-	-
CP13-619-961	2013	20	20.90	-	-
CP13-619-980	2013	40	23.35	-	-
CP13-620-000	2013	70	18.35	-	-

Table 3-1: Summary of the Selected Boreholes, Depth, Lab and In-situ Testing

Based on the above boreholes and CPTs, the general subsoil for the studied area is described in the following Section 4.





4 Subsurface Conditions

4.1 General

As there is no site-specific field investigation for the PPS building at Moody Centre Station, the EBA 2013 and 2014 reports were used for geotechnical subsurface conditions. Soil descriptions discussed in this section are based on the classifications and methodologies that were used in both reports. The closest borehole/CPT to the proposed footprint of the PPS building are BH09-305, CPT619880, CPT619900 and CPT619920, that subsurface conditions are inferred mainly from these borehole and CPT soundings. Additionally, the other nearby boreholes such as BH/BPT12-S3-525, BH12-03, and MR12-S3-524 were also considered in the current project. The location of the boreholes and the most relevant borehole logs are provided in Appendix II.

4.2 Surface Fills

The above-referenced reports include the borehole information obtained in 2009 in which the Fill layer was described as silty sand, some gravel with cobbles and organic material. However, since that time, the existing buildings at the location of BH09-305 were demolished to provide room for the Evergreen Line construction, and a green area was built at that location (as per historical imagery of the Google Earth). Hence, the thickness of the surface Fill is expected to be changed since 2009. The approximate thickness of the Fill layer was considered to be 2 m to 2.5 m

4.3 Upper Silty Sand to Sandy Silt

Below the Fill, the material is generally loose to compact silty Sand to Sand, contains cobbles with some silt and gravel with increasing fine content with depth becoming silty Sand to sandy Silt. The thickness of this layer is about 8 m.

4.4 Silty Clay to Clayey Silt

There is a 7 m layer of soft to firm silty Clay to clayey Silt, which contains lenses of sandy Silt and silty Sand. The Atterberg limits were measured on two samples and plasticity index of 18 and 28 were reported.

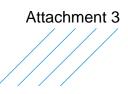
4.5 Lower Silty Sand to Sandy Silt

Below the silty Clay to clayey Silt layer, there is an approximately 2m thickness layer which mainly consists of loose to compact silty Sand to sandy Silt layer, trace gravel.

4.6 Sand and Gravel

This layer generally consists of compact to very dense Sand and Gravel, trace cobbles. The thickness of the layer is approximately 15 m.





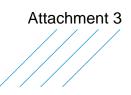
4.7 Till-Like

The very dense sandy Silt, some gravel with trace cobbles encountered at depth of 32 m, inferred as till-like soil. The Till-like material in the studied area is generally silty Sand to sandy Silt with variable gravel, cobbles and boulders content and may contain water-bearing lenses of sand and gravel.

4.8 Groundwater

The groundwater table was located within the subsurface fill layer at approximately 1.5 metre below ground surface (mbgs) to 3.5 mbgs. In BH09-305, the groundwater table was not recorded but a wet sample collected at depth of 2 m. In the current design, water table considered to be at depth of 2 m. It is considered likely that the water table on site is influenced by the nearby creek, and vary seasonally with snow melt and increased precipitation.





5 Seismic Considerations

NBCC 2020 has adopted the use of foundation factors that are dependent on local site soils condition, shaking level, and site period for structural design considerations. The effects of local site conditions are characterized based on the average shear wave velocity or SPT N value of the soils or rock within the upper 30 m of the project location.

Based on the high-level liquefaction screening, the sand and silt underlying the site are susceptible to liquefaction during major earthquake events (1:2475-year return periods). Since these layers are potentially liquefiable, the site is classified as "Site Class F" in accordance with NBCC 2020. However, since the fundamental period of the proposed structure is less than 0.5 second (according to the structural team it is equal to 0.17 seconds) values of "Site Class D" can be used for the design.

SNC-Lavalin has obtained site specific hazard parameters for the investigation area from the National Resources Canada website¹. A copy of the results for the area can be found in Appendix III.

The predicted peak ground horizontal accelerations and spectral accelerations for the different earthquake return period at the project site are provided in Table 5-1 based on NBCC 2020 seismic hazard values.

Table 5-1: Summary of 2020 NBCC Site Class D Seismic Hazard Parameters

Location	Seismic Return Period (yr)	PGA (g)	Sa (0.2)	Sa (0.5)	Sa (1.0)	Sa (2.0)
Moody Centre PPS Building	2475	0.468	1.02	1.06	0.753	0.460

5.1 Liquefaction Analysis

Since there is no site-specific response analysis required, the simplified total-stress based procedure was incorporated using the site-specific Peak Ground Acceleration (PGA) and earthquake magnitude. The liquefaction assessments were completed using available SPT blow counts from BH09-305. The following subsections describe methodology and summarize results of liquefaction analyses.

5.1.1 Methodology

The potential for seismic liquefaction triggering for the non-cohesive soils have been assessed using Boulanger and Idriss (2014) and Idriss and Boulanger (2008) SPT-based methods using the commercially available software CLiq. 3.3.2.9 developed by GeoLogismiki.

In the current assessment, as per Section 4, the upper soil layers are generally silty Sand, sandy Silt to Sand, which are potentially liquefiable. The factor of safety against liquefaction/cyclic softening at each depth was calculated using the following relation:

$$FS = \frac{CRR_{7.5}MSF}{CSR}K_{\alpha}K_{\sigma}$$

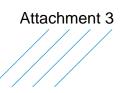
Where MSF=magnitude scaling factor

K_a = ground slope correction factor

 K_s = overburden stress correction factor

¹ Natural Resources Canada (2020). Seismic Hazard Calculator. Retrieved from <u>2020 National Building Code of Canada Seismic</u> <u>Hazard Tool (nrcan.gc.ca)</u>





A factor of safety less than 1.0 Indicates a high liquefaction potential. However, computing a factor of safety against soil liquefaction is not as important as carrying out the analysis with a consistent methodology and taking into consideration the anticipated variability of the in-situ soil resistance and variables that define the earthquake ground motions.

The Cyclic Resistance Ratio (CRR) of layers to cyclic loading was estimated from the SPT measurements, specifically from BH09-305. In addition, the CSR was modified for earthquake magnitude, overburden pressure, and shear stress level in static condition.

It is worth noting that, at the time of this report and as per SNC-Lavalin correspondence with National Resources Canada (dated July 19, 2022), the 6th generation of seismic hazard deaggregation analyses are not yet released, so that for the purpose of the current liquefaction assessment, the 5th generation deaggregation plots were used. As per the 5th generation hazard model provided by Geological Survey of Canada (GSC), the moment magnitude (Mw) for the site is considered 7.1 for crustal and Inslab earthquake suites and 8.4 for interface earthquakes.

5.1.1.1 Cyclic Stress Ratio

For sand, the CRR for liquefaction assessment is calculated using both Boulanger and Idriss (2014) and Idriss and Boulanger (2008) methods by normalizing the SPT blow counts to overburden pressure, and adjustment based on the fines content in order to represent the equivalent clean sand (N1_{60,cs}).

5.1.1.2 Extent of Potential Liquefiable Layers

Liquefaction triggering assessment was completed according to the methodology described in Section 5.1.1. Table 5-2 demonstrates the extent of liquefaction.

Liquefaction triggering assessment results and the profile of FOS are presented schematically in Appendix IV.

Table 5-2: Extent of Liquefaction

	Extent of Liquefaction Elevation (m)		
	ldriss & Boulanger (2008)	Boulanger & Idriss (2014)	
Circulified Dreedure 0475.	+8.8 to +8.1	+8.8 to +8.1	
Simplified Procedure, 2475yr	+7.4 to +2.0	+7.4 to +2.0	

5.1.1.3 Post-Liquefaction Settlement and Lateral Ground Movement

Dissipation of the excess pore water pressure developed during and after strong ground motions would cause settlement of the ground. Semi-empirical method developed and proposed by Idriss and Boulanger (2008) and Wu et al. (2002) were used to estimate the post-liquefaction settlement. The proposed methodology is very similar to the one proposed by Yoshimine et al (2006).

Post-liquefaction free-filed settlement was estimated for the generalized soil profile and presented in Table 5-3. The anticipated differential settlement should be expected to be 50% of the total.

Soil liquefaction will result in lateral ground displacements. The lateral displacements are caused by strain softening of liquefied soil and earthquake-induced cyclic inertia forces. The ground lateral displacements, for a screening level assessment, can be estimated using the simplified empirical method proposed by Youd et al. (2002). In addition, liquefaction may induce large lateral ground spreading displacements, maybe in the order of several metres, however the liquefaction-induced permanent displacements are



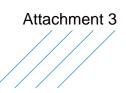


difficult to quantify with confidence. As the SkyTrain At-grade guideway is supported by Deep Soil Mixing (DSM) wall (EBA 2014 report), and the footprint of the PPS building located in south side of the guideway, it is anticipated that there is no post-liquefaction later displacement along the North-South direction. However, there would be lateral displacement in the East-West direction (the ground slope is around 1.5%) in case of liquefaction. The estimated post-liquefaction and lateral displacement are presented in Table 5-3.

Table 5-3: Post-liquefaction Reconsolidation Settlement and Lateral Displacement

	Post Liquefaction Settlement (mm)	Lateral Displacement (m)
2475yr Earthquake	200-300 mm	0.05-0.3m





6 Geotechnical Recommendations

6.1 General Construction Recommendations

6.1.1 Site Preparation

Preparation of the site for construction is anticipated to consist of some or all of the following items:

- Removal and replacement of the existing green area;
- Stripping of the PPS footprint, including the removal of organic materials, such as topsoil, grass, stumps, and garbage from the area adjacent to the existing SkyTrain retaining wall/fence; and
- Since there is no site-specific field investigation, BH09-305 (EBA 2013 and 2014 report) was used. Based on this borehole the fill and topsoil material (to the EL +9.00m) should be removed a backfilled with engineering fill.

6.1.2 Frost Protection

Frost penetration will impact foundations due to the expansion of pore water in the soils. Based on historical temperature data for the Port Moody area, the estimated maximum frost penetration depth is approximately 450 mm. The effect of snow cover, a higher ground water table, vehicle traffic, and higher moisture contents all affect the depth of frost penetration.

Any concrete constructions such as footings should be designed so that they are founded on a freely draining material with their base at least 450 mm below the surrounding ground cover to reduce the risk of frost induced stresses.

6.1.3 Subgrade Preparation

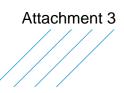
Subgrade preparations should consist of the removal of organics and loose or saturated surficial soils. The exposed native deposits should be inspected for organic materials, saturated spots, and material consistency by a geotechnical engineer upon exposure through excavation. Any soft spots, or saturated zones shall be removed and sub-excavated by 200 mm before being replaced with an approved compacted engineered granular fill. When the excavation floor is deemed acceptable, a proof roll test shall be performed to verify its competence. This proof roll shall be witnessed and verified by SNC-Lavalin's geotechnical engineer.

In the absence of site-specific field investigation, the BH09-305 (EBA 2013 and 2014 report) was used. In this borehole, the depth of fill and topsoil was around 2 m to 2.5 m. Due to variability of the existing fill soils, it is anticipated that this material should be removed from the project footprint and replaced with engineered fill material. Besides, as per Section 5.1, the soil below EL. +8.8 m is deemed to be liquefiable, hence to minimize the differential settlement the subgrade should be reinforced with geogrid.

Furthermore, subgrade preparation prior to structural elements installation should include excavation to design elevation, sub-excavation of boulders, any isolated locations that contain organic fill, protruding construction debris, wood waste or other oversaturated soils, and soft or yielding soils, and replacement of this unsuitable soil with engineered structural fill prepared as per Section 6.1.4.

Subgrade soils should be reviewed by SNC-Lavalin's geotechnical engineer prior to compaction or backfilling.





6.1.4 Engineered Fill

Engineered fill material should be free from any frozen soil, organic materials, and contamination. All placed engineered fill material should be compacted to 100% of Standard Proctor Maximum Dry Density (SPMDD) at moisture contents within ± 2% of optimum. Individual layers of placed loose material (lifts) shall not exceed 300 mm in thickness.

Qualified geotechnical personnel should monitor the quality and placement of all fill soils. The compaction of the fill should be routinely monitored by field density testing. Test results should be submitted to the geotechnical engineer for review.

6.1.5 Building Drainage and Dewatering

Groundwater, where encountered, will likely be found just above the level of the nearby creek and within the fill soils. If dewatering during construction is necessary (either from seepage or during rainfall events), construction dewatering procedures should include routing of water to a suitable outlet. Within excavations, it is recommended that dewatering include directing water to sump areas, from where it may be pumped to a suitable outlet. These sumps and pumps (along with a power source) are to be provided by the contractor and be readily available to prevent water from pooling in the excavations. Both surface water run-off and groundwater should be managed in this way while following local guidelines and bylaws on construction water discharge.

The foundation drainage system for the PPS building at Moody Centre should consist of a minimum 150 mm diameter perforated rigid pipe placed (perforations downwards) along the exterior base of the foundation perimeter and tight-lined to a storm drain system or another suitable outlet. The pipe should be bedded on at least 100 mm of 19 mm coarse clear crushed gravel and be backfilled with minimum 200 mm of a similar drainage gravel wrapped in nonwoven geotextile with a minimum of 150 mm of overlap.

The site should also be graded so that surface water run-off is directed away from the PPS building and towards catch-basins, which are connected to a storm sewer drain or another suitable outlet.

6.1.6 Construction Excavations

Excavations that remain open for less than seven days are considered to be temporary and are likely to be the dominant excavation type encountered during construction. Although not anticipated to be deeper than 1 m, temporary excavation slopes shall be no steeper than 1.5(H):1(V) for temporary shallow (less than 1.8 m) excavations in the native, compact, fill. Water should not be allowed to flow on the side slopes of the excavation or pond at the base of the excavation. Relevant British Columbia (WorkSafeBC) regulations should be adhered to for minimum requirements for temporary excavations. Note, excavations in excess of 1.2 m depth should be reviewed by the geotechnical engineer as required by WorkSafeBC.

6.1.7 Utility Construction and Bedding Requirements

It is understood that new catch-basins and associated sewer lines are to be constructed as a part of the PPS building at Moody Centre.

Newly constructed sewer pipes should be founded on 200 mm of compacted, clean, bedding sand. This same bedding sand should be placed and compacted around the pipe on all sides to 95 % SPMDD using a vibrating plate tamper.





Utility trenches should be excavated to below the frost penetration depth described previously and using the same excavation recommendations presented above. Where sloped excavations are not desired to facilitate utility installation, trench boxes or an alternative, compliant, engineered shoring system can be used to limit the footprint of utility excavations, subject to approval of the geotechnical engineer. Support excavation may be near vertical provided a shoring system is properly installed prior to worker entry.

6.1.8 Slab-on-Grade

Prior to slab-on-grade construction, the subgrade should be prepared and reviewed as described in Section 6.1.3. Slabs-on-grade should be constructed on a minimum 150 mm thick layer of 19 mm clear crushed gravel to act as a capillary break. It is also recommended that a vapor barrier consisting of polyethylene sheeting be placed between the slab and the bedding material to limit the migration of moisture.

Grade re-instatement below the slab should be conducted using engineered fill as described in Section 6.1.4. In-place density testing should be completed on the under-slab fills to confirm that the fill placed below the slab has been compacted to a minimum of 100% of SPMDD. The Geotechnical Engineer should review the subgrade prior to both concrete and fill placement.

A geotextile liner should be placed between the clear crushed gravel fills and silt or clay soils (where encountered) to act as a particle break and improve the long-term performance of the subgrade. This geotextile should consist of a Nilex 4551 or approved equivalent and rolls of this geotextile should overlap at least 200 mm to ensure coverage during the placement of backfill soils.

6.2 Foundation Design Recommendations

6.2.1 Design Recommendations

Based on the anticipated PPS elements and the presence of near surface competent soils (having engineered fill to the elevation of +9.0 m), shallow raft foundations are the recommended foundation type to be utilized at the Moody Centre PPS location. This section provides recommendations for the design and construction of these footings.

Footings placed on the engineered fill, if placed as per Sections 6.1.1, 6.1.3 and 6.1.4 may be designed using the following:

• Raft footings may be designed using Serviceability Limit State (SLS) bearing resistance of 150 kPa. Factored ultimate bearing resistance (ULS) may be taken as 225 kPa.

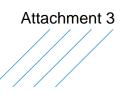
The subgrade modulus for engineering fill can be considered as 10 MN/m3.

Footings/slabs should not be constructed during freezing conditions without appropriate heating and hoarding procedures being followed. As mentioned in Section 6.1.3, the engineered fill below the raft foundation should be reinforced with geogrids (Nilex Biaxial BX1200 or approved equivalent product) to minimize the effect of post-liquefaction differential settlement.

6.2.2 Long-term Consolidation Settlement

As stated in Section 4, there is a 7 m-thick layer of soft to firm silty Clay to clayey Silt, which contains lenses of sandy Silt and silty Sand with the thickness of 25 mm to 50 mm. These fine-grained materials are considered to be potentially compressible, and under sustained loading there may experience long-term consolidation settlement.





The long-term consolidation settlement was estimated using Settle3 (Rocscience, Ver. 5.007). Considering a sustained uniform load of 45 kPa was applied to the raft footing (Correspondence with structural engineer [October 31, 2022]), the settlement was estimated to be in the order of 120 mm to 150 mm. The long-term settlement will be decreased to 50 mm if the minimum distance within 3 m from footprint of the structure.

It is worth noting that in the absence of consolidation tests, the parameters for calculations were estimated using empirical correlations.

6.3 Pavement Recommendations

According to the architectural design team, the preliminary plans for the Moody Centre PPS site call for the addition of a Permeable Architectural Precast Concrete Blocks (PACB) for the access roads and parking areas. Based on the results of the field investigation, SNC-Lavalin has developed a road pavement structure using PACB system that is designed to support the expected traffic loads for the project area. The pavement details are provided in Table 6-1.

Table 6-1: Pavement Structure Recommendations

Pavement Element	Pavement	Bedding Layer/Road Base Fill	Sub-Base Fill
Composition	Architectural Precast Heavy Duty Concrete Blocks	19 mm MINUS with no fines	75 mm MINUS
Thickness (mm)	150 mm	100 mm	450 mm

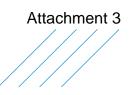
An excavation depth of approximately 700 mm is expected in order to remove the existing mixed fills (though final designed site grading may vary the ultimate excavation depth required). Should excavations encounter contaminated soil, soil that is saturated or overly soft, hog fuel or other debris-heavy fills, some over-excavations may be required to remove these unsuitable materials.

It is recommended that a layer of biaxial geogrid to be placed below the sub-base fill layer along the access road and parking areas to control potential differential settlement due to the presence of compressible subsurface soil. The geogrid should be protected from any potential damages during construction by using either geotextile layers or smaller particle materials such as 19 mm minus material with minimum 100 mm thickness.

Gradation of the bedding layer material should be in accordance with AASHTO #57 and should be free of fine content. As with foundation fills, all fill materials are to be compacted to 100% of SPMDD and be within $\pm 2\%$ of optimum moisture content as determined though Proctor laboratory testing. Loose lift thickness is not to exceed 300 mm in thickness. Some existing fills (if deemed suitable by the on-site geotechnical engineer) may be reused below the designed pavement sections as below pavement fill, however, it is expected that the existing fill will not be suitable for use as "other Grade Fill" and will be removed or otherwise recycled.

Site grading and catch-basin (CB) coverage should be designed to ensure proper drainage of the gravel road surface and to prevent pooling. Service connections and pipe diameters should be designed to provide efficient drainage, and oil/water separators may be added as needed.





7 Construction Review

The preceding section outlines recommendations for the design and construction of the proposed development. Review of certain aspects of construction are required to satisfy the requirements of the BC Building Codes and Engineers and Geoscience BC (EGBC) requirements and to document that the recommendations of the geotechnical report are followed.

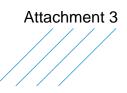
It is the contractor's responsibility to contact SNC-Lavalin a minimum of 48 hours in advance to notify us that a field review is required. In summary, geotechnical engineer field reviews are required for the following aspects of the work but not limited to:

- Review of site preparation;
- Review of grading, drainage system installation, Quality Assurance on structural grade fill placement, compaction testing, and road/parking construction;
- Review of engineered fill placement and compaction testing, including density testing of any engineered fill placed under slabs-on-grade and footings;
- Review of foundation subgrade prior to footing construction;
- Review of foundation backfill material and compaction; and
- Review of excavations in excess of 1.2 m depth as required by WorkSafeBC.

Since construction will happen in proximity of existing elevated guideway structures and existing utility lines, a detailed construction impact assessment, including a geotechnical instrumentation and monitoring plan, is required prior to the construction commencement.

A construction support scope and cost estimate can be provided in later stages of the project.





8 Closure

Please be advised that the contents of this report are based on information provided to us and results of field and laboratory testing performed by SNC-Lavalin, and our understanding of the proposed development as described in this report. If the development plans change, or if during construction the subsurface soil conditions are noted to be different than those described in this report, SNC-Lavalin should be notified promptly and the recommendations on the geotechnical aspects of the proposed development reviewed and adjusted accordingly. This report assumes that SNC-Lavalin will complete field reviews during construction.

SNC-Lavalin trusts that this report meets your expectations and requirements. Please do not hesitate to contact the signees of this report with any questions, comments or feedback concerning the content or presentation of this report.





9 Notice to Reader

This report has been prepared and the work referred to in this report has been undertaken by SNC-Lavalin Inc. (SNC-Lavalin) for the exclusive use of TransLink, who has been party to the development of the scope of work and understands its limitations. The methodology, findings, conclusions, and recommendations in this report are based solely upon the scope of work and subject to the time and budgetary considerations described in the proposal and/or contract pursuant to which this report was issued. Any use, reliance on, or decision made by a third party based on this report is the sole responsibility of such third party. SNC-Lavalin accepts no liability or responsibility for any damages that may be suffered or incurred by any third party as a result of the use of, reliance on, or any decision made based on this report.

The findings, conclusions, and recommendations in this report (i) have been developed in a manner consistent with the level of skill normally exercised by professionals currently practicing under similar conditions in the area, and (ii) reflect SNC-Lavalin's best judgment based on information available at the time of preparation of this report. No other warranties, either expressed or implied, are made as to the professional services provided under the terms of our original contract and included in this report. The findings and conclusions contained in this report are valid only as of the date of this report and may be based, in part, upon information provided by others. If any of the information is inaccurate, new information is discovered, site conditions change, or standards are amended, modifications to this report may be necessary. The results of this assessment should in no way be construed as a warranty that the subject site is free from any and all environmental impact.

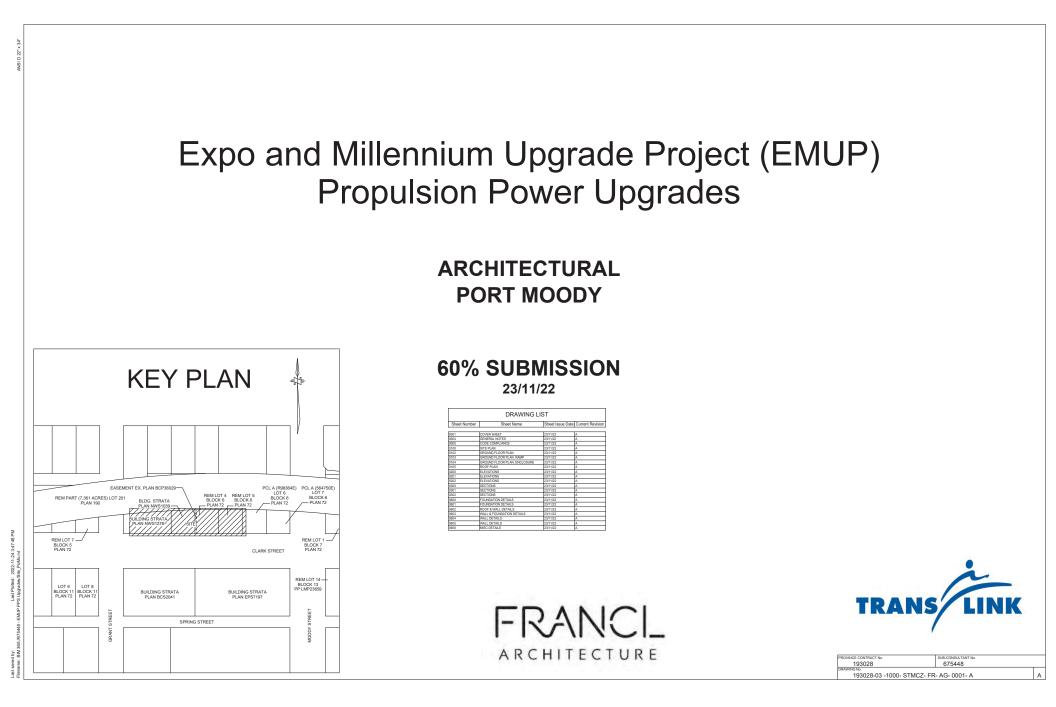
Any soil and rock descriptions in this report and associated logs have been made with the intent of providing general information on the subsurface conditions of the site. This information should not be used as geotechnical data for any purpose unless specifically addressed in the text of this report. Groundwater conditions described in this report refer only to those observed at the location and time of observation noted in the report.

This report must be read as a whole, as sections taken out of context may be misleading. If discrepancies occur between the preliminary (draft) and final version of this report, it is the final version that takes precedence. Nothing in this report is intended to constitute or provide a legal opinion.

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Appendix I

Drawings



ARCHITECTURAL SYMBOLS LEGEND

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GRIDLINE (VERTICAL GRIDLINES ARE NUMBERS.

HORIZONTAL GRIDLINES ARE LETTERS



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ISSUE/REVISION

A 2022-11-23

PA 2022-08-04

I/R DATE BY

60% SUBMISSIO

30% SUBMISSIO

DESCRIPTION

PROJECT NAME: EXPO AND MILLENNIUM UPGRADE PROJECT (EMUP) PROPULSION POWER UPGRADES PORT MOODY

PROJECT ADDRESS: OPPOSITE 2705 CLARKE ST IN PORT MOODY LEGAL DESCRIPTION

MAJOR OCCUPANCY: GROUP F. DIVISION 2 CONSTRUCTION TYPE: NON-COMBU BUILDING HEIGHT: 1 STOREY (7.0m) BUILDING AREA: 224 sq.m STREETS FACING: 1 SPRINKLER SYSTEM: N/A FIRE ALARM SYSTEM: TBC

REFERENCE CODES: (REFER TO CODE REPORT FOR FULL LIST OF APPLICABLE CODES) BCBC - BRITISH COLUMBIA BUILDING CODE 2018 ASHRAEJIES 90.1 2016 EDITION TRANSLINK BUILDING CODE CRITERIA 2019, FIXED GUIDEWAY RAPID TRANSIT SYSTEMS (TBCC)

GENERAL NOTES

THE GENERAL NOTES ARE TO BE READ IN CONJUNCTION WITH THE PROJECT SPECIFICATIONS AND ALL OTHER DISCIPLINES' DOCUMENTS. DO NOT SCALE FROM THE DRAWINGS, USE THE INDICATED DIMENSIONS, CONSULT WITH THE ARCHITECT FOR ANY UNCLEAR DIMENSION 1. THE GREEKA KYTES AND TE READ IN COMUNICIDAW WITH THE PROJECT SPECIFICATIONS AND ALL OTHER DISCPUTED BOCUMENTS DEFORE DESCRIPTION OF THE COMENTS OF THE INDUSTRY DISCRIPTION OF THE ADVECTOR THE ADVECTOR TO ANY UNCLEAR DIMENSION 2. ALL DIMENSIONS ARE INTER CLIN.O. 3. ALL DIMENSIONS SALE BLE VERIFIED AND COORDINATED ON SITE PRIOR TO PROCEEDING WITH CONSTRUCTION. 4. ALL DIMENSIONS SALE BLE VERIFIED AND COORDINATED ON SITE PRIOR TO PROCEEDING WITH CONSTRUCTION. 5. ADDITECTURAL DOCUMENTS GOVERN OVER STRUCTURAL FOR LAYOUT OF ALL COMPONENTS, STRUCTURAL DOCUMENTS STALL GOVERN 5. ADDITECTURAL DOCUMENTS GOVERN OVER STRUCTURAL FOR LAYOUT OF ALL COMPONENTS, STRUCTURAL DOCUMENTS STALL GOVERN 6. ADDITECTURAL DOCUMENTS GOVERN OVER STRUCTURAL FOR LAYOUT OF ALL COMPONENTS, STRUCTURAL DOCUMENTS GOVERN OVER STRUCTURAL FOR LAYOUT OF ALL COMPONENTS, STRUCTURAL DOCUMENTS GOVERN OVER STRUCTURAL FOR LAYOUT OF ALL 6. STRUCTURAL DOCUMENTS GOVERN OVER STRUCTURAL FOR LAYOUT OF ALL 6. STRUCTURAL DOCUMENTS STRUCTURAL FOR LAYOUT OF ALL DOCUMENTS, STRUCTURAL DOCUMENTS, STRUCTURAL DOCUMENTS, STRUCTURAL GOVERN 6. THE APPLICABLE BEOLITICON. 6. THE APPLICABLE BEOLITICON. 6. THE APPCHECTS SHALL BE NOTFIED IN WITING OF ANY DISCREPANCIES AND REQUEST CLARIFOLTON IN WITING ON INTERGON INTERGON. 7. THE APPCHECT SHALL BE NOTFIED IN WITING OF ANY DISCREPANCIES AND REQUEST CLARIFOLTON IN WITING ON ANY DESTING COMPONENTS AND EFFORMED STRUCTURE. LONG 7. WERE EXISTING COMPONENTS AND REFECTO ON CHARGES WITHOUT OF THE ADSTRUCTURAL LAYOUT ALL SPECIFICS THE URAL 8. WERE EXISTING COMPONENTS AND REFECTION OF ALL AMERICAS IN ACCORDANCES WITH ACCEPTOD INDISTY PRACTICE UN OF 8. WERE EXISTING COMPONENTS AND REFECTION OF ALL AMERICAS IN ACCORDANCES WITH ACCEPTOD INDISTY PRACTICE UN OF 8. STRUCTURE DOCUMENTS AND REFECTION OF ALL AMERICAS IN ACCORDANCES WITH ACCEPTOD INDISTY PRACTICE UN OF 8. STRUCTURE DOCUMENTS AND REFECTION OF ALL AMERICAS IN ACCORDANCES WITH ACCEPTOD INDISTY PRACTICE UN OF 8. DIFFORMED ADVENTS AND REFECTION OF ALL AMERICAS I

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REFERENCES

APPR

				ABBRE	VIATIONS
WALL ASS		INSULATED FACE BRICK WALL	NOTES	AARU AFF AVB	AUTOMATIC ASSURED RECEPTIVITY UNIT ABOVE FINISH FLOOR AIR VAPOLIR BARRIER
W1c		89mm FACE BRICK W ANTI-GRAFFITY COATING 25mm CAVITY THERNALLY BROKEN MASONRY TIE 56mm SRUH-RIGID INSULATION	FRR PROVIDED BY STRUCTURAL WALL	AHU ALUM ANOD ARCH	AIR HANDLING UNIT ALUMINIUM ANODIZED ARCHITECTURAL
FRR -	-	- AIR/WATER/VAPOUR BARRIER		@ CIP	AT CAST IN PLACE
STC -	-	- STRUCTURAL WALL - REFER TO STRUCTURAL DRAWINGS		CL	CENTRE LINE
J-VALUE				CLR COL	CLEAR COLUMN
R-VALUE 11.78				CONC CMU	CONCRETE CONCRETE MASONRY UNIT
	GRAPHIC	STANDING SEAM METAL CLADDING	NOTES	CJ CONT	CONTROL JOINT
W1d		STANDING SEAM METAL PANEL 150mm Z-GIRT (HORIZONTAL)	FRR PROVIDED BY STRUCTURAL WALL	DIA OR Ø	CONTINUOUS DIAMETER
-		- 225mm THERMALLY BROKEN CLIPS W VERTICAL Z-GIRT	WALL	DN DWG	DOWN DRAWING
FRR		 65mm SEMI-RIGID INSULATION (IN BETWEEN CLIPS) AIR/WATER/VAPOUR BARRIER 		EA ELEV	EACH ELEVATION
STC	I I	- STRUCTURAL WALL - REFER TO STRUCTURAL DRAWINGS		ELEC	ELECTRICAL
U-VALUE		Si otti ili do		ERD EQ	EMERGENCY ROOF DRAIN EQUAL
R-VALUE 11.78				EJ	EXPANSION JOINT EXTERIOR
				FF	FINISH FLOOR
				FA FAP	FIRE ALARM FIRE ANNUNCIATOR PANEL
				FE	FIRE EXTINGUISHER FIRE EXTINGUISHER CABINET
	GRAPHIC	CAST-IN-PLACE CONCRETE WALL	NOTES	FHC	FIRE HOSE CABINET
W2b	1.1.1.1.1.1.1.1	SIZES PER STRUCTURAL DESIGN EXPOSED NATURAL CONCRETE FINISH		FH FR	FIRE HYDRANT FIRE RATED
-	<u>11.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1</u>			FRR	FIRE RESISTANCE RATING FLOOR
FRR 2HR				FD	FLOOR DRAIN
STC -				FURR GALV	FURRING GALVANIZED
ROOF ASS				GA GWB	GAUGE GYPSUM WALL BOARD
NOOI AGO				HDWR	HARDWARE
	GRAPHIC	STANDING SEAM ROOF	NOTES	HB	HOSE BIBB
R2		STANDING SEAM METAL ROOF 175mm THERMALLY BROKEN CLIPS W HORIZONTAL	CEILING ULC DESIGN- 1506	HR MAU	HOUR MAKEUP AIR UNIT
		HAT-TRACK - VAPOUR PERMEABLE SEPARATION MEMBRANE		MH MAX	MANHOLE MAXIMUM
		- 150mm RIGID INSULATION IN BETWEEN CLIPS		MECH	MECHANICAL
STC		AIR/WATER/VAPOUR BARRIER 12.5mm DECK OVERLAY BOARD		MIN MISC	MINIMUM
FRR 2HR STC U-VALUE		AIR/WATER/VAPOUR BARRIER 12.5mm DECK OVERLAY BOARD STEEL DECK_REFER TO STRUCTURAL DRAWINGS		MISC N/A	MINIMUM MISCELLANEOUS NOT APPLICABLE
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UMINIUM NODIZED RCHITECTURAL AST IN PLACE ENTRE LINE LEAR NORTH ARROW ONCRETE ONCRETE MASONRY UNIT ONTROL JOINT ONTINUOUS IAMETER OWN BUILDING ELEVATION CALLOUT DETAIL NUMBER RAWING ACH LEVATION LECTRICAL EMERGENCY ROOF DRAIN (QUAL EXPANSION JOINT EXTERIOR INISH FLOOR IRE ALARM IRE ANNUNCIATOR PANEL IRE EXPLOSION BUILDING SECTION CALLOUT SHEET NUMBE (1 A10 IRE EXTINGUISHER TRE HOSE CABINET IRE HYDRANT IRE RATED IRE RESISTANCE RATING LOOR CALLOUT LOOR DRAIN 'LOOR DRAIN UURRING GALVANIZED GAUGE SYPSUM WALL BOARD Name FLOOR LEVEL ARDWARE IORIZONTAL IOSE BIBB Elevation SPOT ELEVATION IOUR MAKEUP AIR UNIT MAREUP AIR UNIT MANHOLE MAXIMUM MECHANICAL MINIMUM MISCELLANEOUS WALL. ROOF TAG IOT APPLICABLE IOT IN CONTRACT IOT TO SCALE IN CENTRE 0000 ROOF TAG . FLOOR TAG /ERHEAD (101) DOOR TAG AINT RE-CAST CONCRETE RE-FINISHED METAL ROPULSION POWER STATION REFLECTED CEILING PLAN REQUIRED RIGHT OF WAY ROOM ROOF DRAIN ROUGH OPENING SANITARY GENERAL DOOR NOTES ALL EXTERIOR DOORS ARE TO CONFORM TO THE ASTM F476-84 STANDARD. DOOR HARDWARE IS TO BE REKEYED AFTER CONSTRUCTION SELF ADHERED INSULATED EXTERIOR METAL DOORS TO CONFORM TO CAN/CGSB-92.5-M RATED DOORS AND FRAMES SHALL HAVE ULC LABEL IMILAR SIMILAR SPECIFICATIONS STAINLESS STEEI STRUCTURAL ARTED DOORS AND FRAMES SHALL HAVE LUC LABEL
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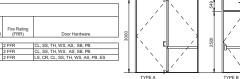
THRESHOLD WEATHER STRIPPING

PUSH BAR SECURITY ASTRAGAL FULL

Author

SPRING BOLT CARD READER

LENGHT ELECTRIC STRIKE



KEY PLAN

REGISTRATION

PROJECT DRAWING INFORMATION EMUP DRAWN BY DESIGNED BY Designer PROPULSION CHECKED BY Checker POWER APPROVED BY Approve UPGRADES

PORT MOODY

SB CR PB AS

ES

PROJECT NUMBER 193028 (SNC #675448) SHEET TITLE GENERAL NOTES

FENCE, TYPE TBC

DRAWING NUMBER

CONTRACT NUMBER

EC-1

193028-03 -1000- STMCZ- FR- AG- 0004-

TRANS//LINK

Mark Mark Height Width

3000

2500 45

2500 900 45

MECHANICAL SNC-Lavalin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 327 604 662-3555 tel 604 662-7688 fax

Door Schedule

Door and Frame Finish

INSM PT-2 INSM PT-2 INSM PT-2

INSM PT-2

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CONSULTANTS

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SNC-Lavalin

Srvc-Lavain Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 327 604 662-3555 tel 604 662-7688 fax www.snclavalin.com

CONSULTANTS ELECTRICAL

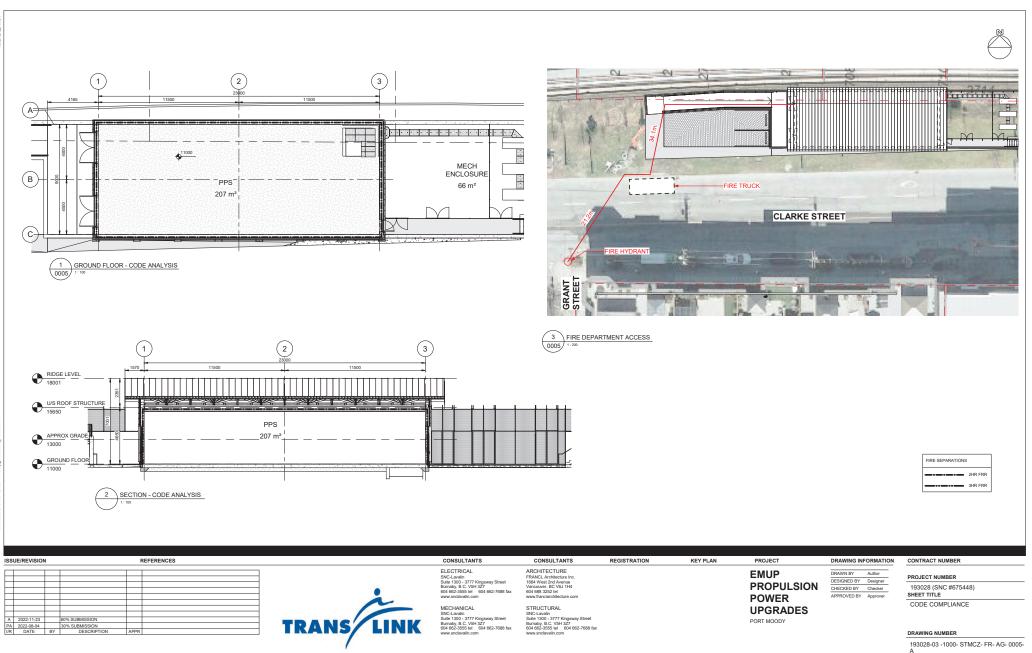
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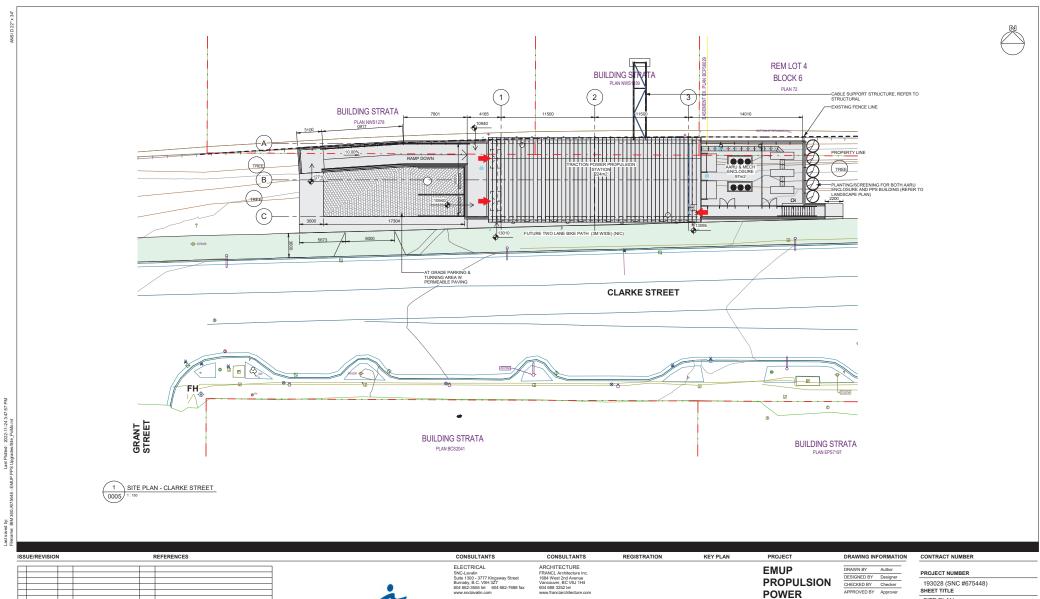
Height Width

3100 2600 PSF 3100 2600 PSF

3100 1000 PSF

WALL/ROOF/FLOOR ASSEMBLY





MECHANICAL SNC-Lavalin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 327 604 662-37655 tel 604 662-7688 fax www.snclavalin.com

TRANS

LINK

A 2022-11-23

60% SUBMISSION

APPR

 PA
 2022-08-04
 30% SUBMISSION

 I/R
 DATE
 BY
 DESCRIPTION

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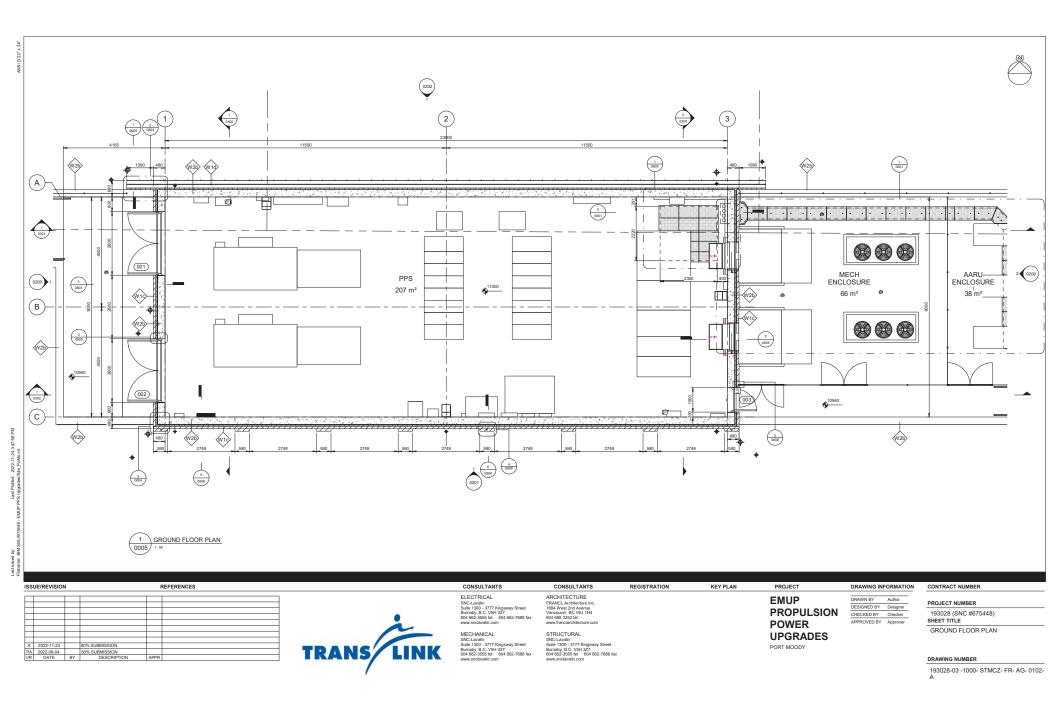
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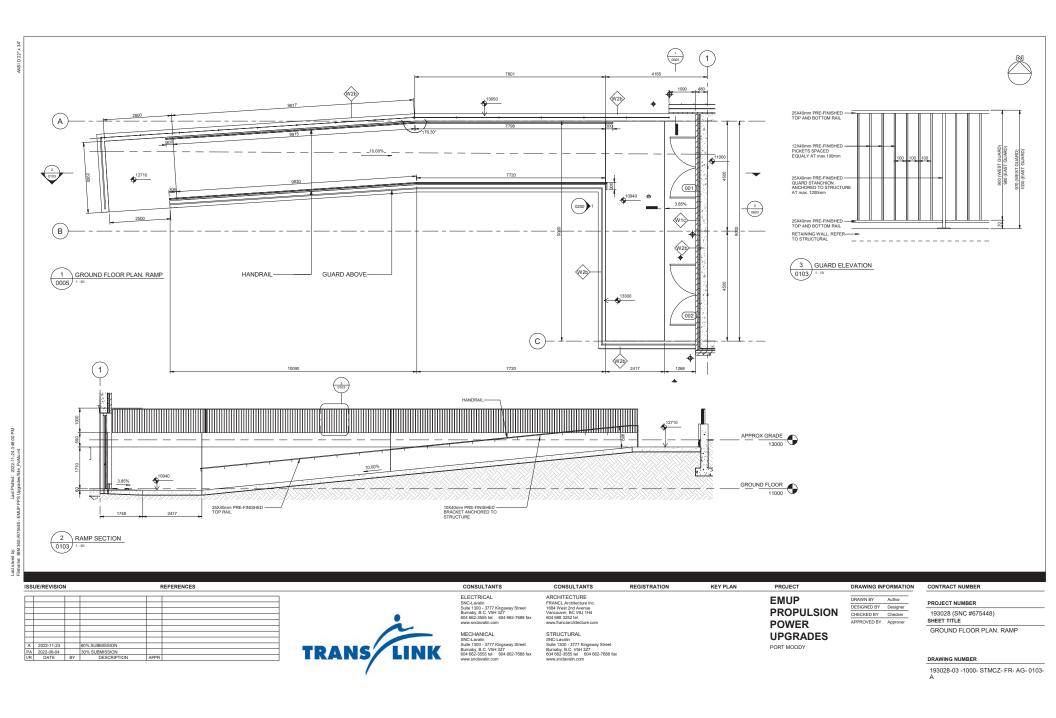
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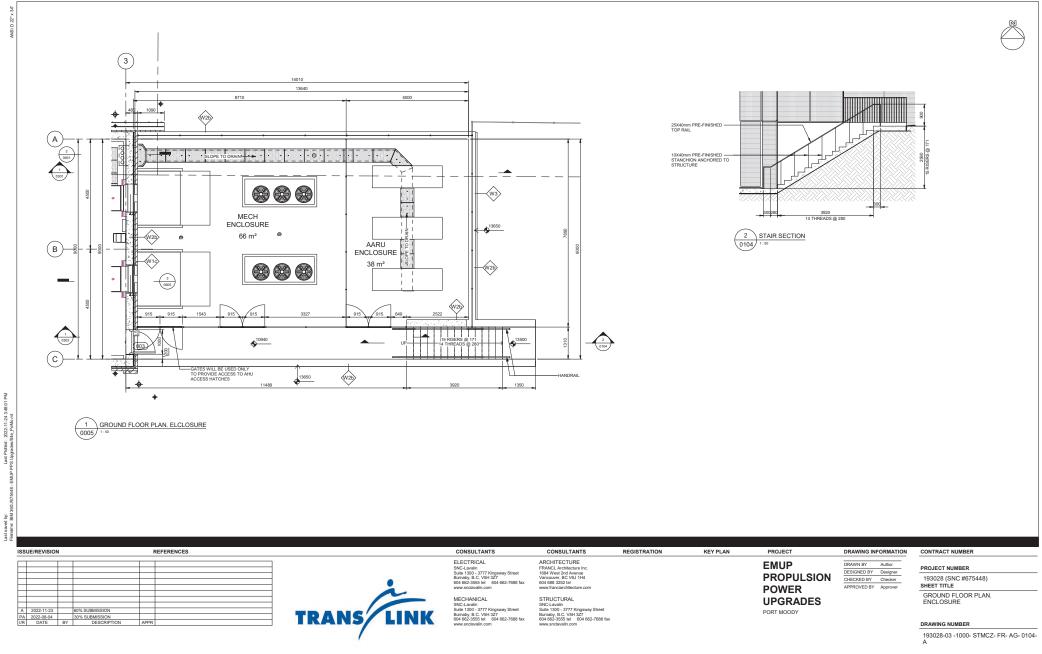
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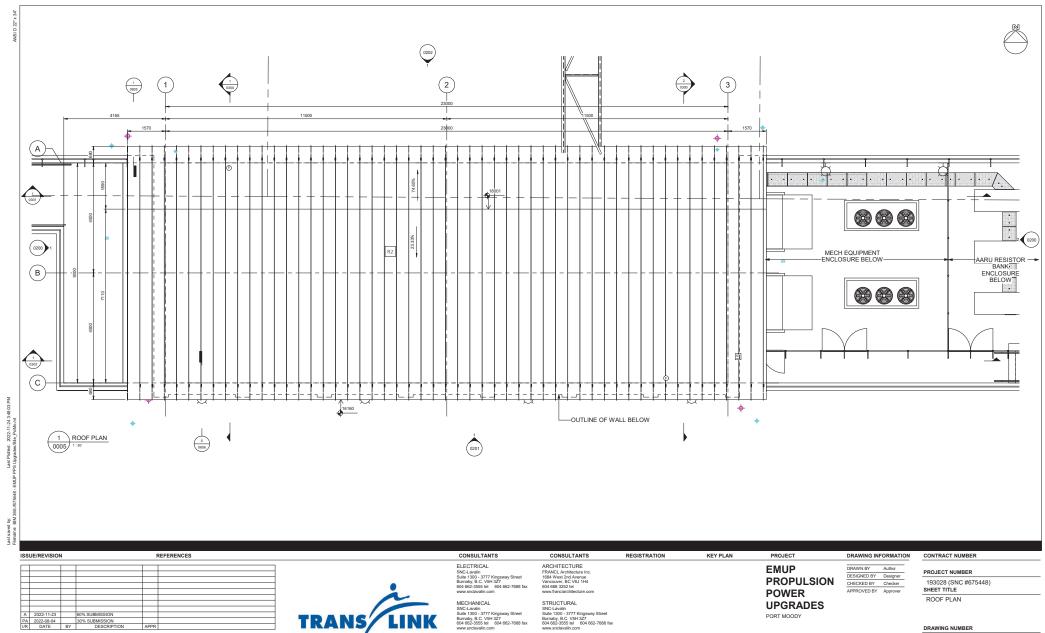








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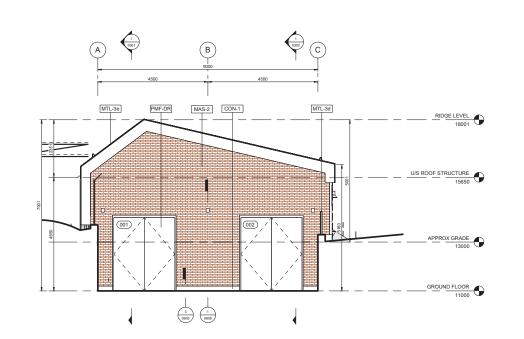
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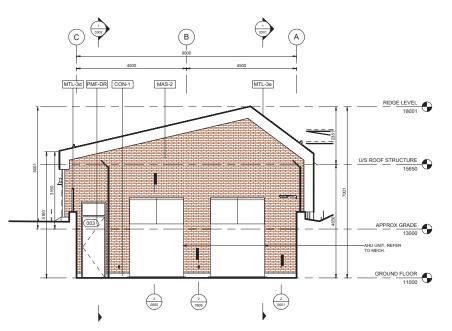
A 2022-11-23 60% SUBMISSION PA 2022-08-04 30% SUBMISSION U/R DATE BY DESCRIPTION

APPR

PORT MOODY

193028-03 -1000- STMCZ- FR- AG- 0105-Α





NSID 22"



REGISTRATION

KEY PLAN

ELEVATION	MATER	IAL LE	GEND

- FACE BRICK, COLOR TBC GUARD WITH PICKETS BLACK, COLOR TBC STANDING SERVIE BLACK, COLOUR TBC GUARDW PICKETS BLACK, COLOUR TBC CONCRETE FACED RIGH INSULATION INSULATED METAL DOOR # PRESSED METAL FRAME, FARATED, COLOUR TBC FENCE, TYPE TBC

CONTRACT NUMBER

PROJECT NUMBER

SHEET TITLE

ELEVATIONS

193028 (SNC #675448)

- MAS-2 MTL-3f MTL-3e MTL-3d CON -1 PMF-DR
- FC-1

ISSUE/REVISION

Α	2022-11-23		60% SUBMISSION		
PA	2022-08-04		30% SUBMISSION		
I/R	DATE	BY	DESCRIPTION	APPR	
_					

REFERENCES



CONSULTANTS ELECTRICAL

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MECHANICAL SNC-Lavalin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 327 604 662-355 tel 604 662-7688 fax www.snclavalin.com

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CONSULTANTS

STRUCTURAL SNC-Lavalin Suite 1300 - 3777 Kingsway Street Burnaby, B.C. V5H 327 604 662-3555 tel 604 662-7688 fax www.snclavalin.com

PROJECT EMUP PROPULSION

POWER UPGRADES PORT MOODY

DRAWING NUMBER

DRAWING INFORMATION

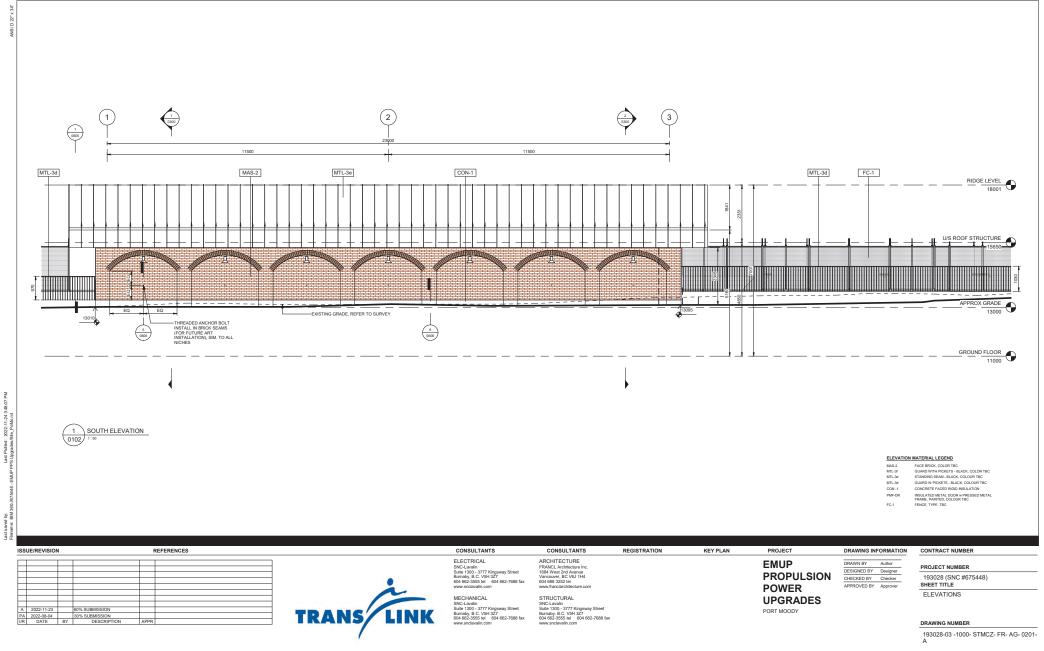
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DESIGNED BY Designer

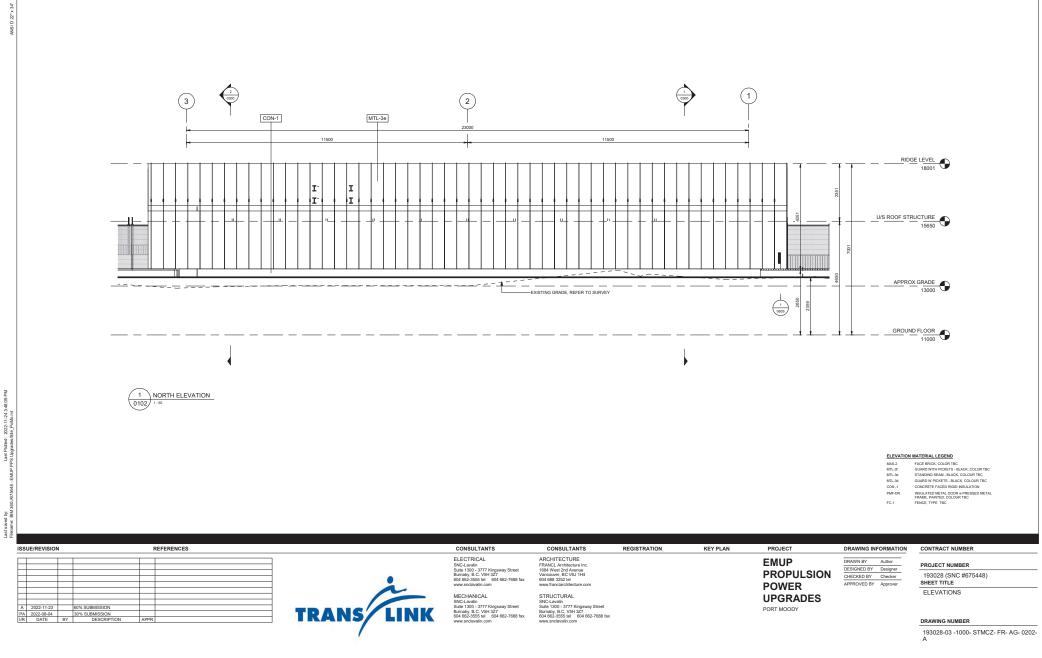
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APPROVED BY Approve

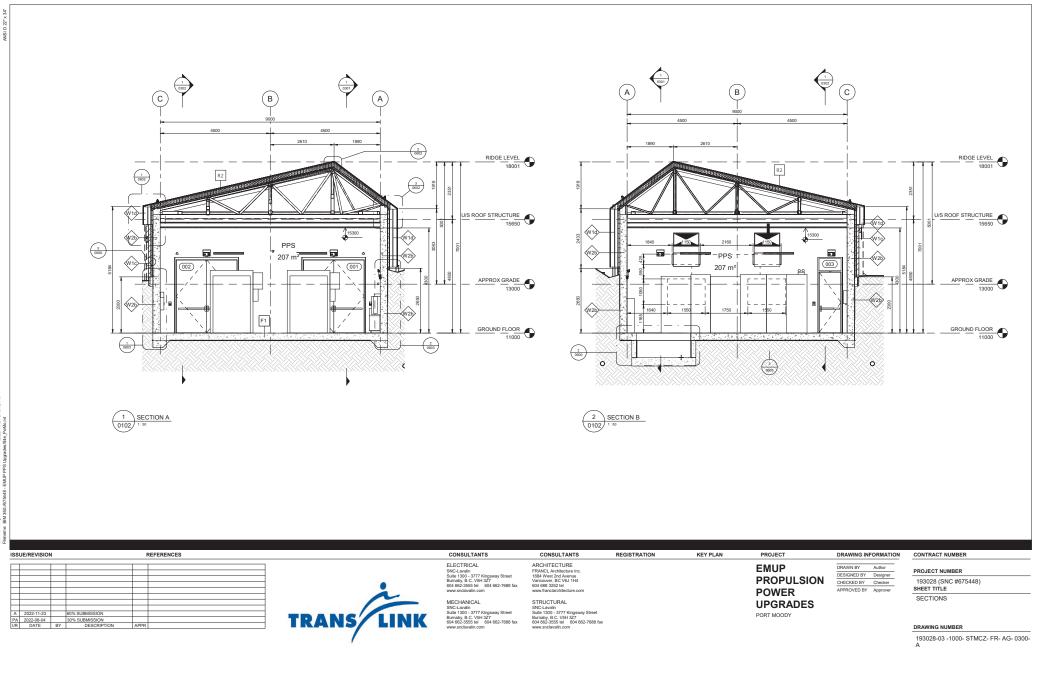
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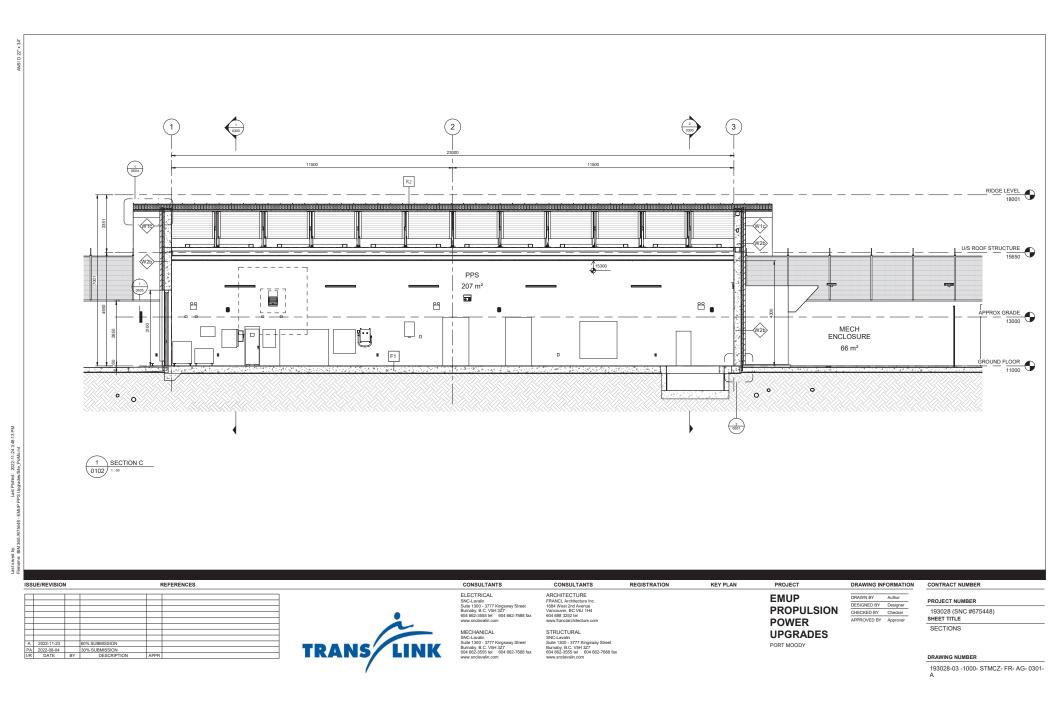


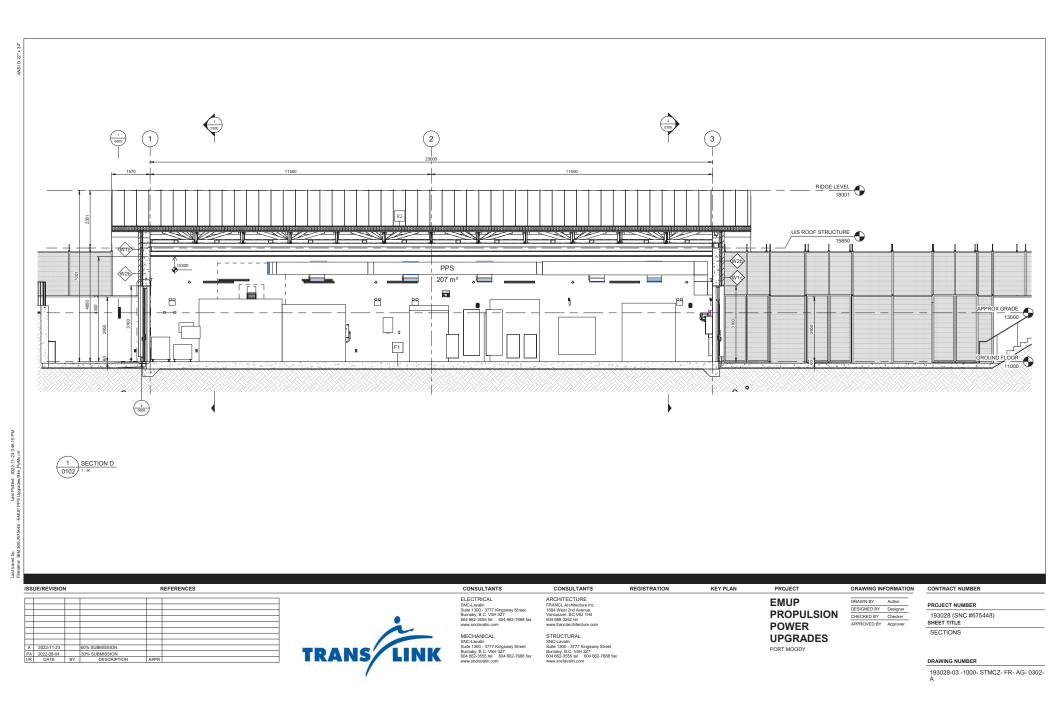
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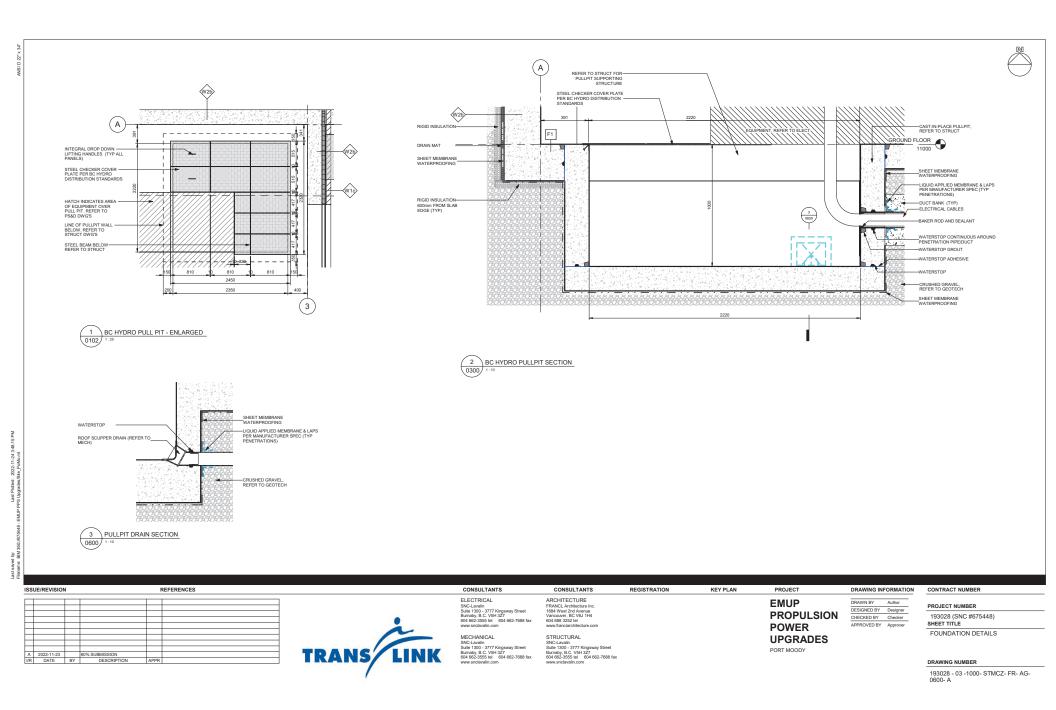


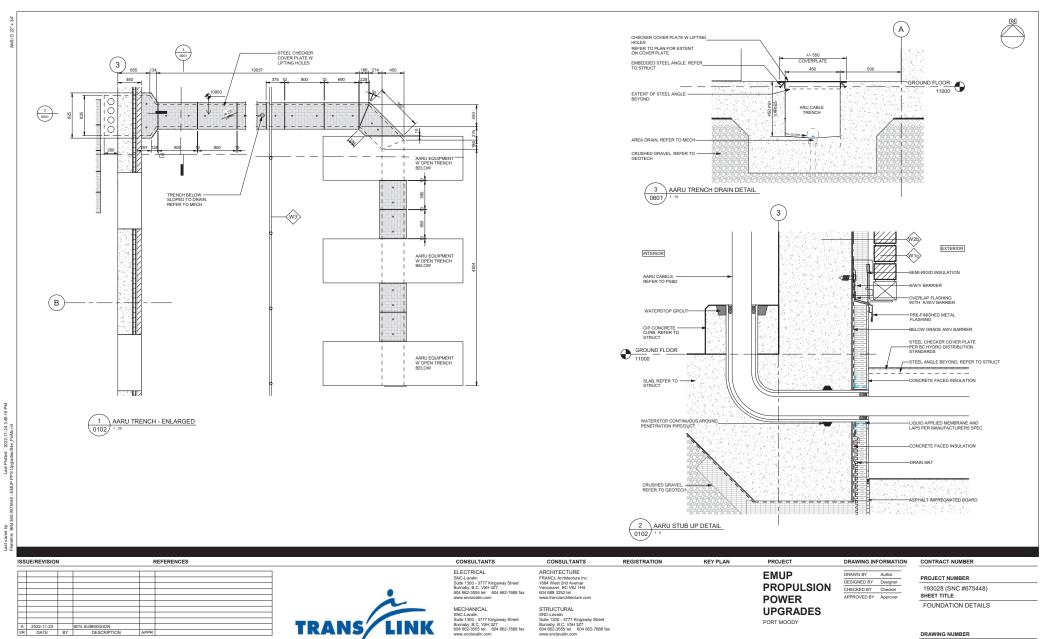
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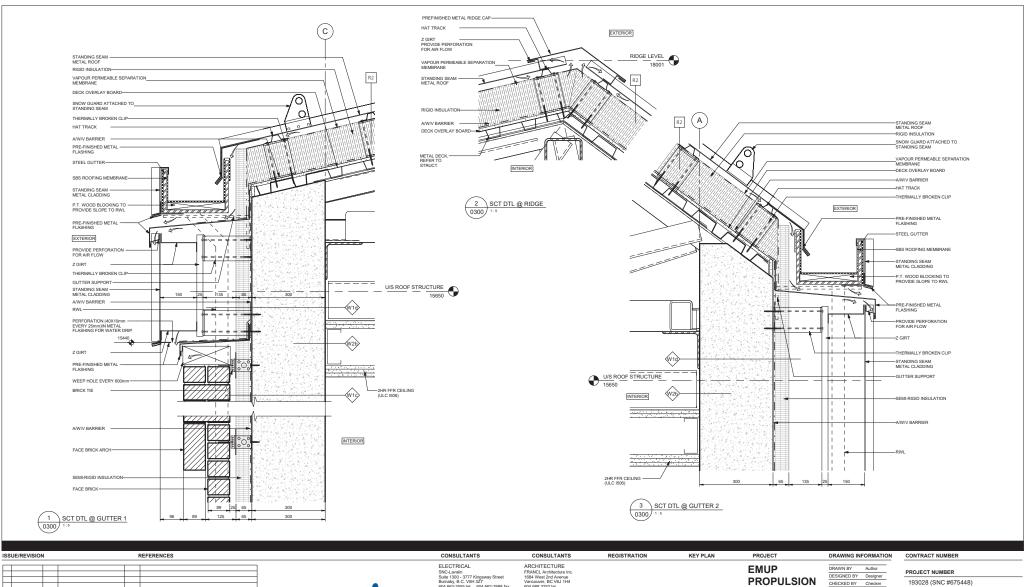
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FRANCL Architecture Inc. 1684 West 2nd Avenue Vancouver, BC V6J 1H4 604 688 3252 tel www.franclarchitecture.co

POWER UPGRADES

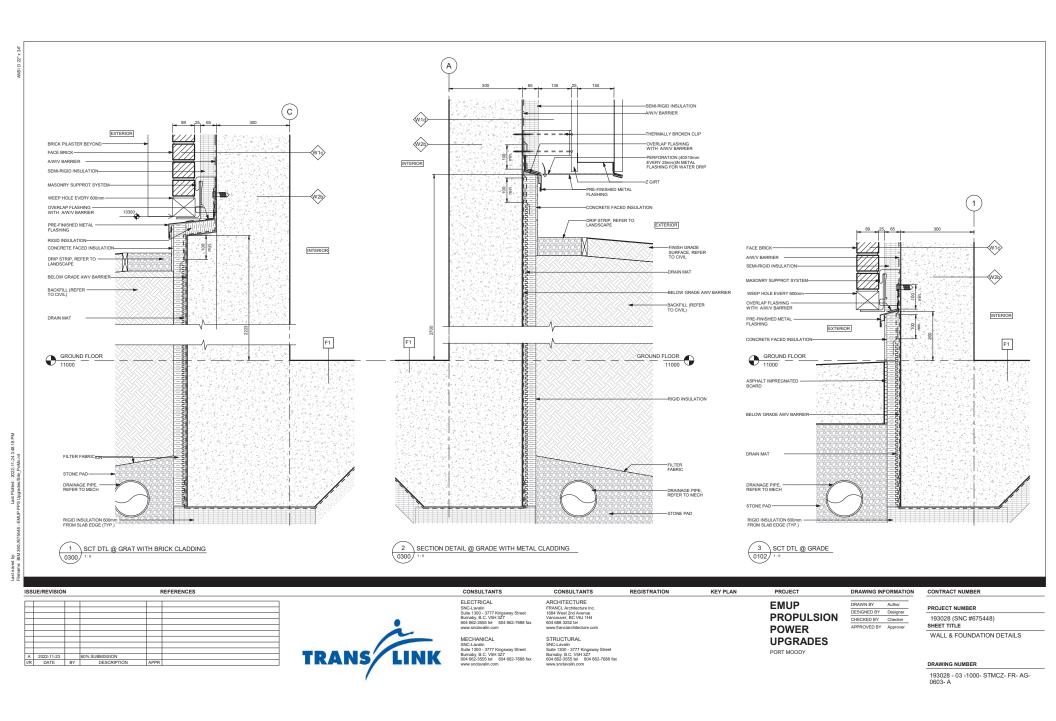
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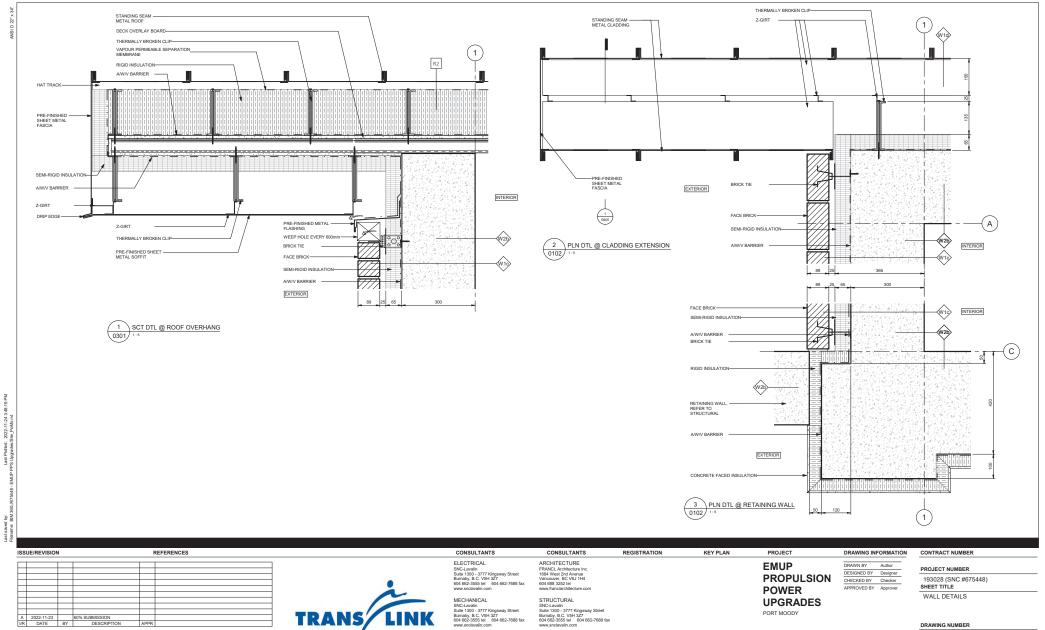
SHEET TITLE ROOF & WALL DETAILS

APPROVED BY Approver

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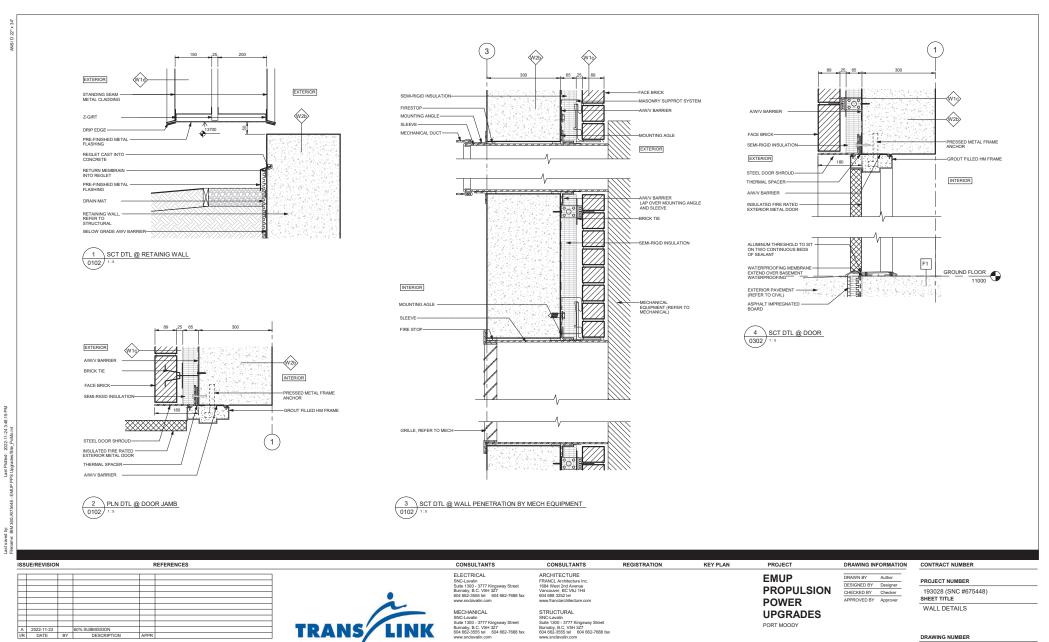
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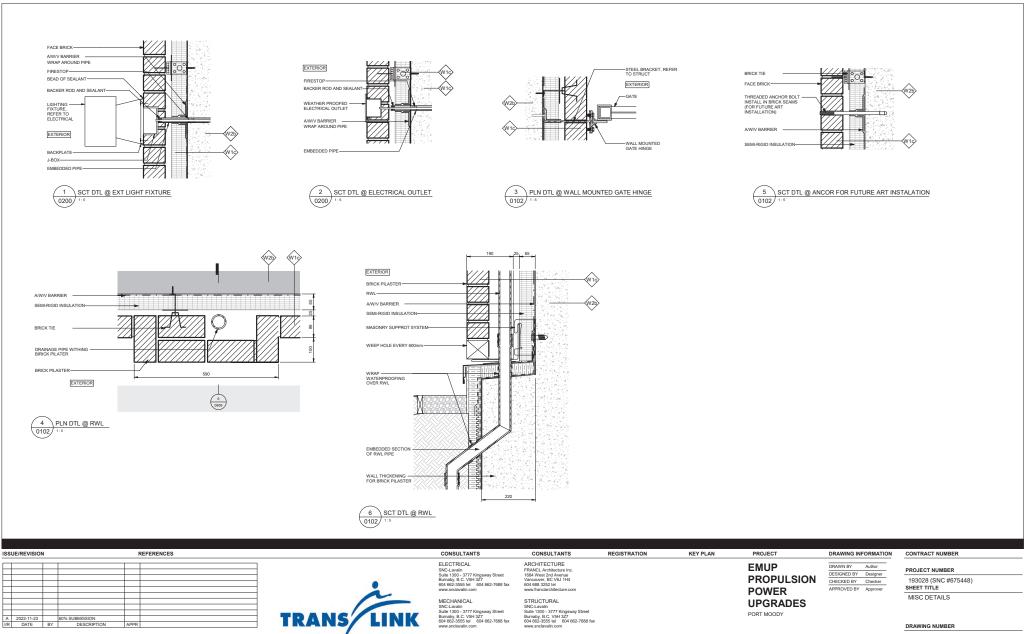
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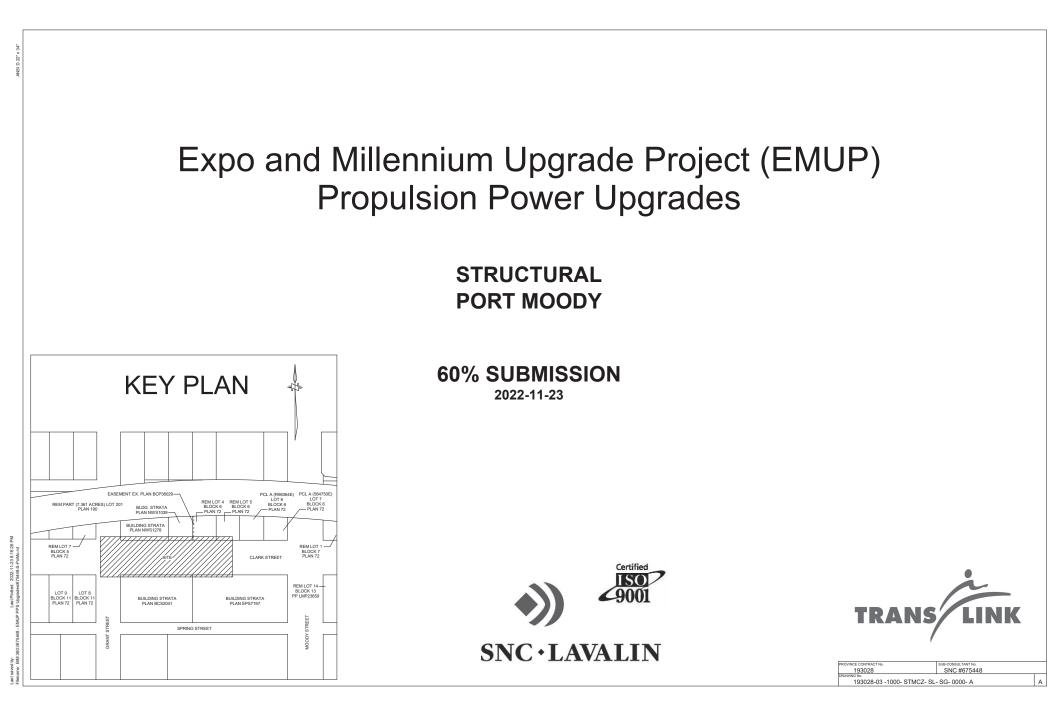
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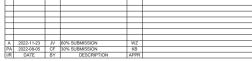
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STRUCTURAL NOTES	E MEASUREMENT IS USED ON ALL DRAWINGS. ELEVATIONS AND STATIONS	CONCRETE NOTES 1. CONCRETE SHALL CONFORM TO CSA STANDARD A23.1-19/A23.2-19	FOUNDATIONS 1. FOUNDATIONS ARE DESIGNED IN ACCORDANCE WITH GEOTECHNICAL DESIGN REPORT	ABBREVIATIONS
OTHERWISE, THE CONT	S AND ALL OTHER DIMENSIONS ARE SHOWN IN MILLIMETRES UNLESS NOTED TRACTOR SHALL VERIFY DIMENSIONS BEFORE CONSTRUCTION AND ES BEFORE PROCEEDING WITH THE WORK. DO NOT SCALE DRAWINGS.	ALL CAST-IN-PLACE CONCRETE SHALL HAVE PROPERTIES AS FOLLOWS: COMPRESSIVE MAX. WCM EXPOSURE MAX. SIZE OF	REFERENCED UNDER DESIGN DATA OF THE GENERAL NOTES. 2. BEARING SURFACES MUST BE APPROVED BY THE GEOTECHNICAL ENGINEER IMMEDIATELY	APPROX - APPROXIMATELY ARCH - ARCHITECTURAL AVE - AVENUE
 READ THE STRUCTURAL DOCUMENTS. 	L DRAWINGS IN CONJUNCTION WITH ALL OTHER PERTINENT CONTRACT	ELEMENT STRENGTH 28 DAYS (MPa) NAX: WUM EAP/USURG: CLASS COARSE AGGREGATE AIR CONTENT AGGREGATE FOUNDATION 30 0.5 F1 20 5-8%	BEFORE FOOTING CONCRETE IS PLACED. 3. FOOTING ELEVATIONS AND SIZES ARE SUBJECT TO REVISION WHERE SITE CONDITIONS DIFFER FROM ANTICIPATED SOIL CONDITIONS	BH - BOREHOLE BW - BOTH WAY C/L - CENTRELINE C/P - CAST IN PLACE
CONTRACTOR TO COMP CONSTRUCTION REPORT CONSTRUCTION REPORT CONSTRUCTION . URITY THE STRUCTUR 5. VERIFY SIZE AND LOCA 6. DRAWINGS SHOW COM CONSTRUCTION LOAD CONSTRUCTION LOAD . CONSTRUCTION LOAD . CONSTRUCTION LOAD TO THE DESIGNER FOR REGISTERED IN THE P TELEPORARY SHORING.	ECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR DIMENSIONS OF NO OTHER COMPONENTS NOT SANUL OTHER CHUCINAL DRAWINGS. SIRVE ECURPRENT DURANSION OF THE STRUCTURAL DRAWINGS. SIRVE ECURPRENT DRAWINGS AND ALL OTHER CONTEXPONENT TO SCREENANCES AND GETAL APPROVIL PROOT TO DROCEEDING WITH WAL DESIGNATES AND GETAL APPROVIL PROOT TO DROCEEDING WITH DETED STRUCTURES AND GETAL APPROVINGS, RECESSES ETC INFERTD STRUCTURES GAV. THE CONTRACTOR IN RESONSIBLE FOR THE C. SHORMS CANN. TO CONTRACTOR IN RESONSIBLE FOR THE C. SHORMS CANN. DEVINES AND CONTRACTOR IN RESONSIBLE FOR THE C. SHORMS CANN. DEVINES OF TRACTOR IN RESONSIBLE FOR THE C. SHORMS CANN. DEVINES OF TRACTOR IN RESONSIBLE FOR THE C. SHORMS CANN. DEVINES OF TRACTOR OF RESIST NOS. DO SECURING TEMPORARY STRUCTURES DURANG UNDE OF RETIRE TO COMPARE TO COMPACT TO RESIST NOS. DO SHOLL THE CONTRACTOR OR REACING SHALL BE SUBMITTED IREVIEW THE CONTRACTOR SHALL RETAIN A PROFESSIONAL ENGINEER AND BREQUIRIES OLUMIATE TO DESIGN AND TAKE RESPONSIBILITY FOR AND BREQUIRIES OLUMIANT TO DESIGN AND TAKE RESPONSIBILITY FOR AND BREQUIRIES TO COMPLETE THE CONSTRUCTION.	EXTERIOR SLAB (35 DAYS) 0.4 C1 20 5-8% INTERIOR SLAB ON GRADE GRADE SUBPRINCE SUBPRINCE AND ROOF OVERLAY 30 - N 20 NONE 2. CONCRETE COVERLAY 30 - N 20 NONE 3. CONCRETE COVERLAY - N 20 NONE 3. CONCRETE COVERLAY - N 20 NONE 4. CONCRETE COVERLAY - N 20 NONE CONCRETE COVERLAY - 0 NONE SUBSENDED AS AND 24YOUR FIRE RATING REQUIREMENT SHALL BE PROVIDED AS FOLLOWS, UNLESS NOTED OTHERWISE - 00 mm 0. CONCRETE CAST AGAINST FORMVORK MAD SUBSECUENTLY EXPOSED TO CARAFIT OR WEATH OF WEATH ST WEAT	E FOOTINGS AND SLAB-ON-GRADE BEARING ON COMPACTED, GRANULAR STRUCTURAL FLL SHALL BE COMMACTED TO A MONITED PROCTOR SA OUTLINED IN THE GOTICSINGLE DURINGERS FOUNDATED TO AN OWNER PROCTOR SA OUTLINED IN THE GOTICSINGLE DURINGERS FOUNDER STATUS ON TESTICITION SPECIAL TO BE MADE BOTTED TO THE PLACEBER ON BOTTING THICKLYRE OF COMPACTED INTIMINIANS CONSIDER OF MADE PLACEBER ON STATUS ALL POOTING WALLS SHALL BE CARTIERED RELOW CAU WALLS UNLESS DETAILED OTHERWISE. DOWNELS STALL BE CARTIEND BELOW CAU WALLS UNLESS DETAILED OTHERWISE. DOWNELS TO MATCH VERTICAL BARS. FOUNDATION WALLS SHALL BE CARTIERED BELOW CAU WALLS UNLESS DETAILED OTHERWISE. DOWNELS TO MATCH VERTICAL BARS. FOUNDATION WALLS SHALL BE CARTIERED BELOW CAU WALLS UNLESS DETAILED OTHERWISE. CONCORTE CONCETE: CONTROL PERION ON SERVICE CONFERENCE TO CONDINING PERION ON SERVICES OF THE WALL AFTER CONCETE: CONTROL PERION ON SERVICES OF THE WALL ATTER CONCETE CONFIDENCE PERION ON SERVICES OF THE WALL ATTER CONCETES CONFERENCE CONCORTE ON THE ARCHITECTURAL AND OTHER CONSULT TANTS DRIVINGS FOR GROUND ELEVATIONS, DRAINING BLOPES, WATERPROCIPING, ETC. CONCORTENCE MOLTS DIVERSEDED IN CONCEPTE IN ACCORDANCE WITH THE FOLLOWING	CLR - CLEAR CMU - CONCRETE MASONRY UNIT CAL - CONCRETE MASONRY UNIT CAL - CONCRETE MASONRY UNIT CAL - CONCRETE MASONRY UNIT DIA DEPTH STANDARDS ASSOCIATION DIA DAMANG ELEVATOR DWG - DAMANG E - DAMANG FF - FARFACE FF - FARFACE HOR - HORWAY HB - INBOLNO NV - INGERVITON MPa - KLOPASCAL LHS - LEFERVITON MPA - LONGITUDINAL HOR - LEFERVITON MPA - LONGITUDINAL LHS - LEFERVITON MECH - LONGITUDINAL LHS - LEFERVITON MECH - MECHANCAL
a. DRAWING b. GENERAL c. PROJECT 9. DIMENSIONS GIVEN ON	SS AND DRAWING NOTES NOTES CONSTRUCTION SPECIFICATIONS THE PLANS ARE MEASURED AT A REFERENCE TEMPERATURE OF 15°C UNO	 PRIOR TO POURING CONCRETE THE CONTRACTOR SHALL CHECK ALL OPENINGS, ANCHOR BOLTS, INSERTS AND EMBEDDED TEMS REQUIRED FOR MECHANICAL, ELECTRICAL OR UTILITY SUPPORT PURPOSES AS SHOWN ON THE DRAWINGS, ANY DISCREMANCIES NOT REPORTED TO THE ENGINEER FOR CLARIFICATION SHALL BECOME RESPONSIBILITY OF THE CONTRACTOR. 	GUIDELINES EXCEPT WITH THE APPROVAL OF THE STRUCTURAL ENGINEER. L CONDUTS a. LOCATE BETWEEN REINFORCING STEEL LAYERS. b. MAXIMUM SEE IN ONE LAYER TO BE TAY OF THE CONSTITUTIES A THORNESS.	MIN - MININUM mm MLILMETRE MEGAPASCAL MPa - MEGAPASCAL NF - NEAR SDE NS - NEAR SDE NTS - NOT SCALE
DESIGN DATA 1. DESIGN CODES: BRITISH COLUME CANICSA 3304.1 CANICSA 233.31	ARE MEASURED HORIZONTALLY UNO. BIA BUILDING CODE 2018 (BCBC) 19 DESIGN OF CASCRETE STRUCTURES B DESIGN OF CASCRETE STRUCTURES	GROUT SHALL BE OF NON-SHRINK, NON-METALLC TYPE, IMIMIMIA J DAYS COMPRESSIVE STREMATH SHALL BE A 40 MPG UNLESS NOTED THERWIJSE. CONCRETE FINISHES: BASE SLAB TOP TOOPTON TOOPTOOPTOOPTONTOOPTOOPTOOPTOOPTOOPTOOPT	e. CHOSSING OF THREE LAVERS WILL NOT BE PERMITTED. e. CLEAR SPACE DEWIREN PARALLE. CONJUITS SHALL BE ONE DUMETER OR 40 MINMUM HOUTH BORDONTALLY AND VERTICALLY. E. DLCATE SETWICEN PARALLE. CONJUITS SHALL BE ONE DUMATER OR 40 MINMUM E. DLCATE SETWICEN VERTICAL DE UT 10 ET 10 OF 116 ELAS HILDONESS. b. MAXMUM SEZE TO BE 10 OF 116 ELAS HILDONESS. e. CLEAR SPACE BETWEEN UDCITS OF BE 300 mm. #. PIES setting of the BIVER DUCITS OF BE 300 mm. #. REINFORCEME AND SHALL BE ONESCHALED TO MISS THE PIPE AS MUCH AS POSSIBLE. setting of the SPACE TO BE 10 OF 116 ELAS HILDONESS. e. CLEAR SPACE MASS HILD AND VERTICAL DE TOM SIST THE PIPE AS MUCH AS POSSIBLE. setting of the SPACE AND SHALL BE ONES THE PIPE AS MUCH AS POSSIBLE.	OIB - OUTBOUND OIC - ON CATUREOUND OIC - ON CATUREOUND RE - REVISION REV - REVISION REV - REVISION ROM - REVISION REV - REVISION REV - REVISION REV - REVISION STA - STATION STB - STANDARD TBD - TO BE DETEMINED
2. DESIGN LOADS: FLOOR LIVE LOAD CEILING ATTACH	DING CODE CRITERIA (TLBCC) D - 12 kPa	CONSTRUCTION TOLERANCES TO BE PROVIDED IN PROJECT CONSTRUCTION SPECIFICATIONS. CONCRETE REINFORCEMENT NOTES REINFORCEMENT STEEL SHALL BE IN ACCORDANCE WITH CANICSA-G30.18-M82 GRADE 400W.	C. ANY DARS CUT SHALL BE REPLACED WITH BARS OF THE SAME SIZE, ONE ON EACH SIDE OF THE PIPE, TWUE THE LENGTH OF UP SPICE: a. APPLY SWELL STOP WATER STOP AROUND PIPE AT CENTRE OF WALL THICKNESS. COORDINATE WITH ANOTHER CITERAL AND MECHANICAL DRAWING FOR OPENINGS, CURBS, SLEEVES, WATERPROOFING, ETC.	THK THICK TOC TOP OF CONCRETE TOW TOP OF WALL TYP TYPEQLE UNO UNLESS NOTED OTHERWISE UNO UNLESS NOTED OTHERWISE VERT VERTICAL
ROOF LIVE LOAD ROOF SNOW (15 WIND (150) SEISMIC 3. IMPORTANCE CATEGOR 4. GEOTECHNICAL CONSIL TBD PENDING GEOTECH	30) So -30 JPa -0.30 kPa -0.30 kPa -0.30 kPa -1.25 (ULS) ND 0.75 (SLS) Image: Image	2. ALL LAPS OF REINFORCING BARS FOR SPICES SHULLE BE AS FOLLOWS UN O: BAR SIZE UNICCATED UNICCATED TO JP BARS) 110M 110M 400 450 120M 550 650 220M 1000 1300 30M 1200 1600 *HORIZONTAL REINFORCEMENT WITH MORE THAN 300 mm CONCETE BLOW BARS EMAR SIZE MAR DIA di GRADE (MPa) 90' STD HOCKS 10M 10.0 430 1400 90' 10M 10.0 400 140 90' 110' 100' 10M 10.0 400 180' 1400 90' 110' 130' 100' 100' 130' 100' 120' 160' 130' 100' 130' 100' 100' 130' 100' 130' 100' 100' 130' 100' 100' 100' 130' 100' 100' 100' 100' 100' 100' 100' 100' 100' 100' 100' 100' 100' 100'		W • WIDTH W • WITH DRAWING NUMBER • BHEET TITLE ISSUED DATE 0000 COVER SHEET 2022/1-33 A 0001 GENERAL NOTES -SHEET 2022/1-32 A 0002 GENERAL NOTES - SHEET 2022/1-32 A 0001 GENERAL NOTES - SHEET 2022/1-32 A 0002 GENERAL NOTES - SHEET 2022/1-32 A 0001 FOOTRG ANG GROUND FLOOR PLAN 2022/1-32 A 0002 FOOTF FRUINS ELEVATIONS AND DETALS - SHEET 1 2022/1-32 A 0000 POOFF TRUISS ELEVATIONS AND DETALS - SHEET 1 2022/1-32 A 0000 ROOFF TRUISS ELEVATIONS AND DETALS - SHEET 2 2022/1-32 A
Liseurers Biologicade - EMile Program	REFERENCES	STANDARD 90 ⁰ HOOK STANDARD 90 ⁰ HOOK STANDARD 190 ⁰ HOOK		ROJECT DRAWING INFORMATION CONTRACT NUMBER
		ELECTRICAL SNC-Lavalin	ARCHITECTURE	









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PROPULSION POWER UPGRADES

PORT MOODY

DESIGNED BY C.FRANCIS CHECKED BY M.HAN APPROVED BY K.BRIGNALL

193028 (SNC #675448) SHEET TITLE GENERAL NOTES - SHEET 1

DRAWING NUMBER

193028 - 03 -1000- STMCZ- SL- SG- 0001-

LOAD BEARING MASONRY

3.

- FOR ADDITIONAL REQUIREMENTS SEE UNIT MASONRY SPECIFICATION AND STRUCTURAL DRAWINGS
- 2. MASONRY WORK SHALL CONFORM TO CSA \$304 AND ITS REFERENCED DOCUMENTS, INCLUDING:
- CONCRETE BLOCK TO CAN/CSA-A165.1, TYPE H/20/A, UNLESS NOTED OTHERWISE ON

- Contenti Le Backen po Uner Castro, Dari The Fraziva, Unetado Montrar To Cancesa Avita Type 's For ALL WALLS GROUT TO CANCESA-NT3 TYPE 'S FOR ALL WALLS GROUT TO CANCESA-NT3 TYPE 'S FOR ALL WALLS MASONRY' WIRE REINFORCING TO CSA ASI'A HABOATS MOT REINFORCING BARS TO CANCESA-G30. 18 400 MPa JOINT REINFORCING TO CSA ASI'A AND CSA G30.15 CONNECTORS TO CANCESA-337 FIRALTICE TO CANCESA-337

- STRUCTURAL DRAWINGS INDICATE ONLY LOAD-BEARING WALLS. DESIGN IS BASED ON ENGINEERING ANALYSIS ACCORDING TO CSA S304.
- CONCRETE MASONRY UNITS SHALL CONFORM TO CSA A165 SERIES 04. COMPRESSIVE STRENGTH FOR CMU SHALL BE 20 MPa AT 28 DAYS WITH fm = 10 MPa. MASONRY CONTRACTOR SHALL PROVIDE TO THE ENGINEER WRITTEN CONFIRMATION OF UNIT STRENGTHS PRIOR TO INSTALLATION. 4
- 5 USE ONLY TYPE 'S' MORTAR TO CSA A179-14. NO CALCIUM CHLORIDE IN ANY FORM IS PERMITTED IN THE GROUT OR MORTAR MIXES. MINIMUM STRENGTH OF MORTAR CUBE AT 28 DAYS SHALL BE 20MPa
- 6. MASONRY CONTRACTOR MUST DISCUSS ALL MASONRY CONSTRUCTION WITH THE ENGINEER PRIOR TO COMMENCING WORK
- FILL BLOCK CORES UNDER ALL CONCENTRATED LOADS WITH 20 MPa CONCRETE GROUT TO A DEPTH OF AT LEAST 400 mm MEASURED DOWN FROM THE BEARING.
- PROVIDE GALVANIZED HEAVY DUTY LADDER TYPE OR GALVANIZED TRUSS TYPE MASONRY JOINT REINFORCING 3.8 mm DIA (9 Ga) IN HORIZONTAL JOINTS EVERY SECOND COURSE (400 mm) UNLESS NOTED OTHERWISE IN DRAVININGS.
- PROVIDE LINTELS OVER ALL OPENINGS IN WALLS.
- PROVIDE 2-15M CONTINUOUS IN TOP COURSE OF WALL, UNDER BEAM OR JOIST BEARING AND FILL CELLS SOLID WITH 20 MPa CONCRETE GROUT. 10.
- 11. LAPS: WIRE REINFORCEMENT - 200 mm 450 mm 10M BARS 15M BARS 20M BARS - 650 mm - 850 mm
- 12 LINEESS NOTED OTHERWISE PROVIDE 2-15M VERTICAL BARS FULL HEIGHT AT:
- ENDS OF WALLS. EACH CORNER AND AT INTERSECTIONS. EACH SIDE OF DOORS AND OTHER OPENINGS.
- 13. ALL VERTICAL REINFORCING SHALL BE CONTINUED TO WITHIN 50 mm OF TOP OF THE WALL. PROVIDE HOOKED DOWELS FROM BOTTOM OF CONCRETE BASE TO MATCH VERTICAL MASONRY 14. WALL REINFORCING. THE CONTRACTOR AND MASONRY CONTRACTOR SHALL ENSURE DOWELS ARE CENTERED ACCURATELY WITH CELLS OVER.
- 15. ALL BARS MUST BE CONTINUOUS, PROPERLY LAPPED AT SPLICES. AT CORNERS AND INTERSECTIONS, HORIZONTAL REINFORCEMENT SHALL BE BENT AND LAPPED.
- PROVIDE CLEANOUTS FOR ALL CELLS TO BE REINFORCED, REPEAT CLEANOUTS ABOVE BOND
- FILL ALL CELLS WITH 20 MPa CONCRETE (10 mm AGGREGATE, 180 mm SLUMP) VIBRATE OR PUDDLE TO FILL CELLS COMPLETELY. 17.
- 18. FILL CELLS IN 1200 mm LIFTS.

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ISSUE/REVISION

- 19. CONTROL JOINTS SHALL BE INSTALLED AT MAXIMUM SPACING OF 6000 mm UNLESS OTHERWISE SHOWN ON DRAWINGS CONTROL JOINTS AND EXPANSION JOINTS SHALL BE CONTINUED THROUGH BOND BEAMS UNLESS 20.
- OTHERWISE SHOW OUTSIDE FACE OF CONCRETE MASONRY UNIT WALLS SHALL BE WATERPROOFED AS PER SPECIFICATIONS AND ARCHITECTURAL DRAWINGS. 21.
- NO MASONRY WORK SHALL BE PERMITED WITH TEMPERATURE BELOW 5° CELSIUS, UNLESS COLD TEMPERATURE CONSTRUCTION PRACTICE IS IMPLEMENTED. 22.
- PROVIDE 1200 mm STARTER DOWELS TO MATCH ALL VERTICAL REINFORCING, UNLESS NOTED 23.
- 24. LOAD BEARING MASONRY IS SHOWN AS ON PLAN.
- 25. DO NOT USE EXPANSION ANCHORS IN MASONRY WALL.
- INTERLOCK COURSES SHALL BE PROVIDED AT ALL WALL CORNERS AND INTERSECTIONS WITH RUNNING BOND. UNLESS NOTED OTHERWISE ON DRAWINGS. 26.

STRUCTURAL & MISCELLANEOUS STEEL NOTES

- STRUCTURAL STEEL DESIGN, FABRICATION AND ERECTION SHALL COMPLY WITH THE LATEST EDITIONS OF GOVERING CODES, SPECIFICATIONS AND STANDARDS INCLUDING: BC BUILDING CODE (FEGS) CISC CODE OF STANDARD PRACTICE FOR STRUCTURAL STEEL CAN/CSN 16-14 LIMIT STRAS DESIGN OF STRUCTURES
- STRUCTURAL STEEL INTERNALS:
 STRUCTURAL STRUC
 - THE DRAWING. d. WELDING RODS CSA-W59-18, E49xx LOW HYDROGEN TYPE.
- THE DESIGN, DETAILING, AND FABRICATION OF MEMBER CONNECTIONS SHALL BE IN ACCORDANCE WITH CURRENT PRACTICES TO CONTROL BRITTLE FRACTURE OF MEMBERS OR ASSEMBLIES.
- DESIGN ALL BOLTED STRUCTURAL CONNECTIONS AS BEARING TYPE IN ACCORDANCE WITH CIS STANDARDS USING A MINIMUM OF TWO M20 BOLTS. SMALLER SIZE BOLTS MAY BE USED IN MIN MEMBER CONNECTIONS AS APPROVED ON SHOP DRAWINGS. 5
- DESIGN BEAM CONNECTIONS IN ACCORDANCE WITH CISC HANDBOOK LIMIT STATE DESIGN, LATEST EDITION JUSING THE GREATER OF FOLLOWING BEAM SHEAR PLUS AXIAL LOADS IF SHOWN: a. BEAM REACTIONS IF SHOWN ON THE DRAWINGS b. HALF THE SHEAR CAPACITY OF THE MEMBER CONNECTED
- SPLICE CONNECTIONS SHALL DEVELOP THE FULL DESIGN STRENGTH OF THE MEMBER UNLESS OTHERWISE INDICATED ON THE DRAWINGS.
- 6. USE STANDARD BOLT HOLES IN ACCORDANCE WITH CISC STANDARDS
- CONTACT SURFACES OF CONNECTIONS WHEN ASSEMBLED IN FIELD SHALL BE FREE FROM OIL, LOOSE SCALE, AND ANY FOREIGN MATERIAL THAT WOULD PREVENT THE SOLID SEATING OF THE 7.
- TEST A MINIMUM OF 10% OF ALL BOLTS, BUT NOT LESS THAN ONE BOLT PER MEMBER PER CONNECTION WITH A HAND TORQUE WRENCH TO CHECK THE PROPER EXECUTION OF BOLTING.
- 9. WELD IN ACCORDANCE WITH THE LATEST EDITION OF CSA W59 WELDED STEEL CONSTRUCTION 10. ALL WELDING OPERATORS SHALL BE CERTIFIED IN ACCORDANCE WITH CSA W47.1 (DIVISION 1 OR
- 2.1) QUALIFICATION REQUIREMENTS. PROVIDE ALL GROUT, AS PER SPECIFICATION, BETWEEN TOP OF CONCRETE AND UNDERSIDE OF BASE PLATE. GROUT DIMENSION WHERE IS SHOWN IS NOMINAL. 11
- 12. COATING: a. PAINTING

8.

- ALL STEEL TO BE PAINTED UNLESS NOTED OTHERWISE ON THE DRAWINGS TO SPECIFICATION. PROVIDE TOUCH-UP PAINT TO DAMAGED SURFACES INCLIDING FIELD VELDING AREA. WELDING AREA. b. HOT DIP GALVANIZING WHERE NOTED ON DRAWING STEEL SHALL BE HOT DIP GALVANIZED TO CSA G164.
- REPAIR ALL DAMAGE ON GALVANIZING SURFACES WITH TWO COATS OF ZINGA-ZINC RICH PAINT, INCLUDING FIELD WELDING AREA.
- SUBAIT SHOP DRAWINGS TO THE ENGINEER FOR REVIEW PRIOR TO FARRENTING AND DRAWINGS TO SHOW ALL DEFLAS SHO MATERIAL SPECIFICATION AND THE SEGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF BRITISH COLLIMBIA FOR CONNECTION DESIGN. THE SUBAIRE RAJ LING SHOP MANINGS SHALL BERSPONSIBLE FOR ALL CONFIRMING THAT WORK HAS BEEN COMPLETED IN ACCORDANCE WITH FINAL REVIEWED SHOP DRAWINGS AND LC CODE REQUIREMENTS. 13.
- SHOP AND FIELD INSPECTION AND TESTING OF STEEL FABRICATION, ERECTION AND INSTALLATION TO BE COMPLETED BY AN INDEPENDENT TESTING AND INSPECTION AGENCY RETAINED BY THE OWNER. TESTING AGENCY SHALL BE CERTIFIED TO CASH/0178. 14

- STEEL DECK
 - DESIGN, FABRICATE AND INSTALL STEEL DECK TO CSA S136 AND CANADIAN SHEET STEEL 1. BUILDING INSTITUTE STANDARDS CSSBI 10M. 2.
 - STEEL DECK, UNLESS NOTED OTHERWISE ON DRAWINGS, SHALL BE PREFORMED 38mm DEEP MINIMUM 22GA THICKNESS CONFORMING TO ASTM A650XASTM A653M. STEEL DECK SHALL BE ZUN-COATED CONFORMING TO ASTM A650X63MZ 1757 FOR INTERIOR EXPOSURE 2275 FOR EXTERIOR EXPOSURE AND UNHEATED AREAS. DECK FLUTES SPACED AT 152mm MAXIMUM ON PLATERIOR EXPOSURE AND UNHEATED AREAS. PROVIDE ROOF DECK TYPE P3606 COMPOSITE BY CANAM OR APPROVED EQUIVALENT. 3
 - FASTEN DECK TO SUPPORTING MEMBER AS FOLLOW UNLESS NOTED OTHERWISE ON ROOF PLAN 4
 - a. STRUCTURAL STEEL: HILTI X-ENP-19, @ 300mm c/c MINIMUM FASTEN DECK SIDE LAPS WITH #10 SCREW @ 300mm c/c MINIMUM, UNLESS NOTED OTHERWISE ON ROOF PLAN DRAWING. 5
 - DECKING INSTALLERS MUST PROVIDE A VALID TRAINED OPERATOR HILTI DECK FASTENING SYSTEM CARD. A DIRECT REPRESENTATIVE OF HILTI TO BE ON-SITE DURING INITIAL INSTALLATION TO PROVIDE TRAINING TO INSTALLERS IN PROPER SELECTION AND INSTALLATION PROCEDURES.
 - PLACE DECK SHEET IN A MINIMUM 3 SPAN LENGTH EXCEPT WHERE OTHERWISE APPROVED.
 - UNLESS NOTED OTHERWISE, REINFORCE ALL ROOF DECK OPENINGS GREATER THAN 150mm AND LESS THAN 400 mm WITH 138X38X6 4 EXTENDED AND FASTENED TO MINIMUM TWO FLUTES ON EACH SIDE OF OPENING. FRAME OPENINGS EQUAL TO 400mm OR LARGER WITH MINIMUM TIXF07K9K6 4 EXTENDED TO MAIN STRUCTURAL MEMBERS OR AS SHOWN ON THE DRAVINGS. 8.
 - SUBMIT DECK SHOP DRAWINGS TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL SHOW ALL DETAILS AND MATERIAL SPECIFICATIONS. 9.

FIELD REVIEWS

- THE CONTRACTOR'S SUPERINTENDENT IS TO PRE-INSPECT THE WORK TO CONFIRM WORK IS COMPLETED AS PER DOCUMENTS AND PROVIDE THE ENGINEER'S REPRESENTATIVE WITH A MINIMUM OF 72 HOURS NOTICE IN ACCORDANCE WITH BOCE 2018 AND MUNICIPAL BYLAWS FOR MINIMUM OF 72 HOURS NULLE IN ROUGHING TO FORMING. a. GENERAL SITE CONDITIONS PRIOR TO FORMING. b. REINFORCING STEEL AND POUR CONDITIONS PRIOR TO EACH CONCRETE POUR. c. STRUCTURAL STEEL FRAMING PRIOR TO BEING COVERED.
- THE GEOTECHNICAL ENGINEER SHALL REVIEW FINAL PLANS AND INSPECT THE SITE PREPARATION WORK TO COMFIRM THAT THE SOLL CONDITIONS ARE CONSISTENT WITH DESIGN ASSUMPTIONS AND DESIGN RECOMMENDATIONS. INSPECTION TO INCLUE THE FOLLIDUE THE FOLLIDUE THE a. BASE INATERIALS FOR CONFIRMATION OF ASSUMED SOLI BEARING. b. TESTING: FOR COMPACTION OF ANY STRUCTURAL FLUX REQUIRED UNDER FOOTINGS AND
 - GRADE SLAB. c. BACKFILL AROUND / UNDER GROUND SERVICES.
- THE GEOTECHNICAL ENGINEER TO BE NOTIFIED MINIMUM 2-WEEKS PRIOR TO ANY FOOTING POURS FOR INSPECTION AS NOTED ABOVE. 3.
- REINSPECTION REQUIRED BY THE ENGINEER DUE TO INCOMPLETE WORK AND / OR DEFICIENCIES FROM PREVIOUS INSPECTIONS. SHALL BE AT THE EXPENSE OF THE CONTRACTOR.
- ALL WORK SHALL BE MADE ACCESSIBLE FOR INSPECTION. FAILURE TO GIVE REQUIRED NOTIFICATION AND ACCESSIBILITY MAY RESULT IN THE ENGINEER REQUESTING THE REMOVAL AND REPLACEMENT OF THE WORK AT THE CONTRACTORS EXPENSE.
- REVIEW OF THE WORK, OR ANY PORTION THEREOF, BY THE ENGINEER SHALL NOT IN ANY WAY RELIEVE THE CONTRACTOR OF HIS RESPONSIBILITY AND OBLIGATION TO COMPLY WITH THE CONTRACT DRAWINGS AND SPECIFICATIONS.

REFERENCES

A 2022-11-23 JV 60% SUBMISSION
 PA
 2022-08-05
 CF
 30% SUBMISSION

 I/R
 DATE
 BY
 DESCRIP
 KB DESCRIPTION APPR





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ARCHITECTURE

ERANCI Arch

KEY PLAN

EMUP PROPULSION POWER

PROJECT

UPGRADES

PORT MOODY

DRAWING INFORMATION CONTRACT NUMBER

DRAWN BY A.CHEUNG PROJECT NUMBER DESIGNED BY C.FRANCIS CHECKED BY M.HAN APPROVED BY K BRIGNALL

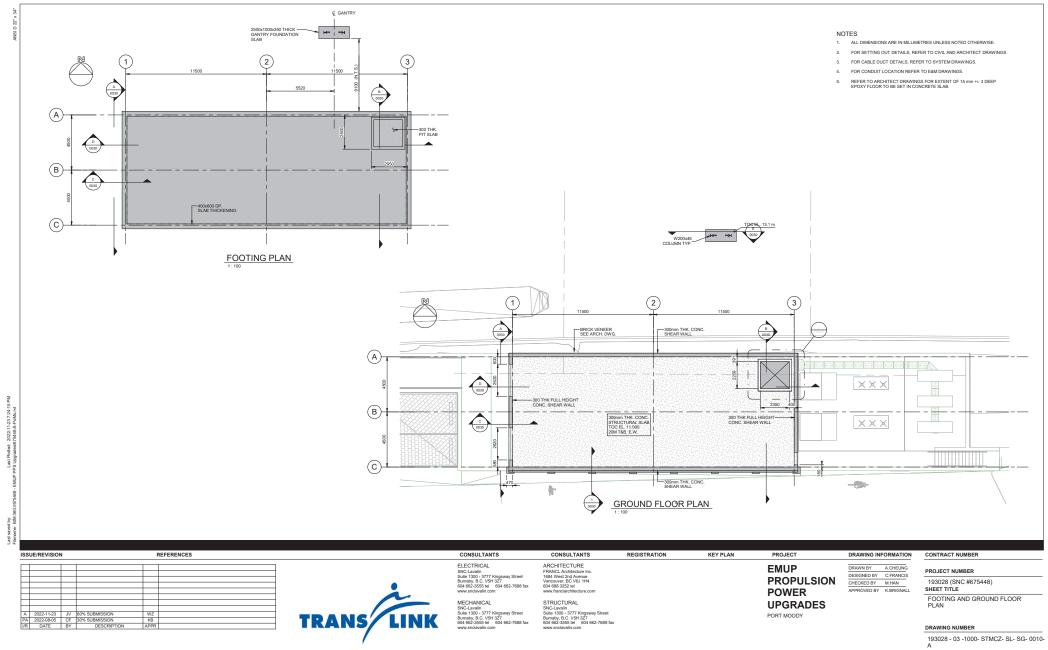
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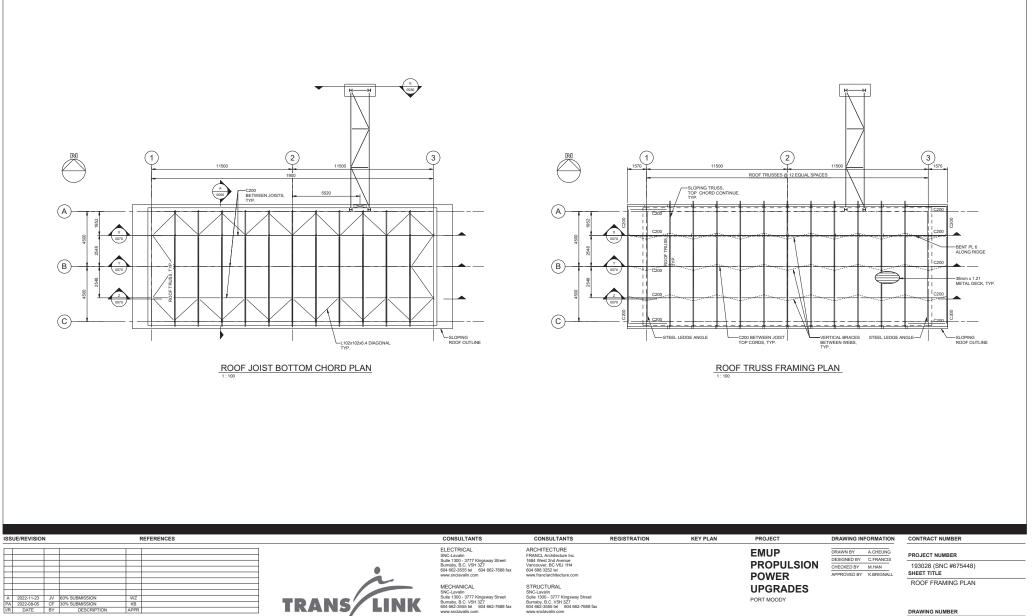
GENERAL NOTES - SHEET 2

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REGISTRATION

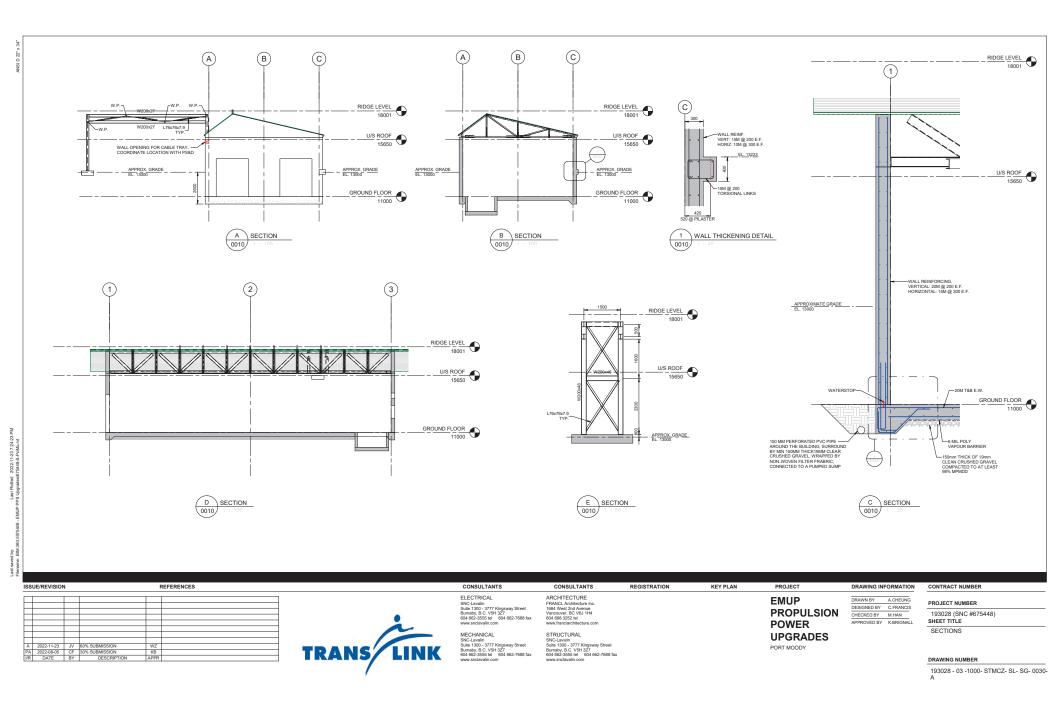


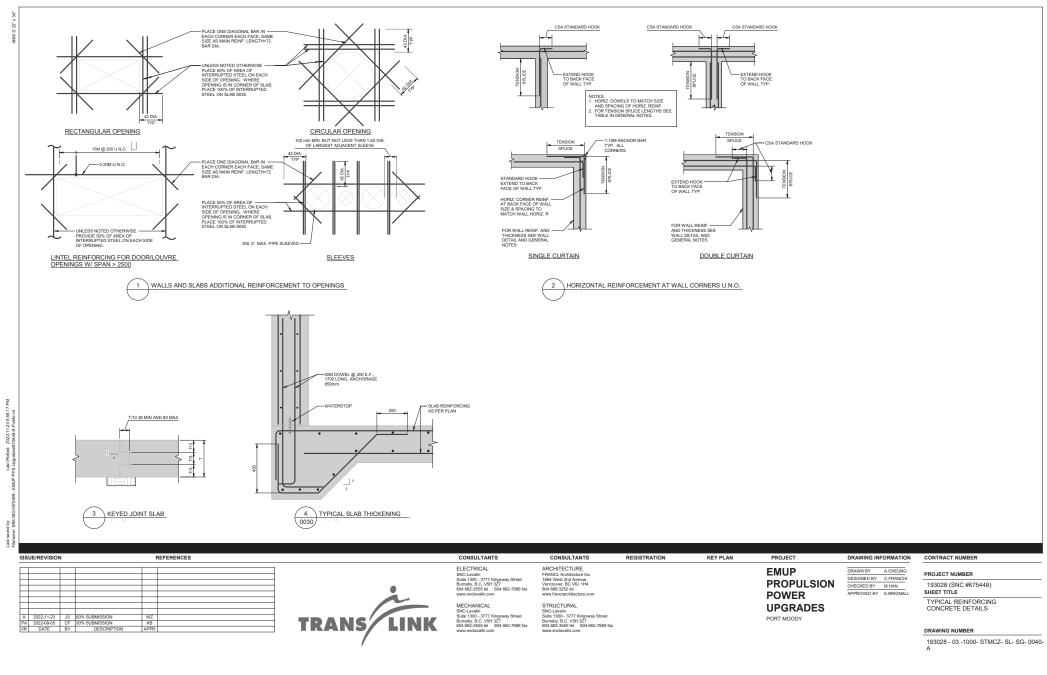


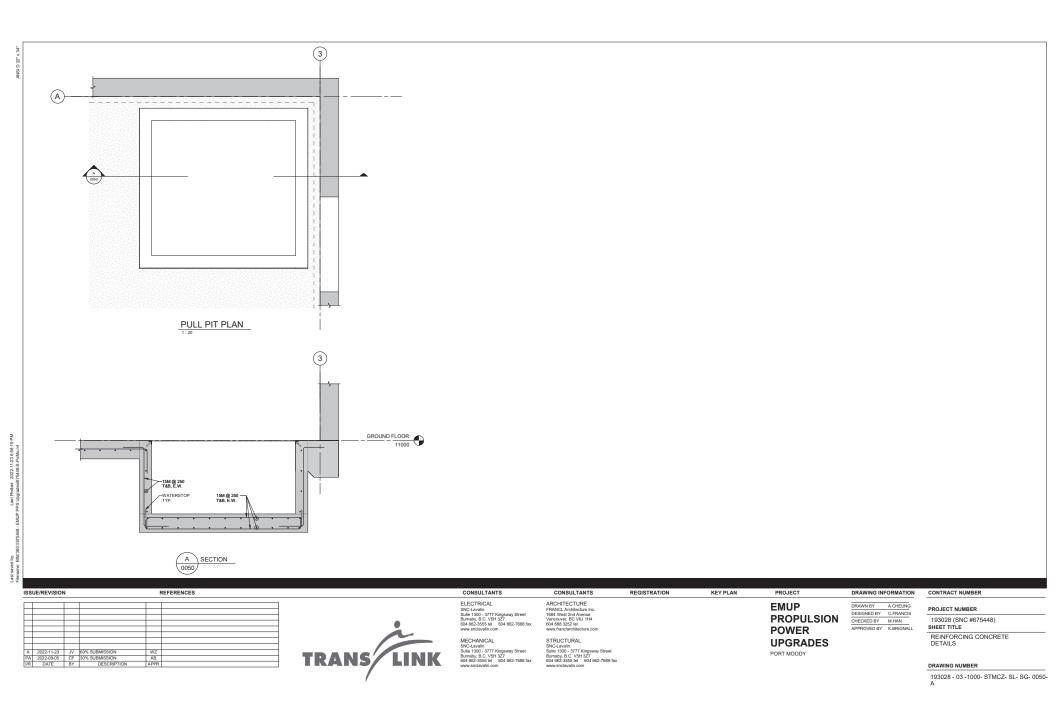
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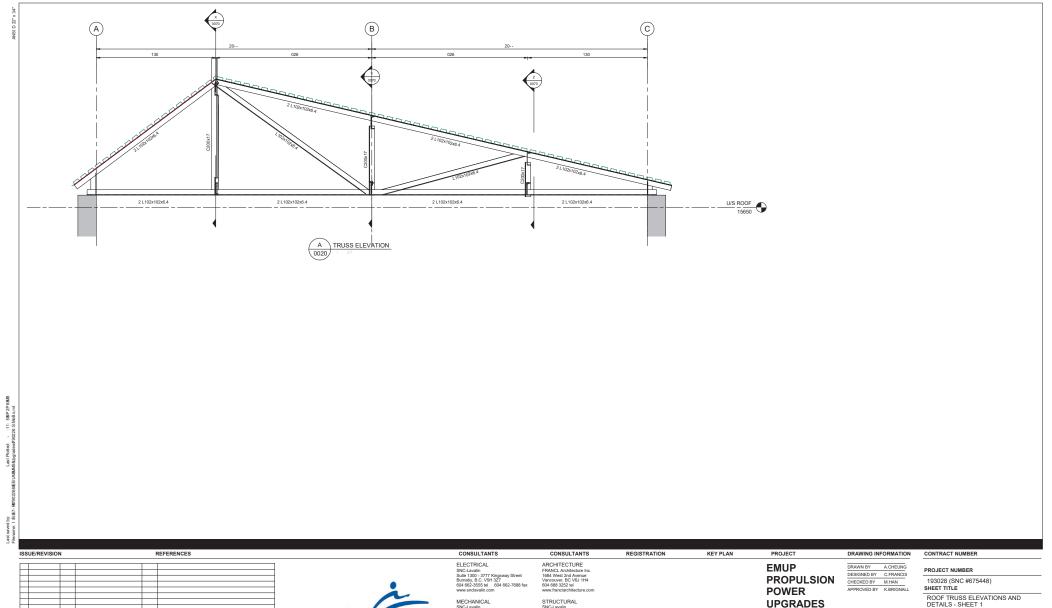
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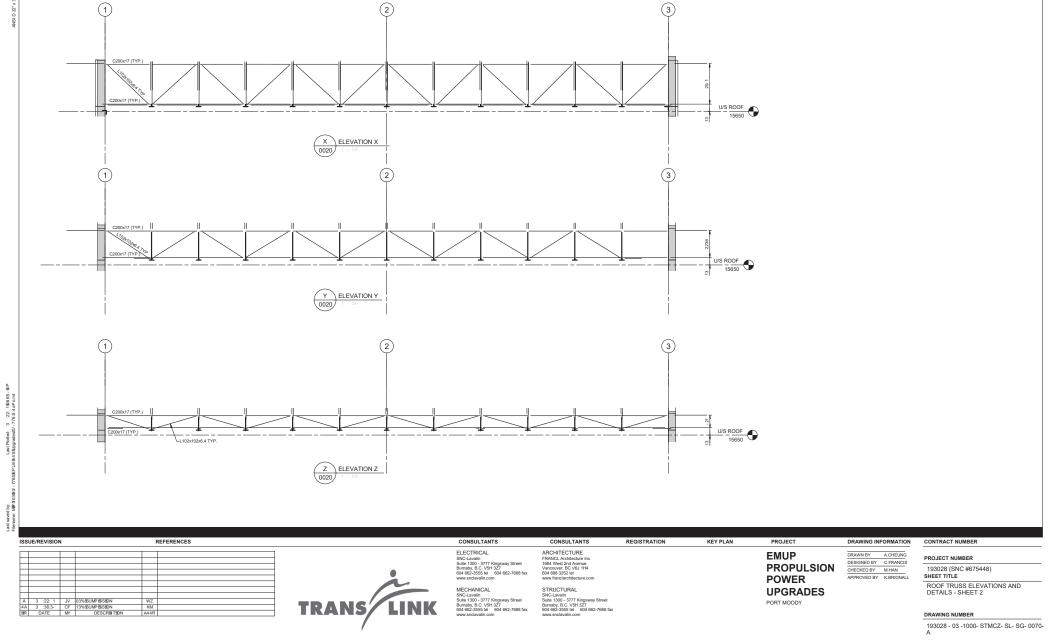


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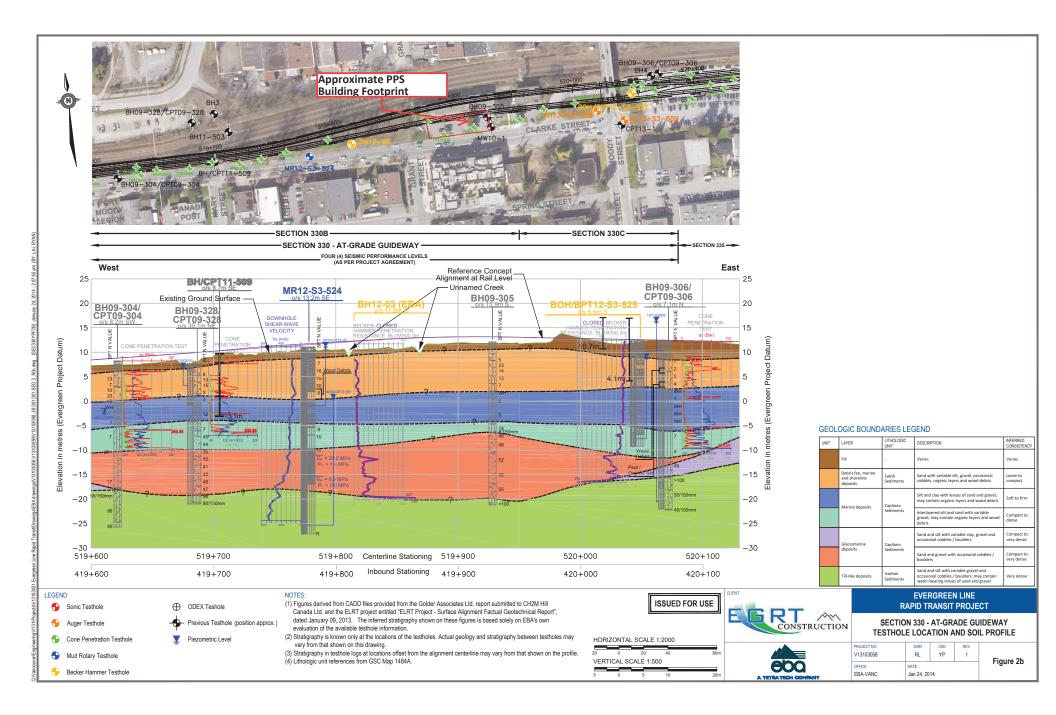
UPGRADES

PORT MOODY



Appendix II

Borehole/CPT Logs



115
1

RECORD OF BOREHOLE: BH09-305

DYNAMIC PENETRATION RESISTANCE, BLOWS/0.3m

SAMPLES

LOCATION: 2708 & 2710 Clarke Street, Port Moody N: 5458164.77 E: 510899.29 Survey Provided by: Mason Surveys Inc.

SOIL PROFILE

SAMPLER HAMMER, 64kg; DROP, 762mm

DRILLING DATE: July 2 to 3, 2009

DRILLING CONTRACTOR: Foundex Explorations Ltd.

HYDRAULIC CONDUCTIVITY, k, cm/s

SHEET 1 OF 4 DATUM: Local

STANDPIPE

INSTALLATION AND

BORING METHOD ADDITIONAL LAB. TESTING DEPTH SCALI METRES WATER LEVELS 10⁻⁶ 10-5 10-4 STRATA PLOT BLOWS/0.3m 20 40 60 80 10-3 NUMBER ELEV. TYPE SHEAR STRENGTH nat V. + Q - ● rem V. ⊕ U - O WATER CONTENT PERCENT DESCRIPTION NP - Non-Plasti DEPTH Cu, kPa Wp ⊢ -OW (m) 20 40 60 80 20 40 60 80 Ground Surface DRAIN ROCK. [FILL] 12.12 0 0.05 Inferred, moist, dark brown, silty 0 GS 0 SAND, some gravel, contains cobbles and organics/topsoil. [FILL] 1 10.90 1.22 2 GS 0 0 Inferred, moist, brown, silty SAND, some gravel, contains cobbles, rootlets and topsoil. 2 9.68 2 44 2008/141105-141-0115 [EVERGREEN PROJECT)GINT08-1411-0115_FACTUAL REPORT.GPJ OutputForm8DREHOLE (ALTO) Tomplate BC REGION TEMPLATE BETA 1.GDT Lbrary/BC REGION LBRARY GLB KCapes 4/10/10 3 Inferred. loose, moist to wet, brown to grey SAND, some silt and gravel, contains cobbles. 51 DO 0 3 5 8.31 3.81 . 0 4 51 DO 23 0 М 4 - at 4.88m, cobble. 5 at 5.18m, cobble. Compact, moist to wet, brown to grey, gravelly SAND, some silt, contains silt layers (25 mm thick) and cobbles. 51 DO 14 5 HT-1000 Truck Mounted Rotary Drill Rig 6 Mud Rotary 5.11 7 51 DO 12 0 М 6 Compact to loose, wet, grey, silty SAND to sandy SILT, trace gravel. 8 3.58 - at 8.53 to 9.45m, trace black 8.53 organics. 51 DO 7 0 7 9 Loose, wet, grey, silty SAND to sandy SILT, trace clay. 10 CONTINUED NEXT PAGE ACTIVE/YEAR : LOGGED UW 74 DEPTH SCALE Golder Associates CHECKED: W.L. 1:50 Ciala-7

Project Co-ordinates to UTM NAD 83 Zone 10 Conversion: Project North * 0.999596150; Project East * 0.999596150

		T No.: 08-1411-0115	R	ECO	DR	RD	0	F BORE	EHO	LE:	BH	09-3	05					EET 2 OF 4
N: 5	4581	0N: 2708 & 2710 Clarke Street, Port Moody 64.77 E: 510899.29						DRILLIN					oration	. 1 4 4			DA	TUM: Local
		ided by: Mason Surveys Inc.						DRILLIN	3 CONI	RACIO	C. Found	ex ⊨xpl	orations	s līd.				
		R HAMMER, 64kg; DROP, 762mm SOIL PROFILE			SA	MPL	ES	DYNAMIC PE RESISTANCE	NETRAT	ION		HYDR	AULIC (TIVITY,	т	ГТ	STANDPIPE
MEIRES	BORING METHOD		5				_	RESISTANCE 20	40		, \	1	k, cm/		10-4	10'3	ADDITIONAL LAB. TESTING	INSTALLATION AND WATER LEVELS
Р Ц	IG ME	DESCRIPTION	STRATA PLOT	ELEV.	NUMBER	TYPE	BLOWS/0.3m	SHEAR STRE									DITIO	
ž	ORIN	DESCRIPTION	RAT,	DEPTH (m)	NUN	∣≿	LOW:	Cu, kPa		rem V. 🕀	U-Ō	W	р ——	O ^W	/ NP - N	WI Ion-Plastic	ADC LAB.	
_	В		S	(,			8	20	40	<u>60 8</u>	0	2	20	40		80		
0				16:88		-												
					8	51 DO	wн						0—				мн	
					ľ	DO									1.			
1																		
		Soft to firm, wet, grey SILTY CLAY to CLAY, contains sandy silt and silty																
		sand layers (25 to 50mm).			9A	76 TO												
2						1												
					9B	51 DO	6									9		
5				-0.99														
			Ш	13.11														
			ŊИ		10	76 TO						1	$+ \circ$				H, C, LV	
			КIJ															
ı			КU					Ð	+									
	gi		ИJ															
	Drill R		ИI															
	Rotary		Ш															
5	k Mounted Mud Rotary	Firm, wet, grey CLAYEY SILT, some fine sand, contains soft, grey, silty clay and silty sand layers.	[]//		11	51 DO	5							S				
	HT-1000 Truck Mounted Rotary Drill Rig Mud Rotary	and sity sand layers.	ſИ															
	000 Tri		ſИ															
	HT-10		КIJ															
3			KU															
			ИJ															
			ИI															
			IЩ	-4.65														
,				16.76														
		Loose to compact, wet, grey, silty																
		SAND to sandy SILT, trace gravel.			\vdash	1												
3					12	51 DO	14											
l																		
l				-6.32 18.44		51 DO	8/15	Omm					\$					
			0° ه		13B	51 DO	46					0						
9			0 0		\vdash	1												
		Compact to dense, wet, grey SAND and GRAVEL, trace silt, contains																
		cobbles. - at 19.50m, cobble.	0 0															
			o d															
,	_L		io Lini		┣-	₋	_			+	L			∔	-	+	-	
		CONTINUED NEXT PAGE																
ישר	отце	SCALE														100	GED: L	.W.
	50								plot	er ates							CKED: V	

PROJECT No.: 08-1411-0115

Project Co-ordinates to UTM NAD 83 Zone 10 Conversion: Project North * 0.999596150; Project East * 0.999596150

Attachment 3

SHEET 2 OF 4

	PLER HAMMER, 64kg; DROP, 762mm						DYNAMIC PEN			`		CONDUCTIVITY,	_		STANDPIPE
THOL	SOIL PROFILE	5		SA	MPL		RESISTANCE,	BLOWS	/0.3m	ل ا	k, cm	10 ⁻⁵ 10 ⁻⁴ 10 ⁻³	I	LING	INSTALLATION ANI WATER LEVELS
BORING METHOD		STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	BLOWS/0.3m	SHEAR STREN Cu, kPa	IGTH	atV. + emV.⊕	Q - ● U - O		CONTENT PERCENT OW W NP - Non-1 40 60 80		LAB. TESTING	
•		0.0												_	
1	- at 20.72m, cobble. Compact to dense, wet, grey SAND and GRAVEL, trace silt, contains cobbles. (continued)			14	51 DO	48					0			м	
2	- at 22.25m, cobble.														
8 2 2 9 9 9 1TT-1000 Truck Montel Ratary Drill Rig 7 2 2 1000 Truck Montel Ratary Drill Rig 7 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	- at 25.60 to 26.21m, sand and gravel layer. Dense, moist to wet, grey SAND, trace gravel and silt.		22.86		51 DO	52					0			м	
,	Dense, moist to wet, grey SAND and GRAVEL, some silt, contains cobbles.	<u> </u>	-16.84 28.96	17	51	55									

Project Co-ordinates to UTM NAD 83 Zone 10 Conversion: Project North * 0.999596150; Project East * 0.999596150

Attachment 3

SHEET 3 OF 4

PROJECT No.: 08-1411-0115

RECORD OF BOREHOLE: BH09-305

		CT No.: 08-1411-0115	F	RECO	DR	RD	0	F BO	RE	HOL	E:	BH	09-3	05					IEET 4 OF 4
N:	5458	ION: 2708 & 2710 Clarke Street, Port Moody 164.77 E: 510899.29										o 3, 2009			1.4.1			DA	TUM: Local
								DRI	LLING	CONTR	ACTOR	R: Found	ex Explo	orations	Lta.				
		ER HAMMER, 64kg; DROP, 762mm SOIL PROFILE			SA	MPL	ES	DYNAM RESIST	IC PEN	ETRATIO	DN .	$\overline{\mathbf{N}}$	HYDR	AULIC Ç	ONDUC	TIVITY,	т	_	STANDPIPE
DEPTH SCALE METRES	BORING METHOD		ОТ				E	20 RESIST.				, \					0-3	ADDITIONAL LAB. TESTING	INSTALLATION AND WATER LEVELS
TH S	M DV	DESCRIPTION	STRATA PLOT	ELEV.	NUMBER	TYPE	BLOWS/0.3m	SHEAR	STREN	GTH r	atV. +	Q - ●	w	ATER C			Î NT	DITIC TES	
	BORII		TRAT	DEPTH (m)	Ñ	F	BLOW	Cu, kPa			em V. 🕀		Wp		O ^W	NP - No	WI on-Plastic	AD	
			S				-	20	4	06	8 0	0	2	0 4	10 6	50 8	30		
- 30		- at 30.18m, cobble.			17	DO 51 DO	55						0						
						100													
			0																
	Bi																		
- 31	Drill R	Dense, moist to wet, grey SAND and GRAVEL, some silt, contains cobbles.	0																
	Rotary	(continued)																	
	Unted		<u>م</u>																
	Ick Mo			-19.89															
- 32	HT-1000 Truck Mounted Rotary Drill Rig	- at 32.00m, cobble.		32.00															
	HT-10																		
		Very dense, moist, grey, sandy SILT, some gravel, contains cobbles.		1															
		some gravel, contains cobbles.	Ø																
- 33				-	18	51 DO	>100						0						
		End of Doubledo	3 11.	-21.18 33.30															Completed borehole
		End of Borehole.																	grouted to surface.
• 34																			
- 35																			
· 36																			
00																			
37																			
38																			
39																			
- 40																			
DE	PTH	SCALE						Ð		11							LOG	GED:	W.
	50							T	Ass		r						CHEC	KED:	W.L.

PROJECT No.: 08-1411-0115

Project Co-ordinates to UTM NAD 83 Zone 10 Conversion: Project North * 0.999596150; Project East * 0.999596150

Attachment 3

		CONSTRUCTION							rke St and Moody S							
		CONSTRUCTION			: 520+			oiui	OFFSET: 7 m R	, , , ,		<u></u>		TON: 12.3		
LIE	NT: S	SGJV	NOR	RTHIN	G: 54	5817	0.69		EASTING: 51101	9.07			ZONE: '	10		
(m)	METHOD			LEGEND		TE	RATION STS		UNCE CHAMBER PRESSURE (kPa) 40 80 120 160	DIS	ARTIC SIZE TRIBU	TION	100 FIE REMOULD	LD VANE (kPa)) ▲ 00 :	andpipe
Depth (m)	DRILLING METHOD	DESCRIPTION		GRAPHICAL LEGEND	SAMPLE NUMBER	BPT N _b (Blows)	Bounce Chamber Pressure (kPa)	● B O BE	BECKER PENETRATION TEST (N _b) ECKER OPEN HOLE TEST (N (Blows/305mm) 40 80 120 160	FINES (%)	SAND (%)	GRAVEL (%)	10 PLASTIC 20	M.C. LIC	0 0 2UID 1 1 80	25 mm Standpipe
1		SAND and GRAVEL, trace silt, occasional cobbles, contains rootlets and wood fragments, damp, loose, dark brown; fine to coarse sand; fine and coarse sub-rounded gravel (FILL)			1	3 8 6	83 83 83			7	55	38	•			12
I	-	SAND, some gravel, trace silt, damp, loose, brown; medium sand; coarse subrounded to subangular gravel (FILL)	 r			6 11 12	83 83 83									11
2		SAND, silty, some gravel, wet, compact, brown; fine			2	11	83 90			5	78	17				
3 <u>℃</u>		sand, fine subangular gravel (POSSIBLE FILL)			3	9 12	90 90			22	61	17				10 <u> </u>
1 01-Mar-13	-	SAND, some gravel, trace silt, wet, compact, varies gr to dark brown; fine to medium sand; fine and coarse subangular to subrounded gravel	ey :		4	11 13	103 103	•		6	75	19	•			i co-Mar-
4	osed Ended)	Coalse Subangular to Subrounded graver			5	17 17 15	103 103 103									8
5	pen and Cl	 pull-out test at 4.6 m (casing friction - closed ended) contains frequent pieces of fibrous dark brown wood 				15 14	103 103									
	ker Hammer (Open and Closed Ended)	debris up to 250 mm long			7	12 10	103 103			7	74	19	•			7
6	Bec				8	6	90 90			6	81	13	•			6
7		SAND and SILT, trace gravel, trace clay, wet, compact browny grey; fine to medium sand; fine and coar subangular to subrounded gravel; contains occasional to frequent pieces of fibrous wood debris	t, T		9	9 12	90 90	•								
8		SILT, sandy, trace clay, trace gravel, moist, firm, grey,			10	878	90 90 90			49	49	2				
		low plasticity; fine sand, contains pieces of dark brown decomposing wood debris			11	8 8 15	90 90 90 110	•		70	29	1	₽			4
9		SAND, some silt, trace to some clay, wet, compact, gri fine sand y - pull-out test at 9.5 m (casing friction - closed ended)	ey;		12	13 14 22	110 110 117									3
			- / - ייסח					↓/÷ ₽・⊑	oundex Exploration					LETION D		20.1
	1	1066 West Hastings Street Vancouver	-						80 Becker Hammer					TED: 11/1		23.1
	Ē	BC V6E 3X2 CANADA p. 604.685.0275			BY: F		/-	v-1						LETED: 1		12
AT	ETRA	TECH COMPANY f. 604.684.6241			ED BY								Page			-

(i) Had between the second s	12.39 m N. (kPa) ▲ 00 400 8
CLIENT: SGJV NORTHING: 5458170.69 EASTING: 511019.07 ZONE: 10 (u) 0H 301 0H 301 20043 0H 301 200453 0H 301 200453 </td <td>(kPa) ▲ 00 400 E (kPa) PEAK PEAK 00 400 LIQUID LIQUID</td>	(kPa) ▲ 00 400 E (kPa) PEAK PEAK 00 400 LIQUID LIQUID
Image: Constraint of the second se	00 400 E (kPa) PEAK 30 40 LIQUID
(m) Hug Hug DESCRIPTION Hug PENETRATION TESTS	00 400 E (kPa) PEAK 30 40 LIQUID
40 40 40 40 40 40 40 40 40 40 40 40 40 4	00 00 00 00 00 00 00 00 00 00 00 00 00
- 20 40	60 80
plasticity; fine sand	
$\begin{bmatrix} 12 & 90 & \bullet & \vdots & \vdots & \vdots & \vdots & \vdots & \vdots \\ -11 & 13 & 11 & 90 & \bullet & \vdots & \vdots & \vdots & \vdots & \vdots & \\ 13 & 11 & 90 & \bullet & \vdots & \vdots & \vdots & \vdots & 96 & 4 & 0 & \vdots & \\ -11 & 13 & 11 & 90 & \bullet & \vdots & 1 & \vdots & \vdots & 96 & 4 & 0 & \\ -11 & 12 & 13 & 11 & 90 & \bullet & \vdots & 1 & \vdots & 1$	
- 12 - contains occasional lenses of fine SAND below 11.9 m	
13 SILT, some clay, trace sand, wet, soft to firm, grey, low plasticity; fine sand	
	<u></u>
14 Image: Second state s	
 firm, grey, low plasticity; fine sand, fine subangular gravel - pull-out test at 14.3 m (casing friction - closed ended) - 16 - 10 -	
SAND, silty, trace clay, wet, compact, grey; fine sand	
- 18 22 110 120 110 11	
SILT, sandy, some clay, moist to wet, firm, grey; fine	
- pull-out test at 19.2 m (casing friction - closed ended)	
Itilize 21 138 Itilize I	
1066 West Hastings Street Vancouver BC V65 3/2	
BC V6E 3X2 CANADA p. 604.685.0275 LOGGED BY: RB COMPLETE	ED: 11/15/2012
18 19 10 19 17 110 19 17 110 19 SILT, sandy, some clay, moist to wet, firm, grey; fine sand 19 17 110 15 110 15 110 15 110 15 110 15 110 15 110 15 110 15 110 15 100 15 100 15 100 15 100 15 100 <td></td>	

Attach	ment	3
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			BC)F	RE	H(C	E	LC)():	B	0	Η	/E	3F	די	-1	2	-5	<u>}3</u>	-52	25
	5	CONSTRUCTION	LOCAT	ION	I: NE	corn	er of	Clarke	e St a	nd M	oody	St,	Por	rt Mo	oody	/PR	OJE	ЕСТ	NC): V1	1310	3058-	01
		CONSTRUCTION	STATIO	DN:	520+	049		(OFFS	ET: 7	7 m F	2				EL	EVA	ATIC)N:	12.3	39 m		
CLIE	NT:	SGJV	NORTH	HINC	G: 54	5817	0.69	I	EAST	ING:	5110	019.0	07			ZO	NE:						
	8		END	ш	н	PENET	RATION	BOUN	CE CHAN (I	MBER P kPa)	RESSU	IRE		RTIC	LE			OCKE 200	T PEN 30		a) 🔺 400	e	
(E	ITH I		LEG	SAMPLE TYPE	UMB		STS	40	80	120	160) [DIST	RIBU		F	REMOU	LDED	VANE) PEAK	lidpu	Elevation (m)
Depth (m)	NGN	DESCRIPTION	ICAL	PLE	LE N	slows)	amber (kPa)	• BEC	KER PE	NETRA	TION TE	EST	(%)	(%)	r (%)		10	20	30)	♦ 40	ר Sta	atio
ď	DRILLING METHOD		GRAPHICAL LEGEND	SAN	SAMPLE NUMBER	BPT N _b (Blows)	Bounce Chamber Pressure (kPa)	O BECI	KER OPE (Blows	ÈN HOL s/305mn	E TEST n)	(N _b)	FINES (%)	SAND (%)	GRAVEL (%)	PL/	ASTIC	Ν	M.C.	LI	QUID	25 mm Standpipe	Elev
			5		0,	₩	8	40) <u>80</u> : :	120	160)	ш	S	Ъ		20	40	6	0	80	~ !-+ %	· _
F				:		24	138		: :		Ì.												
F		- trace clay below 20.4 m		÷		21	138											-					8 -
E						27	138															₩.	
21				÷	21	24	124			.;;/. :::)									•••••				
Ę						20	117		: :	ŀ													9 —
Ē		End of Becker open hole drilling at 21.6 m depth (targe	t :[].·	<u>.</u>		23	117											-				<u></u>	<u> </u>
- 22		depth reached). - Standpipe piezometer installed in borehole as indicat				23				ļ													-
						24	117			Ň													
E_		- pull-out test at 22.3 m (casing friction - closed ended)				38	131			: :\ : :\								-		:			-10 -
Ē						34	138											÷		:			
23						29	159				Ϊ <u>λ</u>												
F						37	159	Ī			i ii												
F						91	159		$\overline{\ }$		E												-11
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24	inded					91	159			D i i	e e e fer E e fe									•••••••			
Ę	ecker Hammer (Open and Closed Ended					75	159				1												-12
Ē	d Clo					67	152		Þ														
	en an					60	152		•														
- 20	0 0					59	152		•		7												
E_	mmei	- pull-out test at 25.3 m (casing friction - closed ended)				63	145											-		:			-13 -
Ē	er Ha					52	145																
- 26	Beck					40	145			•									•••••				
F						37	145											÷		:			-14
F						53	145											÷		:			-14 -
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27						83				• • • • • • • •	1					· · · ; ·			·;; : :	••••••	· · · · · · · · · · · · · · · · · · ·		
E						55	152	/			/							-		:			-15
F						29	138				É												-
28						31	138				1 												
						65	145		•		1							-		:			
<u>-</u>		 pull-out test No. 7 at 28.4 m (casing friction - closed ended) 				66	145											÷		:			-16
F		endou)				166	145			/	\checkmark												
- 29						204	145			•		204	,										-
1		End of Testhole at 29.1 m depth (refusal). - Standpipe piezometer installed in testhole as indicate	d.									: [-		:			1
		 Estimates of the soil consistency were determined fro BPT blowcounts. drill rig performance, and visual 	m						: :	: :								÷		÷			1
		classification of recovered samples. These																÷		:			1
	<u> </u>	1066 West Hastings Street	DRILL	NG	CON	TRA	сто	R: Fol	undex	Expl	lorati	ons				 C	ON	IPLE	ETI(I NC	DEP	TH: 29	9.1m
	4	Vancouver BC V6E 3X2	DRILL													5	STA	RTE	D: ´	11/1	3/20	12	
	E	CANADA p. 604.685.0275	LOGG	ED	BY: R	B										C	ON	IPLE	ETE	D: 1	1/15	/2012)
AT	ETRA	TECH COMPANY f. 604.684.6241	REVIE	WF	DBY	BH	-			-	-					F	aae	30	of 4				

E	-	E P		BC)F	RE	H(C	E	EL	()(3:	В	SO)H	/E	BF	ךכ	[1	2	-S	3-	-52	25
		CO	NSTRUCTION	LOCA				er of	Cla	_					i, Po	ort Mo	pody							058-0)1
			~	STATI						-			7 m									12.3	9 m		
CLIE		SGJV		NORT	-	G: 54	58170).69		_			: 511 PRESS					ZC)NE			N. (kPa			
Depth (m)	DRILLING METHOD	DE	SCRIPTION	N I FGFN	SAMPI F TYPF	SAMPLE NUMBER	TE			40	(80	kPa) 12	0 16	60	DIS	ARTIC SIZE TRIBU	TION		100	20 FIELD) 3() VANE	00 4 E (kPa) PE		25 mm Standpipe	Elevation (m)
Dept	DRILLING	DL		GRAPHICAL LEGEND	SAMPI	SAMPLE	BPT N _b (Blows)	Bounce Chamber Pressure (kPa)		ECKE	(R OPE (Blows	(N _b) EN HOI 5/305m	LE TES m)	T (N _b)	FINES (%)	SAND (%)	GRAVEL (%)	PL	10 ASTIC		M.C.	LIC	0 QUID 1 30	25 mm S	Elevati
	DR	judgem - Moisture content v be representa disturbance. - Fines content valu in-situ condition - Reported blowcou - Becker open blow	as are based on engineering ent and are subjective. alues for cohesionless soils may n tive of in-situ conditions due to drill as may not be representative of ons due to drilling disturbance. Its are uncorrected field values. count values should not be relied n and are provided for relative nly.	ot		SA		Bound		40	Blows 80					SAI	GRA		20	4			1 30	25	
			1066 West Hastings Street	DRILL	INC	G CON	ITRA	сто	R: F	oun	dex	Exp	lorat	ions	S			(CON	/IPL	ETI	ON E	EPT	H: 29	.1m
			Vancouver BC V6E 3X2	DRILL	INC	RIG	TYPE	E: HA	V-1	80 I	Becl	ker H	lamr	ner				5	STA	RT	ED:	11/1	3/201	2	
	e	200	CANADA p. 604.685.0275	LOGG	ED	BY: F	RB											(CON	/IPL	ETE	D: 1	1/15/	2012	
AT	ETRA	TECH COMPANY	f. 604.684.6241	REVIE														_			of 4				

Attachr	nent 3
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		e or	BORE	EH(C	E		L(0	G	: E	βH	1	2-	03	3				
	9	CONSTRUCTION	LOCATION: CI														TNC): V13	103058-	·01
		CONSTRUCTIO	STATION: 519				_				mR							11.7 n		
CLIE	NT:	SNC-Lavalin	NORTHING: 54	458152	2		E	AS	TIN	G: 5	1078	7			ZON	NE: 10	D			
	OD			PENET	RATION	B	OUNC	E CH	AMBE (kPa)	R PRE	SSURE			LE		POCK 00 20	KET PEN 00 30	N. (kPa) 400		
<u>ا</u>	DRILLING METHOD			TES	STS		40			120	 160	DIS	SIZE TRIBU				D VANE			Elevation (m)
Depth (m)	NG	DESCRIPTIO	Ν	lows)	amber (kPa)	•	BECI	KER P	ENET	RATIC	N TEST	(%)	(%)	GRAVEL (%)		0 2			Backfill	atior
D	RILLI			BPT N _b (Blows)	Bounce Chamber Pressure (kPa)	0	BECK	ER OI (Blo	PÈN H ws/305	IOLE T 5mm)	EST (Nt	FINES (%)	SAND (%)	RAVE	PLAS		M.C.	LIQU		Elev
		NO SAMPLE RECOVERY - VACUUM EXCAVA	TED	- Hereit	8	+:	40	8	80	120	160	- LL	S	5	1	10 2	0 3	<u>30 40</u>	····	• _
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- 1 -																				
-	Hydro-Vac						÷		÷											
	Hydro																			10 -
- 2																				
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-							-													-
-							÷		÷											9 —
- 3		Becker hammer drilling (closed ended) to 32.0 n	depth - no soil samples	2	110															
-		recovered		9	110		-													
-							-													8 -
- 4				12	110				1											
				15	110															-
-				12	110	•	÷													
E				11	110	Þ	-													7 -
- 5				12	110		· · ·	•	;1. _/	·····	•							· · · · · · · · ·		-
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6	Ended)			8	79] : ا												
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-	Becker Hammer (Closed			9	79				-											-
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		1066 West Hastings Stree	DRILLING CO	NTRA	сто	R:	Fou	nde	x Ex	plor	ation	S	-		C	OMPI	ETIC	ON DE	PTH: 3	2m
		Vancouver BC V6E 3X2	DRILLING RIG		: HA	٩V-	180	Be	cker	Har	nmer				_			4/24/2		
	E	CANADA p. 604.685.0275 f. 604.684.6241	LOGGED BY:												_			:D: 4/2	4/2012	
AT	ETRA	TECH COMPANY 1. 604.684.6241	REVIEWED B	Y: BH											Pa	age 1	of 4			

Attachment	3
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	-1	COT AL	BORE	HC)L	E LOG: B	H12	-03
	5	CONSTRUCTION	LOCATION: Clar	ke St.	@	Grant St., Port Moody		PROJECT NO: V13103058-01
		CONSTRUCTION	STATION: 519+8			OFFSET: 15 m R		ELEVATION: 11.7 m
CLIE	NT:	SNC-Lavalin	NORTHING: 545	8152		EASTING: 510787	7	ZONE: 10
				PENETRA		BOUNCE CHAMBER PRESSURE	PARTICLE	▲ POCKET PEN. (kPa) ▲
Ê	HH			TEST	S	<u> </u>	SIZE DISTRIBUTIO	N FIELD VANE (kPa)
Depth (m)	DRILLING METHOD	DESCRIPTION		(sw	a) er	BECKER PENETRATION TEST	(%)	N FIELD VANE (kPa) REMOULDED PEAK 10 20 30 40 PLASTIC M.C. LIQUID PLASTIC M.C. LIQUID
Dep	LLIN			BPT N _b (Blows)	Bounce Chamber Pressure (kPa)	(N _b) O BECKER OPEN HOLE TEST (N _b) (Blows/305mm)	FINES (%) SAND (%) SRAVEL (%)	PLASTIC M.C. LIQUID
	DRI			BPT	Pres	(Blows/305mm) 40 80 120 160	FINES (° SAND (° GRAVEL	
_		Becker hammer drilling (closed ended) to 32.0 m dept	h - no soil samples	26 1	107			
-		recovered						
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- 11				18 1	107	•		
÷				10 1	107	•		
E				9	79	• í í		
E				11	79			0 -
- 12				10	79			
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-				10	79			1 -
- 13				11	79	••••••••••••••••••••••••••••••••••••••		
Ę				11	79			
-				10	79			
				11	79			-2 -
- 14	_			11	79			
Ē	Becker Hammer (Closed Ended)							
Ē	ed E			11	79			
È	(Clos			11	79	•		
15 	mer			11	79	•		
F	Han			11	79			
-	ecker			11	79			-4 -
- 16	ā			11	79			
- 10						T l		_
E				14	79			
F				14	79			-5 -
- 17				12	86	•		
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-				12	86			-6 -
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DT 13				18	93			
EBA.G				16 9	93	•		
1.GPJ				15 9	93	♦		-7 -
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RT 20				17 1	107			
н Ц Ц					90			
31030				16	30			-8 -
(ER V1						R: Foundex Explorations	↓ s	COMPLETION DEPTH: 32m
CVERGREEN - BECKER V13100301 ELRT 2013/2211 GPJ EEA GDT 1203.08		1066 West Hastings Street Vancouver				V-180 Becker Hammer	-	STARTED: 4/24/2012
REEN	E	BC V6E 3X2 CANADA p. 604.685.0275	LOGGED BY: AT		(COMPLETED: 4/24/2012
T A	ETRA	TECH COMPANY f. 604.684.6241	REVIEWED BY:					Page 2 of 4

			BORE	HC)L	E LOG: BH12	-03
	-	CONSTRUCTION				Grant St., Port Moody	PROJECT NO: V13103058-01
			STATION: 519+8	815		OFFSET: 15 m R	ELEVATION: 11.7 m
CLIE	NT:	SNC-Lavalin	NORTHING: 545	58152		EASTING: 510787	ZONE: 10
(L	DRILLING METHOD			PENETRA TEST	ATION S	BOUNCE CHAMBER PRESSURE <u> <u> <u> </u> <u> </u></u></u>	
Depth (m)	NG ME	DESCRIPTION		(swo	nber Pa)	BECKER PENETRATION TEST	N FIELD VANE (kPa) REMOULDED PEAK 10 20 30 40 PLASTIC M.C. LIQUID PLASTIC M.C. LIQUID
Det	SILLIN			BPT N _b (Blows)	Bounce Chamber Pressure (kPa)	BECKER PENETRATION TEST (N,) (N,) (Biows/305mm) 40 80 120 160 (N) (N)	
	ä	Becker hammer drilling (closed ended) to 32.0 m depth	no coil compleo		ਡੋ ^ਣ 90	40 80 120 160 E 00 E	<u>5</u> <u>10</u> 20 <u>30</u> 40
-		recovered	- no son samples		90		
-				16	90		
				17	90		-9
- 21				19 1	103		
-				19 1	103		
				30 1	114		-10 -
- 22				31 1	110		
				23 1	114		
-				24 1	110		-11
- 23				19 1	103		
-				58 1	141		
-				98 1	148		
-				57 1	145		-12
- 24 -	(pe				152		
-	Becker Hammer (Closed Ended)				152		
	Closed				155		-13 —
25	ner (C				155		
-	Ham				114		
-	ecker				128		-14 —
26	ш				138		
					131		
- 					145		
F					145		-15
27 					145 .		
Ē					138		
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					140		
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					152 128		-17
_ 29					128	·····	
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F					138		
F				63 1	138		-18
		1066 West Hastings Street				R: Foundex Explorations	COMPLETION DEPTH: 32m
29	-	Vancouver BC V6E 3X2 CANADA			HA	V-180 Becker Hammer	STARTED: 4/24/2012
AT	ETRA	CANADA p. 604.685.0275 f. 604.684.6241	LOGGED BY: A				COMPLETED: 4/24/2012
			REVIEWED BY:	вн			Page 3 of 4

	-1			BORE	H(C	E	L	00):	В	Η	12	2-	03	3					
	٦		NSTRUCTION	LOCATION: Cla	rke S	t. @	Gran	t St.,	Port I	Иоо	dy				PRO)JE(CT N	0: V1	13103	8058-	01
			NSTRUCTION	STATION: 519+	815			OFF	SET:	15 n	n R				ELE	VAT	ION	: 11.7	'n		
CLIE	NT:	SNC-Lavalin		NORTHING: 54	58152	2		EAS	TING	510	0787	,			ZON	NE: 1	0				
	ДO				PENETI	RATION	BOUI	NCE CH	IAMBER F (kPa)	PRESS	SURE	P/		LE		POC 00 2	KET P	EN. (kPa 300 4			
(E	DRILLING METHOD				TES	STS		40 -	<u> </u>) 1	60	DIST	SIZE FRIBU			FIE	LD VAN	VE (kPa)		_	Elevation (m)
Depth (m)	NG Ng		DESCRIPTION		lows)	amber kPa)	• BE	CKER I		TION	TEST	(%)	(%)	GRAVEL (%)	1		20		◆ 40	Backfill	atior
පී	RILLI				BPT N _b (Blows)	Bounce Chamber Pressure (kPa)	O BEO	CKER C (Blo	(N₀) PEN HOL ws/305m	.E TES m)	ST (N _b)	FINES (%)	SAND (%)	AVE	PLAS		M.C.		QUID		Elev
		Bocker hammer drill	ing (closed ended) to 32.0 m depth	no soil samples	년 55	_≊ ≏ 145			30 12		60	Ē	S	ц	1	10	20	30	40		
Ę		recovered			43	152				Ì											
-					34	148				i											
Ē							ΙΙ														-19 —
- 31 E					37	145				-1											_
-					77	152		Ĩ		1	-22										-
Ē					227	159					25										-20
					250	159					20	•									
		End of Testhole at 3 - Reported BPT blov	2.0 m (refusal). vcounts are uncorrected field value	s.																	
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																	ŀ	·····			
<u> </u>		i					 . г				tic	<u> </u>							<u>:</u> דיידי		
			1066 West Hastings Street Vancouver	DRILLING CON								5			_			ion [: 4/24			n.
	Ē	bo	BC V6E 3X2 CANADA	LOGGED BY: A		HA	v-10	u de		all	IIIEI				_			ED: 4			
AT	ETR	TECH COMPANY	p. 604.685.0275 f. 604.684.6241	REVIEWED BY											-		l of 4		., 27, 2		

			B	OR	EH	OLE LOG: MR12	-S3-524		
	-	CONSTRUCTION	LOC			Street between Grant St and Mary St	PROJECT NO: V13103058-01		
			SIA		519+780		ELEVATION: 11 m		
CLIE	NT:	SGJV	1 1	THING	G: 54581		ZONE: 10		
Depth (m)	DRILLING METHOD	DESCRIPTION	GRAPHICAL LEGEND	SAMPLE NUMBER	PENETRATION TESTS (Blows/300mm)	SHEAR WAVE VELOCITY (m/s) SIZE 1 160 240 480 640 DISTRIBUTION RE	▲ POCKET PEN. (kPa) ▲ 00 200 300 400 FIELD VANE (kPa) 10 20 30 40 STIC M.C. LIQUID STIC M.C. LIQUID		
Del	DRILLIN		GRAPHI	SAMPI	PENETR (Blow		STIC M.C. LIQUID 20 40 60 80		
- - - - -		ASPHALT - 200 mm SAND, gravelly, trace silt, damp, brown; fine to medium sand; fine and coarse, subrounded to subangular gravel (FILL)		1					
- - - - - - - - - - - - -	Hydro-Vac	SAND, gravelly, trace to some silt, moist, orangey brown; fine sand; fine and coarse subangular to subrounded gravel; contains roots (POSSIBLE FILL)		2		15 79 6			
2		- brown at 2.3 m	° 0 0 0	3		• 2 73 25			
		SAND, some gravel, some silt, moist, browny orange; fine to coarse sand; fine gravel SPT* at 3.0 m: 5 / 2 // 2 / 1 / 1 / 3 / 3 / 3 (N = 7), Recovery = 77% (*blowcounts recorded in 75 mm intervals) SAND, silty, trace gravel, moist, loose, grey; fine sand; coarse subrounded gravel; contains dark brown elongate wood pieces up to 15 mm long		4	7	■ ● 13 68 19 ●			
- - - - - - - - - - - - - - - - - - -	Circulation)	 SAND, some silt, trace to some gravel, damp, compact, browny grey; fine to coarse sand; fine subangular to subrounded gravel; contains thin lenses of stiff grey SILT SPT* at 4.6 m: 2 / 4 // 4 / 4 / 4 / 3 / 5 (N = 16), Recovery = 62% (*blowcounts recorded in 75 mm intervals) 	4	5	16	• 13 77 10 •	6		
- 6 	Mud Rotary (Normal (SPT* at 6.1 m: 6 / 5 // 4 / 4 / 3 / 4 / 4 / 5 (N = 15), Recovery = 66% (*blowcounts recorded in 75 mm intervals)	T Z	6	15	11 77 12 •			
0130201.GPJ EBA.GDT 13/03/08		-grey below 7.6 m SPT* at 7.6 m: 6 / 7 // 6 / 8 / 7 / 9 / 8 / 9 (N = 30), Recovery = 81% (*blowcounts recorded in 75 mm intervals)	7	7	30	• 15 83 2 •	3		
EVERGREEN - SOIL AUGERROTARY V13103031_ELRT_20130201.GPJ EBA GDT 1303/08 6 6 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9		SILT, sandy, trace clay, trace gravel, damp, firm, grey, non-plastic; fine sand; contains frequent elongate fragments of dark brown wood debris up to 20 mm long SPT* at 9.1 m: 1 / 1 // 2 / 2 / 1 / 1 / 1 / 2		8	6	67 32 1			
OIL AL		1066 West Hastings Street				ACTOR: Mud Bay Drilling Co.	COMPLETION DEPTH: 38.2m		
S-N-	-	Vancouver BC V6E 3X2 CANADA				PE: Fraste Multidrill XL	STARTED: 12/4/2012		
RGRE	C	p. 604.685.0275	LOGGED BY: RB COMPLETED: 12/6/2						
	TETRA	TECH COMPANY 1. 004.004.0241	RE\	IEWE	D BY: BH	1	Page 1 of 5		

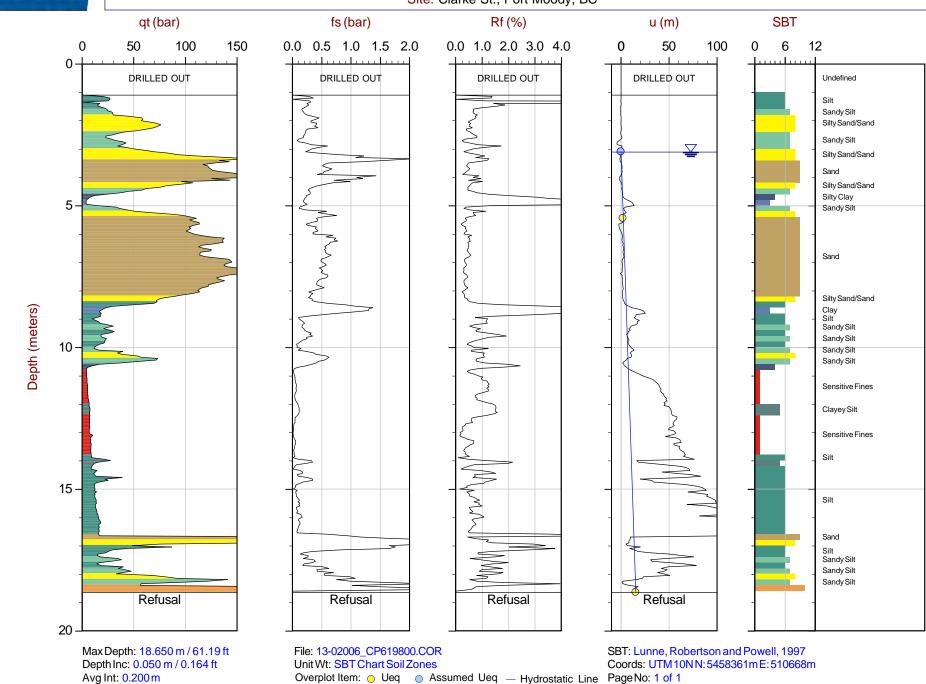
F						OLE LOG: MR12-				
		CONSTRUCTION			: Clarke 519+780	, ,	PROJECT NO: V13103058-01 ELEVATION: 11 m			
CLIE	NT: S	SGJV			54581		ZONE: 10			
Depth (m)	DRILLING METHOD	DESCRIPTION	GRAPHICAL LEGEND SAMPLE TYPE	r	TION TESTS (300mm)	SHEAR WAVE VELOCITY (m/s) SIZE DISTRIBUTION REMC	POCKET PEN. (kPa) ▲ 200 300 400 FIELD VANE (kPa) ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓			
-		(N = 6), Recovery = 87% (*blowcounts recorded in 75 mm intervals)				•				
- 11 -		ORGANIC SILT, some clay to clayey, trace sand, trace gravel, trace shells, moist, firm, grey, high plasticity; fine sand, fine subangular gravel		ST1		······				
- 12		Nilcon vane test at 11.7 m								
- - 13		Inferred SILT, some clay, trace sand, moist, firm, grey, low plasticity		ST2						
-		Nilcon vane test at 13.3 m <u>CLAY, silty, trace sand, trace gravel, moist, firm,</u>				•				
- 14	(Normal Circulation)	grey, low plasticity; fine to medium sand; fine subangular gravel		ST3		••••••••••••••••••••••••••••••••••••••	-3			
- 15 - - 16	Mud Rotary (Normal (Nilcon vane test at 14.8 m				•				
· · 17	-	SILT, sandy, some clay, moist, stiff, grey, low plasticity; fine sand SPT* at 16.5 m: 9 / 6 // 4 / 2 / 2 / 1 / 2 / 2 (N = 9), Recovery = 116% (*blowcounts recorded in 75 mm intervals)		9	9					
- 18 - 19 -		SILT, some sand, some clay, damp, stiff, non-plastic, grey; fine sand SPT* at 18 m: 3 / 3 // 3 / 3 / 2 / 2 / 3 / 2 (N = 10), Recovery = 100% (*blowcounts recorded in 75 mm intervals)		10	10	84 16 0	-7 -7 -7 -7 -7 -7 -7 -7 -7 -7			
		2	י ווקח ו ווקח			ACTOR: Mud Bay Drilling Co.	COMPLETION DEPTH: 38.2m			
		1066 West Hastings Street Vancouver BC V6E 3X2				E: Fraste Multidrill XL	STARTED: 12/4/2012			
	e	CANADA p. 604.685.0275			BY: RB		COMPLETED: 12/6/2012			
AT	ETRA	TECH COMPANY 1. 604.684.6241	REVIEWED BY: BH Page 2 of 5							

E		CONSTRUCTION	LO	CAT	FION		OLE Street betw		ant St	and N		St F	PROJE	ECT NO: 1	/13103	058-0)1
CLIE	NT:	SGJV		RT	HING	G: 54581	50	EASTIN	IG: 51	0753			ZONE				
Depth (m)	DRILLING METHOD	DESCRIPTION	GRAPHICAL LEGEND	SAMPLE TYPE	SAMPLE NUMBER	PENETRATION TESTS (Blows/300mm)) 480 PENETRATIO (N) ETRATION 1 s/300mm)	640 ON TEST	S DISTR	SAND (%) BIZZI BIZ	REMOI	200 FIELD VA JLDED 20	NE (kPa) PEAK 30 40	VW23920	VW24019	Elevation (m)
- 21 - 22 - 22 - 23 - 24 - 25 - 26 - 27 - 26 - 27 - 28 27 - 28 29 - 29	Mud Rotary (Normal Circulation)	SPT* at 20.7 m: 6 / 10 // 13 / 13 / 11 / 11 / 9 / 12 (N = 48), Recovery = 84% (*blowcounts recorded in 75 mm intervals) SAND, trace silt, trace gravel, damp, dense, grey; fine to coarse sand Pressuremeter Test #1 at 22.1 m Pressuremeter Test #1 at 22.1 m			11	48		·			90 2						-10 -11 -12 -13 -14 -15 -16 -17 -18
AT	ETRA	1066 West Hastings Street Vancouver BC V6E 3X2 CANADA p. 604.685.0275 f. 604.684.6241	DF LO	RILL IGG	ing Ed e		ACTOR: Mu PE: Fraste M			j Co.		· · · · · · · · · · · · · ·	STA CON	IPLETION RTED: 12 IPLETED: 3 of 5	4/2012		.2m

	-						OLE LOG: MR12-S3-524	
		CONSTRUCTION				l: Clarke 519+780	Street between Grant St and Mary St PROJECT NO: V13103058- OFFSET: 15 m R ELEVATION: 11 m	-01
LIE	NT: :	SGJV	-			G: 54581		
Depth (m)	DRILLING METHOD	DESCRIPTION	GRAPHICAL LEGEND	SAMPLE TYPE	SAMPLE NUMBER	PENETRATION TESTS (Blows/300mm)	● SHEAR WAVE VELOCITY (m/s) PARTICLE SIZE ▲ POCKET PEN. (kPa) ▲ 100 200 300 400 ● FIELD VANE (kPa) ▲ 100 200 300 400 ● FIELD VANE (kPa) ▲ PEMULED ● FIELD VANE (kPa) ▲ PEMULED <t< th=""><th>Elevation (m)</th></t<>	Elevation (m)
31 32 33 34 35 36 37 38	Mud Rotary (Normal Circulation)	SPT* at 38.1 m: 60 / 93 // - / - / - / - / - / - (N > 50), Recovery = 158% (*blowcounts recorded in 75 mm intervals) End of Testhole at 38.2 m (target depth reached). - Testhole grouted to surface with 63.5 mm diameter Sch. 40 PVC casing and vibrating-wire nie. Zorder with 63.5 mm diameter Sch. 40 PVC casing and vibrating-wire nie. Zorder surface with 63.5 mm diameter Sch. 40 PVC casing and substance with 63.5 mm diameter Sch. 40 PVC casing and su			5 12	>153	50 43 7	-20 - -21 - -22 - -23 - -24 - - -25 - - - -25 - - - -26 - - -
		performance, insitu test results and visual classification of recovered samples. These 1066 West Hastings Street Vancouver	-				ACTOR: Mud Bay Drilling Co. COMPLETION DEPTH: 38	8.2m
	E	BC V6E 3X2 CANADA	-			RIG TYF 3Y: RB	PE: Fraste Multidrill XL STARTED: 12/4/2012 COMPLETED: 12/6/2012	
AT	ETRA	p. 604.685.0275 f. 604.684.6241				DBY:BF		
			R			אוסע	1 Page 4 01 5	

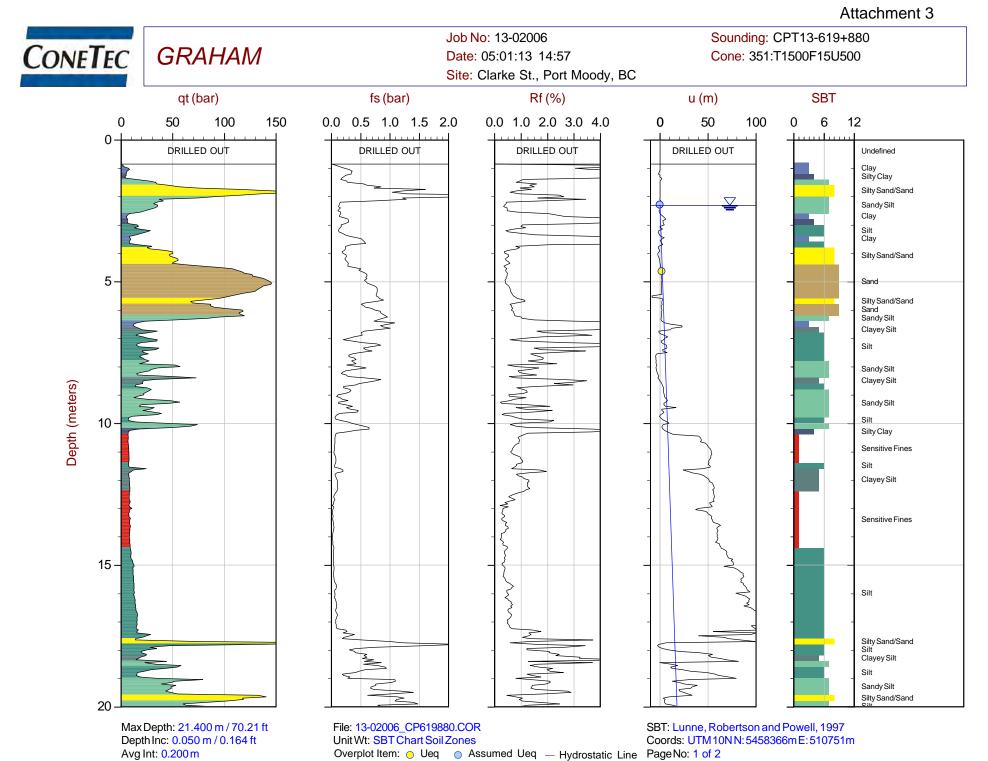
			B	\mathbf{C})R	EH	0		E	•		С)(3:	ľ	M	R	1	2	-(S	3	-{	52	24			
	-	CONSTRUCTION	LO	CA	TION	: Clarke	Stre	et	bet	we	en	Gr	ant	St	anc	l Ma	ary S	St		P	RC)JE	C	ΓN	D: V	1310	3058	-01
	1		ST/	٩TI	ON: {	519+780				C)FF	SE	ET:	15	m F	2				E	LE	VA	TIC	ON:	11	m		
CLI	ENT:	SGJV	-	RT	HING	G: 545815	50			E	AS	STI	NG	: 51						_			10					
Depth (m)	DRILLING METHOD	DESCRIPTION	GRAPHICAL LEGEND	SAMPLE TYPE	SAMPLE NUMBER	PENETRATION TESTS (Blows/300mm)	• •	SHE 160		240	48	80	640		DIS	ARTI SIZI TRIBI	e Jtio	_	1(RE)0 F MOUI	200 IELC .DED) VAI	300 NE (k	(Pa) PE	DO AK	VW23920	VW24019	Elevation (m)
Dept	DRILLING		GRAPHIC	SAMP	SAMPLE	PENETRA (Blows		.ARG 10	E PE (Bl	() ENET	N) RAT 300m <u>30</u>	'ION nm)	TES 40	T (N)	FINES (%)	SAND (%)	GRAVEL (%)		PLAS		20 4(M.C	30	-	UID 100	- CM	ZMA	Elevat
		estimates are based on engineering judgement and are subjective. - SPT blowcounts were recorded in 75 mm (3") intervals.																										
		 Reported blowcounts are uncorrected field values. SPT recovery is based on the length of the 																										
		recovered sample compared to the distance driven. - Moisture content values for cohesionless soils may not be representative of in-situ																										
		conditions due to drilling disturbance. - Borehole location, collar elevation and chainage/offset are approximate.																										
																					••••							
									•																•			
13/03/08																					•••							
SPJ EBA.GDT																												
EVERGREEN - SOIL AUGERROTARY V13103031_ELRT_20130201.GPJ EBA.GDT 13/03/08																												
/13103031_EL																												
ER/ROTARY \								· · · · · · · · · · · · · · · · · · ·																				
	-	1066 West Hastings Street	DR	ILL	ING	CONTRA	\CT	OF	8: N	lud	Ba	ay l	Dril	ling	Cc).	+	-+		Ï	C	ЭM	IPL	ETI	ON	DEP	гн: 3	8.2m
- SO	4	Vancouver BC V6E 3X2	DR	ILL	.ING	RIG TYP						-														4/201		
RGREE	E	CANADA p. 604.685.0275 f. 604.684.6241				BY: RB														-						12/6/2	2012	
A EVE	TETR	T. 604.684.6241	RE	VIE	EWE	D BY: BH															Pa	age	e 5 (of 5				

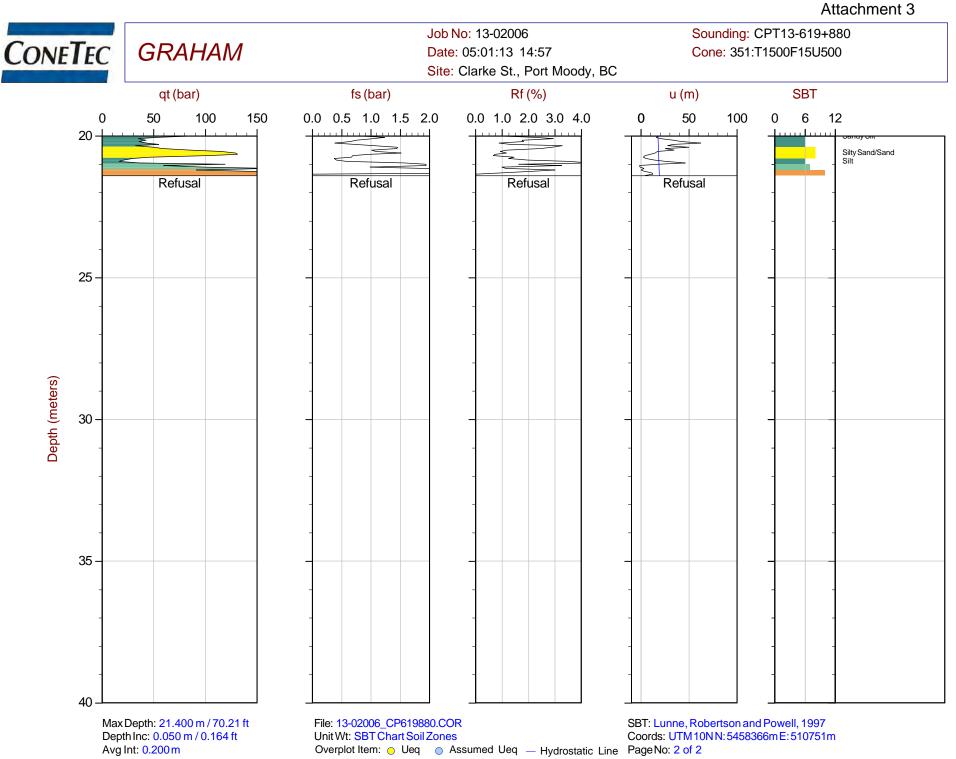
Attachment 3 Sounding: CPT13-619+800 Job No: 13-02006 Date: 05:01:13 12:47 Cone: 351:T1500F15U500 Site: Clarke St., Port Moody, BC Rf (%) SBT u (m) 0.0 1.0 2.0 3.0 4.0 12 0 50 100 0 6 line line li DRILLED OUT DRILLED OUT Undefined Silt Sandy Silt Silty Sand/Sand Sandy Silt ∇ Silty Sand/Sand Sand Silty Sand/Sand Silty Clay Sandy Silt Sand

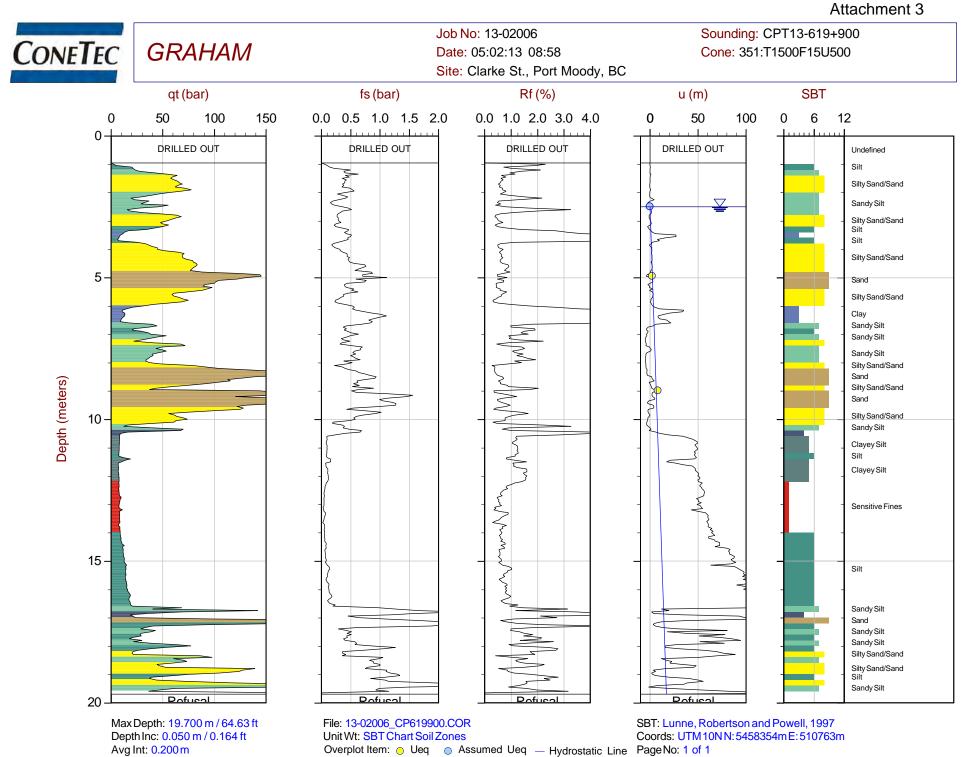


CONETEC

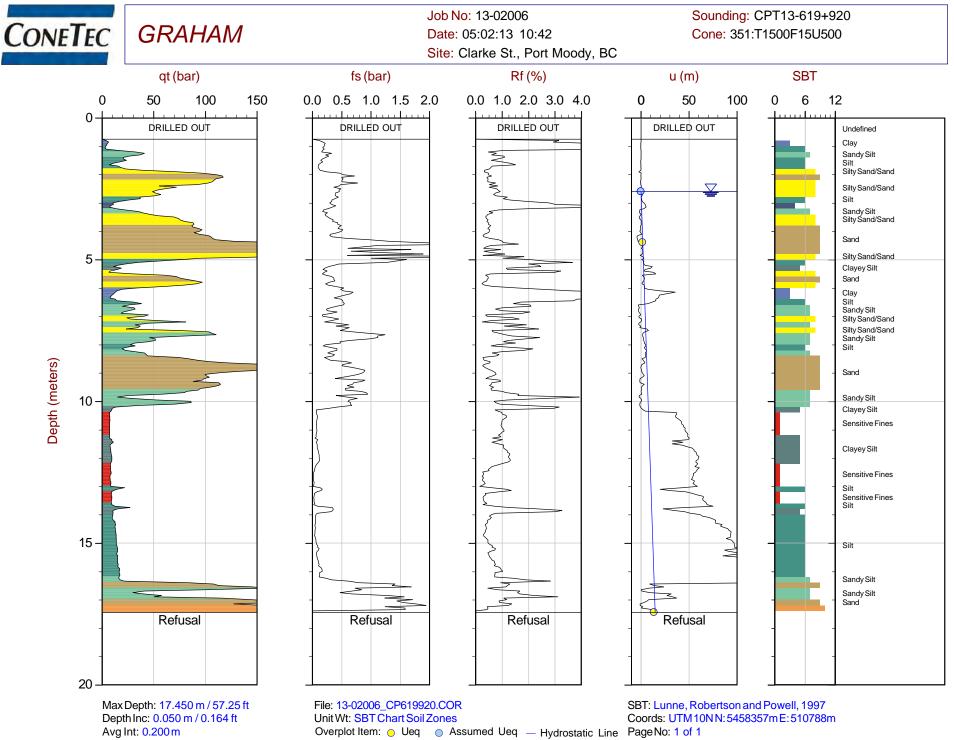
GRAHAM



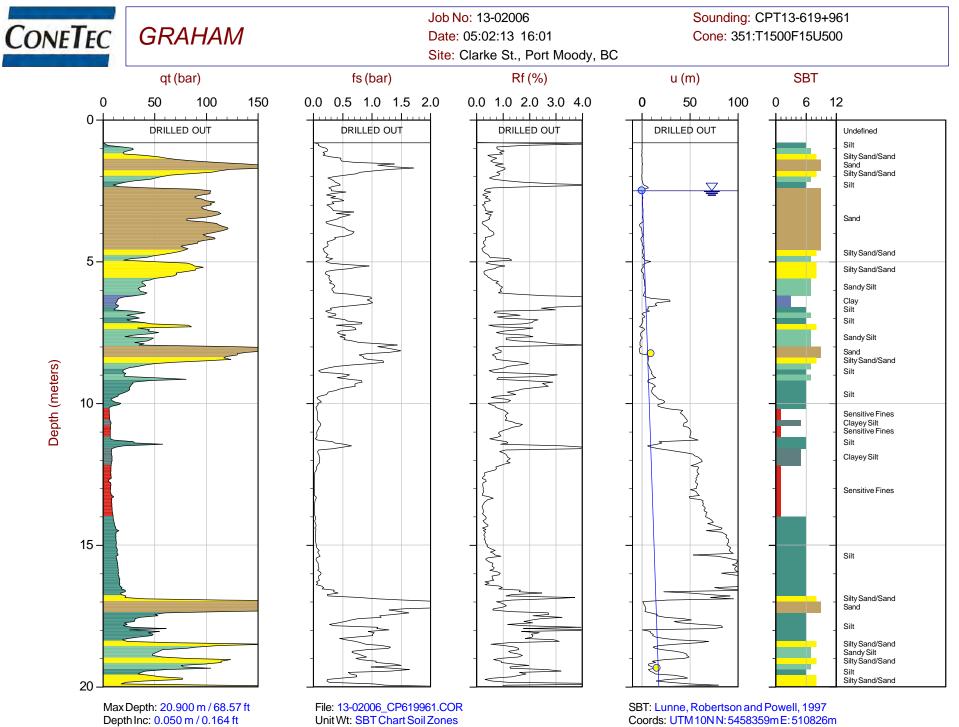






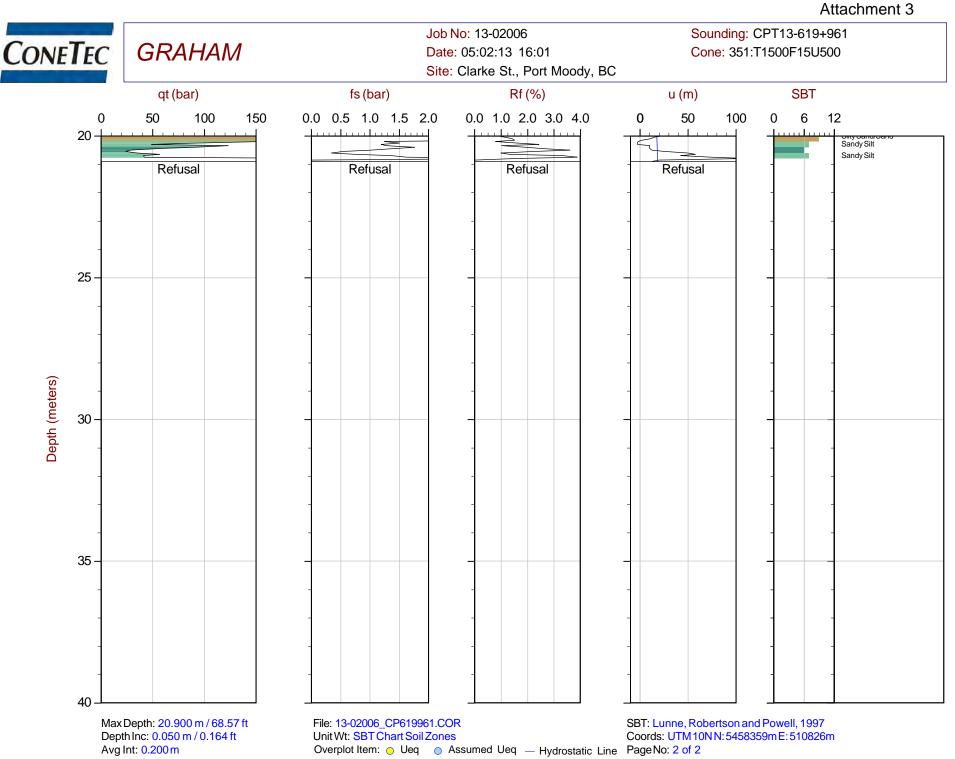


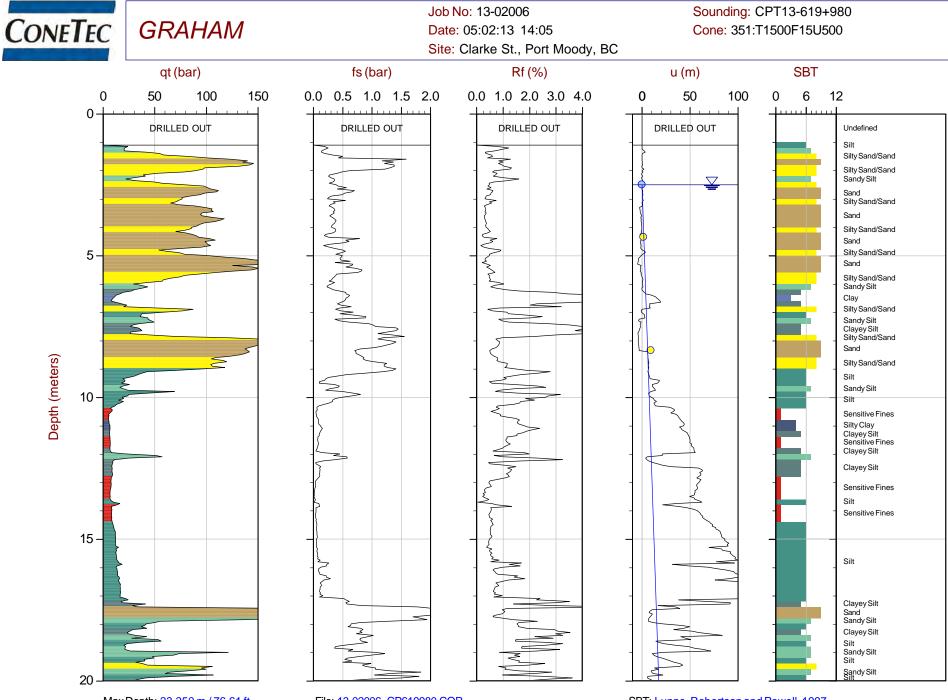




Overplot Item: O Ueq O Assumed Ueq - Hydrostatic Line Page No: 1 of 2

Avg Int: 0.200 m





Max Depth: 23.350 m / 76.61 ft Depth Inc: 0.050 m / 0.164 ft Avg Int: 0.200 m

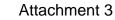
 File: 13-02006_CP619980.COR
 SBT: Lunne, Ro

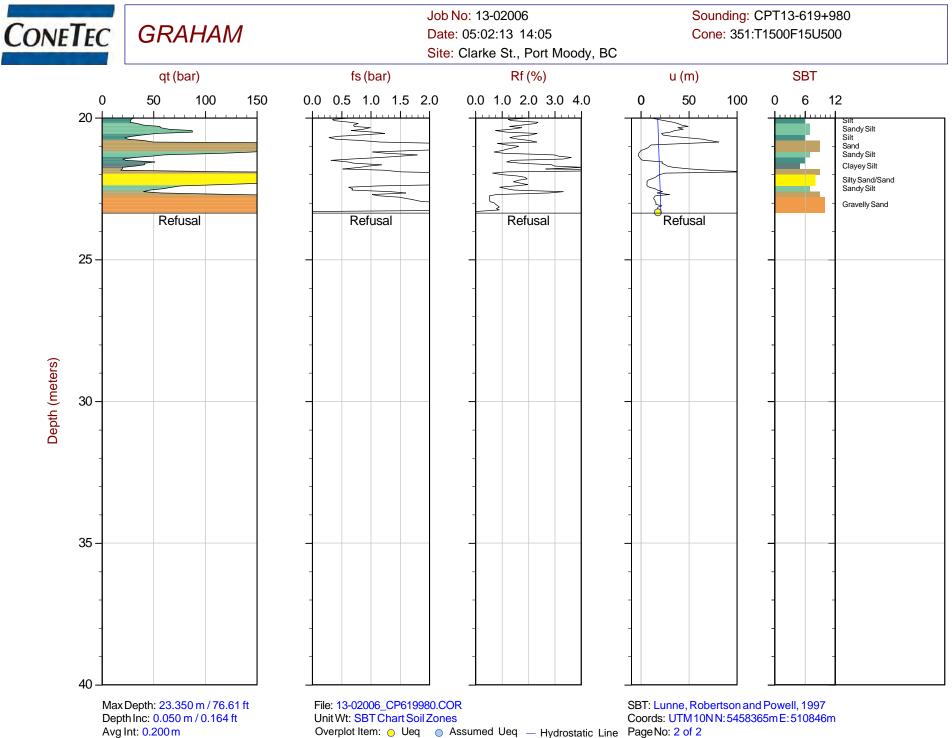
 Unit Wt: SBT Chart Soil Zones
 Coords: UTM10

 Overplot Item:
 Ueq
 Assumed Ueq
 Hydrostatic Line
 Page No: 1 of 2

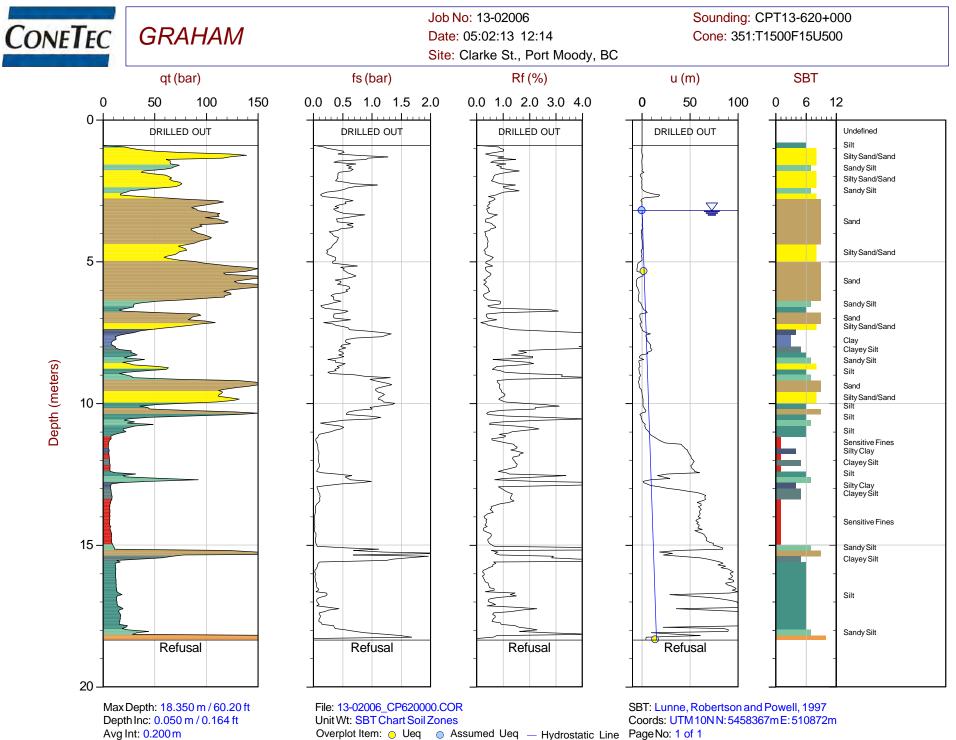
SBT: Lunne, Robertson and Powell, 1997 Coords: UTM10NN:5458365mE:510846m

Attachment 3









Appendix III

NBCC 2020 Seismic Hazard Parameters



Government of Canada

Gouvernement du Canada

Canada.ca > Natural Resources Canada > Earthquakes Canada

2020 National Building Code of Canada Seismic Hazard Tool

This application provides seismic values for the design of buildings in Canada under Part 4 of the National Building Code of Canada (NBC) 2020 as prescribed in Article 1.1.3.1. of Division B of the NBC 2020.

Seismic Hazard Values

User requested values

Code edition	NBC 2020
Site designation X _S	X _D
Latitude (°)	49.278
Longitude (°)	-122.852

Please select one of the tabs below.

NBC 2020	Additional Values	Plots	API	Background Information
HDC LOLO		11013	/ /	Backgroana momation

The NBC 5% damped spectral acceleration values can be viewed in the NBC tab. Additional hazard values for your site can be found below.

The 5%-damped spectral acceleration ($S_a(T)$, where T is the period, in s) and peak ground acceleration (PGA) values are given in units of acceleration due to gravity (g, 9.81 m/s²). Peak ground velocity (PGV) is given in m/s. Probability is expressed in terms of percent (%) exceedance in 50 years.

By default, all probabilities for the user-specified site designation are shown. Other site designations can be selected from the respective drop-down menu in the table. In low hazard regions, a minimum value of 0.001g for $T \le 2.0s$ and of 0.0001g for T > 2.0s is assigned. Further information on the calculation of seismic hazard is provided in the *Background Information* tab.

Site Designation	Probability	S _a (0.05)	S _a (0.1)	S _a (0.2)	S _a (0.3)	S _a (0.5)	S _a (1.0)	S _a (2.0)	S _a (5.0)	S _a (10.0)	PGA	PGV
XD ~	Ali 🗸											
X _D	2	0.606	0.868	1.02	1.12	1.06	0.753	0.46	0.142	0.0541	0.468	0.697
X _D	2.5	0.556	0.801	0.953	1.05	0.982	0.689	0.412	0.121	0.0448	0.435	0.629
X _D	3.5	0.489	0.703	0.851	0.931	0.866	0.599	0.345	0.0942	0.0338	0.387	0.538
X _D	5	0.423	0.61	0.75	0.819	0.753	0.51	0.282	0.0725	0.0254	0.339	0.45
X _D	7	0.365	0.53	0.661	0.717	0.654	0.433	0.23	0.057	0.0197	0.295	0.378
X _D	10	0.311	0.452	0.57	0.618	0.555	0.36	0.183	0.0442	0.015	0.253	0.31
X _D	14	0.263	0.385	0.491	0.529	0.469	0.298	0.145	0.0346	0.0115	0.216	0.254
X _D	20	0.216	0.319	0.412	0.44	0.386	0.238	0.112	0.0262	0.00849	0.178	0.203
X _D	30	0.167	0.247	0.325	0.345	0.298	0.179	0.0811	0.0184	0.00572	0.139	0.153
X _D	40	0.133	0.199	0.265	0.281	0.24	0.141	0.0625	0.0138	0.00412	0.112	0.121

Download CSV

← Go back to the <u>seismic hazard calculator form</u>

Date modified: 2021-04-06

Appendix IV

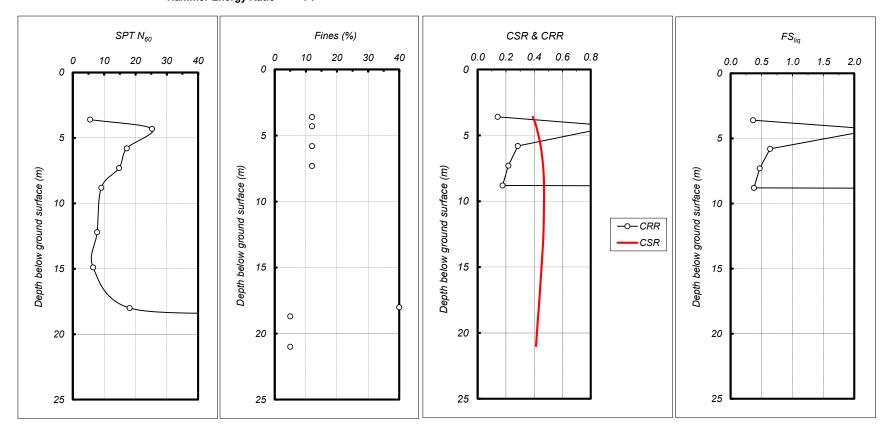
Liquefaction Triggering Assessment

2022-12-15



Project: Moody Centre PPS Borehole: BH09-305 Station: 0

- Input parameters:
- Peak ground accel (g) = 0.47
- Earthquake magnitude, M = 7.1
- Water table depth (m) = 2
- Average g above water table (kN/m3) = 18
- Average g below water table (kN/m3) = Hammer Energy Ratio= 18
 - 74



GEOLOGISHIKI Geotechnical Software Geotechnical Software http://www.geologismiki.gr

SPT BASED LIQUEFACTION ANALYSIS REPORT

Project title : Moody Centre PPS Building

Location : Port Moody, BC

EQ site conditions:

Same as initial

2.00

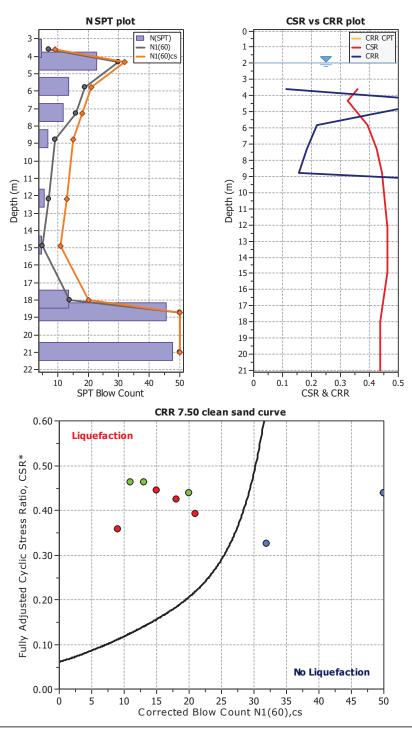
2.00

7.10 0.47 Nearest

Borehole Name : BH09-305

:: Input parameters and analysis properties ::

Analysis method:	Idriss & Boulanger 2014	G.W.T. (in-situ):	
Fines correction method:	Idriss & Boulanger 2014	G.W.T. (earthq.):	1
Sampling method:	Standard Sample	Earthquake magnitude M:	1
Borehole diameter:	65 mm to 11 5mm	Peak ground acceleration:	
Rod length:	1.50	SPT results rounding mode:	
Hammer energy ratio:	1.23		



 $\label{eq:CLiq:v.3.3.2.9-CPT Liquefaction Assessment Software - Report created on: 2022-12-15, 9:55:15 \mbox{ AM Project file: C:\MKB\Projects\Port Moody PPS\Liquefaction\BH09-305.clq}$

CPT name: BH09-305

No	Depth (m)	Weight (kN/m³)	u₀ (kPa)	σ _v (kPa)	Ext. Load (kPa)	oʻ, (kPa)	r _d	CSR	Kσ	MSF _{max}	MSF	CSR*
1	3.60	18.00	15.70	64.80	0.00	49.10	0.97	0.390	1.06	1.17	1.02	0.358
2	4.30	18.00	22.56	77.40	0.00	54.84	0.96	0.414	1.10	2.12	1.16	0.325
3	5.80	18.00	37.28	104.40	0.00	67.12	0.94	0.446	1.06	1.53	1.07	0.392
4	7.30	18.00	51.99	131.40	0.00	79.41	0.91	0.462	1.03	1.42	1.06	0.424
5	8.80	18.00	66.71	158.40	0.00	91.69	0.89	0.470	1.01	1.32	1.04	0.445
6	12.20	17.00	100.06	216.20	0.00	116.14	0.83	0.472	0.99	1.26	1.04	2.000
7	14.90	17.00	126.55	262.10	0.00	135.55	0.78	0.463	0.97	1.21	1.03	2.000
8	18.00	18.50	156.96	319.45	0.00	162.49	0.73	0.439	0.94	1.49	1.07	2.000
9	18.70	18.50	163.83	332.40	0.00	168.57	0.72	0.433	0.85	2.20	1.17	0.439
10	21.00	18.50	186.39	374.95	0.00	188.56	0.68	0.416	0.81	2.20	1.17	0.438

Abbreviations

Depth: Depth from free surface where SPT was performed (m) during eq.

 u_0 :

Water pressure at test point (kPa) during eq. Total overburden pressure at test point (kPa) during eq.

σ_v: σ_v': Effective overburden pressure based on GWT during earthquake (kPa) during eq.

r_d: Nonlinear shear mass factor

CSR: Cyclic Stress Ratio

MSF: Effective overburden stress factor

K₀: CSR*: Magnitude Scaling Factor

CSR fully adjusted

:: Cyclic Resistance Ratio (CRR) numeric results ::

No	Depth (m)	Fines %	u₀ (kPa)	σ _v (kPa)	σ' _v (kPa)	N _{spt}	C _N	C _R	C _B	Cs	C _E	N₁₍₆₀₎	Δ(N ₁) ₆₀	N_{1(60),cs}	CRR _{7.5}	F.S.
1	3.60	12.00	15.70	64.80	49.10	5	1.49	0.85	1.00	1.00	1.23	7	2.07	9	0.111	0.31
2	4.30	12.00	22.56	77.40	54.84	23	1.24	0.85	1.00	1.00	1.23	30	2.07	32	0.644	1.98
3	5.80	12.00	37.28	104.40	67.12	14	1.19	0.95	1.00	1.00	1.23	19	2.07	21	0.219	0.56
4	7.30	12.00	51.99	131.40	79.41	12	1.12	0.95	1.00	1.00	1.23	16	2.07	18	0.184	0.43
5	8.80	55.00	66.71	158.40	91.69	7	1.05	1.00	1.00	1.00	1.23	9	5.61	15	0.156	0.35
6	12.20	80.00	100.06	216.20	116.14	6	0.93	1.00	1.00	1.00	1.23	7	5.54	13	4.000	2.00
7	14.90	80.00	126.55	262.10	135.55	5	0.86	1.00	1.00	1.00	1.23	5	5.54	11	4.000	2.00
8	18.00	40.00	156.96	319.45	162.49	14	0.81	1.00	1.00	1.00	1.23	14	5.58	20	4.000	2.00
9	18.70	5.00	163.83	332.40	168.57	46	0.87	1.00	1.00	1.00	1.23	50	0.00	50	607.527	2.00
10	21.00	5.00	186.39	374.95	188.56	48	0.85	1.00	1.00	1.00	1.23	50	0.00	50	607.527	2.00

Abbreviations

- Depth from free surface where SPT was performed (m) Depth:
- Soil unit weight from previous test point to current (kN/m³) Weight:

u₀: Water pressure at test point (kPa)

Total overburden pressure at test point (kPa) σ_v :

- σ,': Effective overburden pressure based on in situ GWT (kPa)
- Number of blows count in the field (blows/30 cm) N_{SPT}:

 C_N : Overburden pressure factor

Energy ratio factor C_E:

Borehole diameter factor Rod length factor

C_B: C_R:

C_s: Sampling method factor

N₁₍₆₀₎: Number of blows corrected for 60% energy

Fines correction $\Delta N_{1(60),\,cs}$

- Number of blows corrected for 60% energy and fines N_{1(60),cs}:
- Cyclic Resistance Ratio for $M_{\,\scriptscriptstyle W}\,7.50$ CRR_{7.5}:

F.S.: Factor of safety against liquefaction



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